

# STORMWATER REPORT

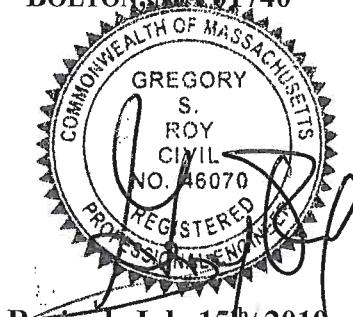
## ENCLAVE AT BOXBOROUGH

STOW ROAD  
BOXBOROUGH, MASSACHUSETTS

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6092

## **TABLE OF CONTENTS**

<b>1.0 Project Narrative .....</b>	<b>3</b>
1.1 <i>Project Type .....</i>	3
1.2 <i>Purpose and Scope.....</i>	3
1.3 <i>LID Measures.....</i>	3
1.4 <i>Site Description.....</i>	3
1.5 <i>Proposed Stormwater Management System.....</i>	4
1.6 <i>Methods of Analysis .....</i>	5
<b>2.0 Stormwater Standards Compliance.....</b>	<b>6</b>
2.1 <i>Standard 1 – Untreated Discharge .....</i>	6
2.2 <i>Standard 2 – Peak Rate Attenuation.....</i>	6
2.3 <i>Standard 3 – Recharge.....</i>	7
2.4 <i>Standard 4 – Water Quality .....</i>	7
2.5 <i>Standard 5 – Land Uses with Higher Pollutant Loads .....</i>	7
2.6 <i>Standard 6 –Critical Areas .....</i>	7
2.7 <i>Standard 7 – Redevelopment.....</i>	7
2.8 <i>Standard 8 – Construction Period Pollution Prevention Plan and Erosion and Sediment Control.....</i>	8
2.9 <i>Standard 9 – Operation and Maintenance Plan .....</i>	8
2.10 <i>Standard 10 – Prohibition of Illicit Discharge .....</i>	8
<b>3.0 Appendices.....</b>	<b>9</b>
Appendix A - Locus & Zoning Map .....	10
Appendix B - Checklist for Stormwater Report.....	12
Appendix C - Soils Data.....	13
Appendix D - Existing Conditions Hydrologic Calculations.....	14
Appendix E - Proposed Conditions Hydrologic Calculations .....	15
Appendix F – Stormwater Calculations.....	16
Appendix G – Construction Period Pollution Prevention .....	17
Appendix H - Operation and Maintenance Plan .....	18
Appendix I - Long Term Pollution Prevention Plan.....	19
<b>4.0 Plans .....</b>	<b>21</b>
<i>Pre-development Watershed Plan.....</i>	22
<i>Post-development Watershed Plan .....</i>	23

## **1.0 Project Narrative**

### ***1.1 Project Type***

The applicant Boxborough Town Center, LLC is proposing the construction of a 50-unit Active Adult Homes Development on the south side of Route 111 just northerly of Burroughs Road. The proposed units consist of 50 homes within 25 duplex buildings approximately 2,356 SF in area. The proposed site is located on Assessor's Map 14- lots 46 & 50 (Parcels: 14-046-000 & 14-050-000). The proposed scope of construction also includes a private roadway, the extension of an existing well-access road as a means of temporary construction access, on-site parking, community garden, stormwater management systems, on-site septic systems including a pump station & leaching field & new utility connections and their associated appurtenances.

### ***1.2 Purpose and Scope***

This report has been prepared to comply with the requirements of the Stormwater Management Standards incorporated in the Massachusetts Wetlands Protection Act Regulations, 310 CMR 10.00. These standards are intended to promote increased groundwater recharge and prevent stormwater discharges from causing or contributing to the pollution of surface waters and ground waters of the Commonwealth. The standards aim to accomplish these goals by encouraging the greater use of low impact development (LID) techniques and improving the operation and maintenance of stormwater best management practices (BMP).

This report addresses compliance of the proposed development with each of the ten stormwater standards, it provides calculations to support the compliance information, and it provides a Long-Term Pollution Prevention Plan and an Operation and Maintenance Plan for the stormwater management system.

### ***1.3 LID Measures***

Care has been taken to lay out the proposed site in a manner that works with existing topography. BMPs, have been used to manage the stormwater runoff. Stormwater from the proposed impervious surface locations are routed to infiltration basins via land flow, curb and gutter systems, or the proposed drainage pipe system. The stormwater basins will reduce run off rates below pre-developed rates while providing water quality pre-treatment by sediment forebays.

### ***1.4 Site Description***

As mentioned, the subject site is found on the south side of Route 111 just northerly of Burroughs Road in Boxborough, MA. The undeveloped wooded site is located on Parcels: 14-046-000 & 14-050-000 (62.48 acres). The site consists chiefly of undeveloped wooded areas. Multiple wetland areas as shown on the

attached Site Plans are present on site located to the north of the proposed development. There is one intermittent stream located within the mentioned BVW areas. As such there is no Riverfront Area on the subject property. The NRCS soil survey information indicates that all of the site is underlain by soils classified as belonging to Hydrologic Soil Groups A (Carlton Fine Sandy Loam), B (Charlton-Hollis-Rock outcrop & Hollis-Rock Outcrop-Charlton complex), C (Paxton Fine Sandy Loam) & D (Swansea Muck, Freetown Muck, Ridgebury Fine Sandy Loam & Whitman Fine Sandy Loam).

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Soils belonging to group B have a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Soils belonging to group D have a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

Due to numerous test holes performed within the vicinity of the project, it was determined that the boundary lines of the HSG D Soils depicted via the NRCS web soil survey were not consistent with the in-situ field work. As such, the boundary lines have been adjusted to appropriately correspond with the testing.

Please refer to Appendix C for further information regarding the soils on-site & existing test hole data.

## ***1.5 Proposed Stormwater Management System***

Runoff from the proposed development will be conveyed and treated through a combination of Best Management Practices (BMP's). The following is a brief discussion of each conveyance and treatment BMP proposed.

### Deep Sump Hooded Catch Basin

Deep sump hooded catch basins are proposed to convey the runoff from the proposed paved areas and roofs to the infiltration basins. These catch basins will discharge to manholes and conventional storm drains.

### Infiltration Basin

The infiltration basins are designed to reduce the runoff rates and increase the groundwater recharge rates. Sediment forebays designed at the entrance of each basin were included to decrease the velocity of flow and increase the settlement of heavy solids prior to the infiltration basin. Riprap will also be installed at the inlet of the sediment forebays and the outlet of the basins to control the overflow of stormwater into the adjacent wetlands and reduce the potential for scouring.

### Grassed Swales

Proposed swales have been designed to convey the flows from the 100-year frequency event. The grass swales will receive runoff from the rears of the proposed roofs and will convey the stormwater flows to associated proposed infiltration basins. The proposed design of the site has increased the Post-Developed time of concentration in Post 7.A when compared to its Pre-Developed time of concentration. This is due to the chain of drainage swales conveying the runoff from the mentioned roofs to Infiltration Basin 1.

### Contech Vortsentry

A Vortsentry shall be installed within the existing drainage system at approximately STA 5+50 on the proposed access drive. The vortsentry enhances the gravitational separation of pollutants from stormwater flows by inducing a swirling motion of the water as it enters the settling chamber. This added device to the existing system will provide the pretreatment required for the added flow to the existing drainage system, creating a system that is compliant with stormwater standards.

## ***1.6 Methods of Analysis***

The United States Department of Agriculture Natural Resources Conservation Service (NRCS) soil cover complex methods (TR-20) were employed to compute runoff quantities for the subject property. Watershed analysis demonstrate that natural drainage patterns drain toward the wetlands (design point). Two design points were modeled to analyze the total runoff from the site. HydroCAD 10.0 computer software was employed in this hydrologic analysis.

A comparison of pre- and post-development runoff quantities at the analysis points were performed in order to design a stormwater management system that will limit peak rates of runoff from the development to predevelopment levels for

24-hour rainfall events of 2-, 10-, 25- and 100-year return frequencies. Watershed boundaries for existing conditions are depicted on the attached Predevelopment Watershed Plan. Post-Developed watershed boundaries are indicated on the Post-development Watershed Plan.

## 2.0 Stormwater Standards Compliance

### 2.1 Standard 1 – Untreated Discharge

The stormwater management system for the proposed development will not result in any new discharges of untreated stormwater to wetland resource areas.

Stormwater management structures have been designed such that there is no erosion or scour to wetland resource areas or waters of the Commonwealth.

### 2.2 Standard 2 – Peak Rate Attenuation

Hydrologic calculations for existing and proposed site conditions are included in Appendices D and E respectively. Calculations for 24-hour rainfall events of 2-, 10-, 25- and 100-year return frequencies are provided. The following table provides a summary of peak rates of runoff related to each of these storms for the design point through which all runoff from the subject property must flow. For all rainfall events considered, the proposed stormwater management system will control runoff from the development such that corresponding peak flows at the design point will be lower than pre-developed rates.

*Table 1: Wetland Design Point Runoff Summary*

	Pre-Developed (ft <sup>3</sup> / sec)	Post-Developed (ft <sup>3</sup> / sec)
<i>Design Point “A”</i>		
2-Year	3.66	3.66
10-Year	12.72	8.78
25-Year	20.15	11.88
100-Year	37.55	33.53
<i>Design Point “B”</i>		
2-Year	0	0
10-Year*	0*	0.01*
25-Year	0.02*	0.03*
100-Year	0.21*	0.24*

\* Design Point B in the Pre-developed condition consists of 91,035 SF of undeveloped woods. Of the 91,035 SF, 79,057 is HSG A & 11,978 SF is HSG B. This results in a weighted CN of 33. Due to the proposed grading & drainage design of the project we have greatly reduced the tributary area of Design Point B to 38,793 SF with a weighted CN of 37. Using the SCS runoff equation, the HydroCAD model computes a trivial increase in flow & volume during the 10-,

25-, and 100-year return frequency. Due to the tributary area reduction, we do not believe there will be an increase in offsite peak flow or volume.

### ***2.3 Standard 3 – Recharge***

NRCS data indicates that the areas within the proposed development consist of soils classified as Carlton Fine Sandy Loam, Charlton-Hollis-Rock outcrop & Hollis-Rock Outcrop-Charlton complex), Paxton Fine Sandy Loam, Swansea Muck, Freetown Muck, Ridgebury Fine Sandy Loam & Whitman Fine Sandy Loam. As mentioned, due to numerous test holes performed within the vicinity of the project, it was determined that the boundary lines of the HSG D Soils depicted via the NRCS web soil survey were not consistent with the in-situ field work. As such, the boundary lines have been adjusted to appropriately correspond with the testing. Infiltration Basin 1 has been designed with an exfiltration rate of 1.02 inches/hour (Loam soil). Infiltration Basin 2 has been designed with an exfiltration rate of 1.02 inches/hour (Loam soil). Please refer to the attached SWMA Test Hole Data from Stamski and McNary, Inc. in Appendix C.

Recharge calculations can be found in Appendix F.

### ***2.4 Standard 4 – Water Quality***

TSS removal calculations have been provided (Appendix F) showing that the proposed TSS removal efficiency from these areas will be 80% using the infiltration basins with the sediment forebay & deep sump hooded catch basin pretreatment. Two TSS calculation sheets have been provided. The sheet with a deep sump catch basin into a sediment forebay shows proper pre-treatment before entering the infiltration basins. The sheet with deep sump catch basin into a infiltration basin shows there is enough TSS removal within the whole system.

### ***2.5 Standard 5 – Land Uses with Higher Pollutant Loads***

The current and proposed uses of the subject site do not constitute land use with higher potential pollutant load, thus Standard 5 does not apply to the proposed project.

### ***2.6 Standard 6 –Critical Areas***

The proposed project does not contain a stormwater discharge within or near to any of the areas as defined as “Critical Areas” at 314 CMR 9.02 and 310 CMR 10.04.

### ***2.7 Standard 7 – Redevelopment***

The proposed project does not meet the standards to be considered a Redevelopment project.

## ***2.8 Standard 8 – Construction Period Pollution Prevention Plan and Erosion and Sediment Control***

Since the project is subject to the filing of an Environmental Protection Agency Notice of Intent (EPA NOI), and the work will be pursuant to the NPDES Construction General Permit for disturbance to an area greater than 1 acre, a copy of the Stormwater Pollution Prevention Plan (SWPPP) will be submitted prior to construction. The SWPPP will satisfy the Standard 8 Construction Period Pollution prevention. And Erosion and Sediment Control Plan is included in the attached Site Plans.

## ***2.9 Standard 9 – Operation and Maintenance Plan***

Refer to Appendix H for a complete copy of the Stormwater Operation and Maintenance Plan.

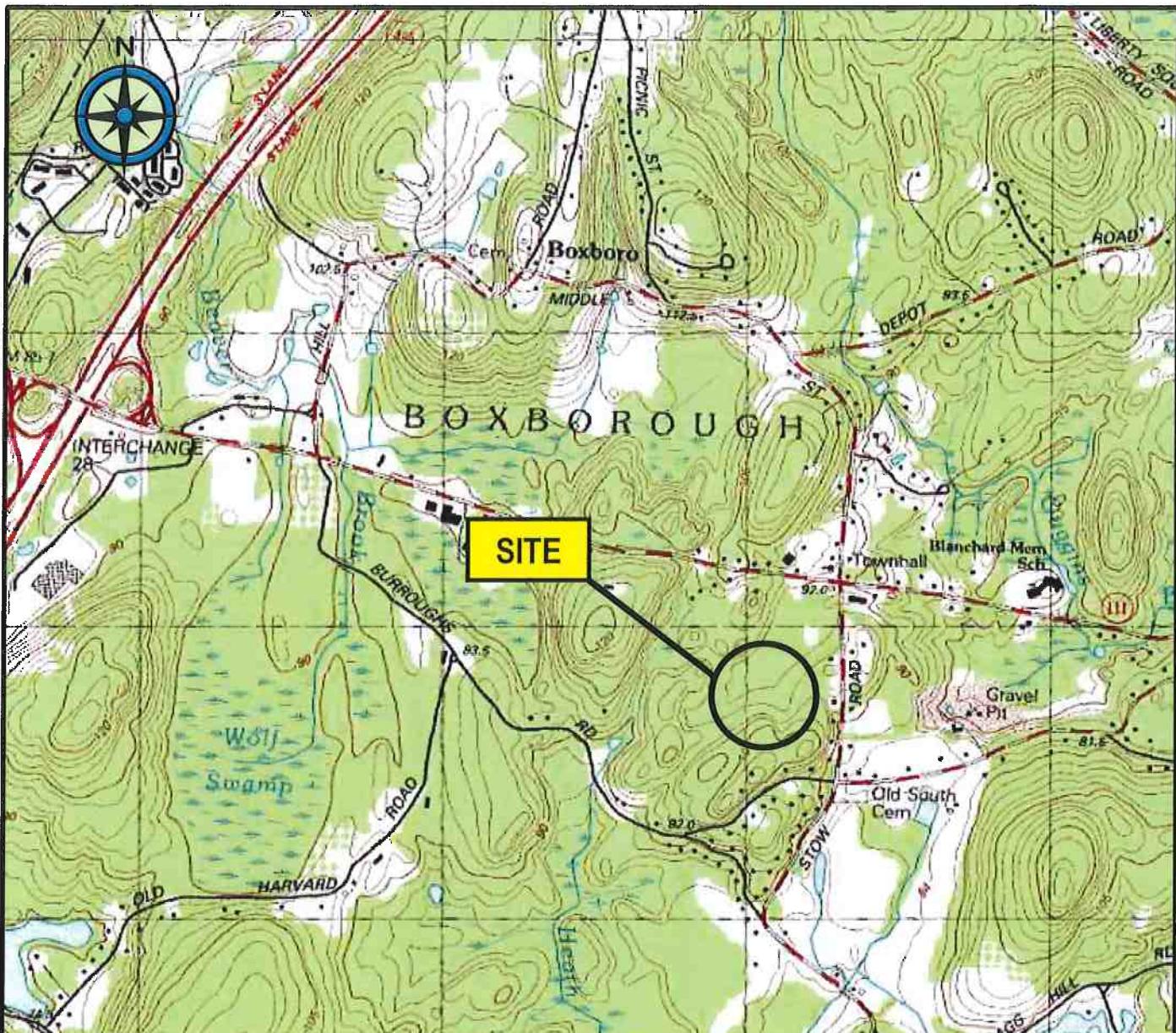
## ***2.10 Standard 10 – Prohibition of Illicit Discharge***

An illicit discharge statement will be prepared after approvals are received and prior to construction.

### **3.0 Appendices**

**Appendix A - Locus Map**

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### Locus Map

NOT TO SCALE

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**Appendix B - Checklist for Stormwater Report**

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Massachusetts Department of Environmental Protection  
Bureau of Resource Protection - Wetlands Program

# Checklist for Stormwater Report

## A. Introduction

**Important:** When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the [Massachusetts Stormwater Handbook](#). The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.<sup>1</sup> This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8<sup>2</sup>
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

<sup>1</sup> The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

<sup>2</sup> For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



# **Massachusetts Department of Environmental Protection**

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# **Checklist for Stormwater Report**

## **B. Stormwater Checklist and Certification**

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

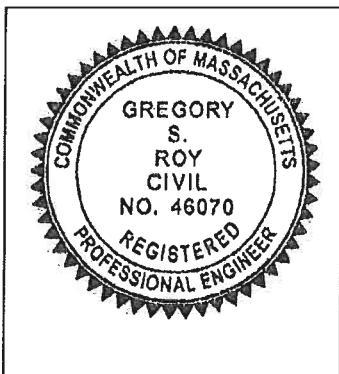
**Note:** Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

## **Registered Professional Engineer's Certification**

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

## Registered Professional Engineer Block and Signature



Signature and Date

## Checklist

**Project Type:** Is the application for new development, redevelopment, or a mix of new and redevelopment?

- New development
  - Redevelopment
  - Mix of New Development and Redevelopment



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# Checklist for Stormwater Report

### Checklist (continued)

**LID Measures:** Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- No disturbance to any Wetland Resource Areas
- Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- Reduced Impervious Area (Redevelopment Only)
- Minimizing disturbance to existing trees and shrubs
- LID Site Design Credit Requested:
  - Credit 1
  - Credit 2
  - Credit 3
- Use of "country drainage" versus curb and gutter conveyance and pipe
- Bioretention Cells (includes Rain Gardens)
- Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- Treebox Filter
- Water Quality Swale
- Grass Channel
- Green Roof
- Other (describe): \_\_\_\_\_

### Standard 1: No New Untreated Discharges

- No new untreated discharges
- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



# Checklist for Stormwater Report

## Checklist (continued)

### Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

### Standard 3: Recharge

- Soil Analysis provided.
- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.
  - Static
  - Simple Dynamic
  - Dynamic Field<sup>1</sup>
- Runoff from all impervious areas at the site discharging to the infiltration BMP.
- Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
  - Site is comprised solely of C and D soils and/or bedrock at the land surface
  - M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
  - Solid Waste Landfill pursuant to 310 CMR 19.000
  - Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

<sup>1</sup> 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



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# Checklist for Stormwater Report

## Checklist (continued)

### Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

### Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
  - Provisions for storing materials and waste products inside or under cover;
  - Vehicle washing controls;
  - Requirements for routine inspections and maintenance of stormwater BMPs;
  - Spill prevention and response plans;
  - Provisions for maintenance of lawns, gardens, and other landscaped areas;
  - Requirements for storage and use of fertilizers, herbicides, and pesticides;
  - Pet waste management provisions;
  - Provisions for operation and management of septic systems;
  - Provisions for solid waste management;
  - Snow disposal and plowing plans relative to Wetland Resource Areas;
  - Winter Road Salt and/or Sand Use and Storage restrictions;
  - Street sweeping schedules;
  - Provisions for prevention of illicit discharges to the stormwater management system;
  - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPPL;
  - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
  - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
  - Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
    - is within the Zone II or Interim Wellhead Protection Area
    - is near or to other critical areas
    - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
    - involves runoff from land uses with higher potential pollutant loads.
  - The Required Water Quality Volume is reduced through use of the LID site Design Credits.
  - Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



# Checklist for Stormwater Report

## Checklist (continued)

### Standard 4: Water Quality (continued)

- The BMP is sized (and calculations provided) based on:
  - The  $\frac{1}{2}$ " or 1" Water Quality Volume or
  - The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the proprietary BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

### Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted *prior to* the discharge of stormwater to the post-construction stormwater BMPs.
- The NPDES Multi-Sector General Permit does *not* cover the land use.
- LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- All exposure has been eliminated.
- All exposure has *not* been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

### Standard 6: Critical Areas

- The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- Critical areas and BMPs are identified in the Stormwater Report.



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# Checklist for Stormwater Report

## Checklist (continued)

### Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
- Limited Project
  - Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
  - Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
  - Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
  - Bike Path and/or Foot Path
  - Redevelopment Project
  - Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

### Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
  - Construction Period Operation and Maintenance Plan;
  - Names of Persons or Entity Responsible for Plan Compliance;
  - Construction Period Pollution Prevention Measures;
  - Erosion and Sedimentation Control Plan Drawings;
  - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
  - Vegetation Planning;
  - Site Development Plan;
  - Construction Sequencing Plan;
  - Sequencing of Erosion and Sedimentation Controls;
  - Operation and Maintenance of Erosion and Sedimentation Controls;
  - Inspection Schedule;
  - Maintenance Schedule;
  - Inspection and Maintenance Log Form.
- A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



# Checklist for Stormwater Report

## Checklist (continued)

### Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has *not* been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- The project is *not* covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

### Standard 9: Operation and Maintenance Plan

- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
  - Name of the stormwater management system owners;
  - Party responsible for operation and maintenance;
  - Schedule for implementation of routine and non-routine maintenance tasks;
  - Plan showing the location of all stormwater BMPs maintenance access areas;
  - Description and delineation of public safety features;
  - Estimated operation and maintenance budget; and
  - Operation and Maintenance Log Form.
- The responsible party is *not* the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
  - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
  - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

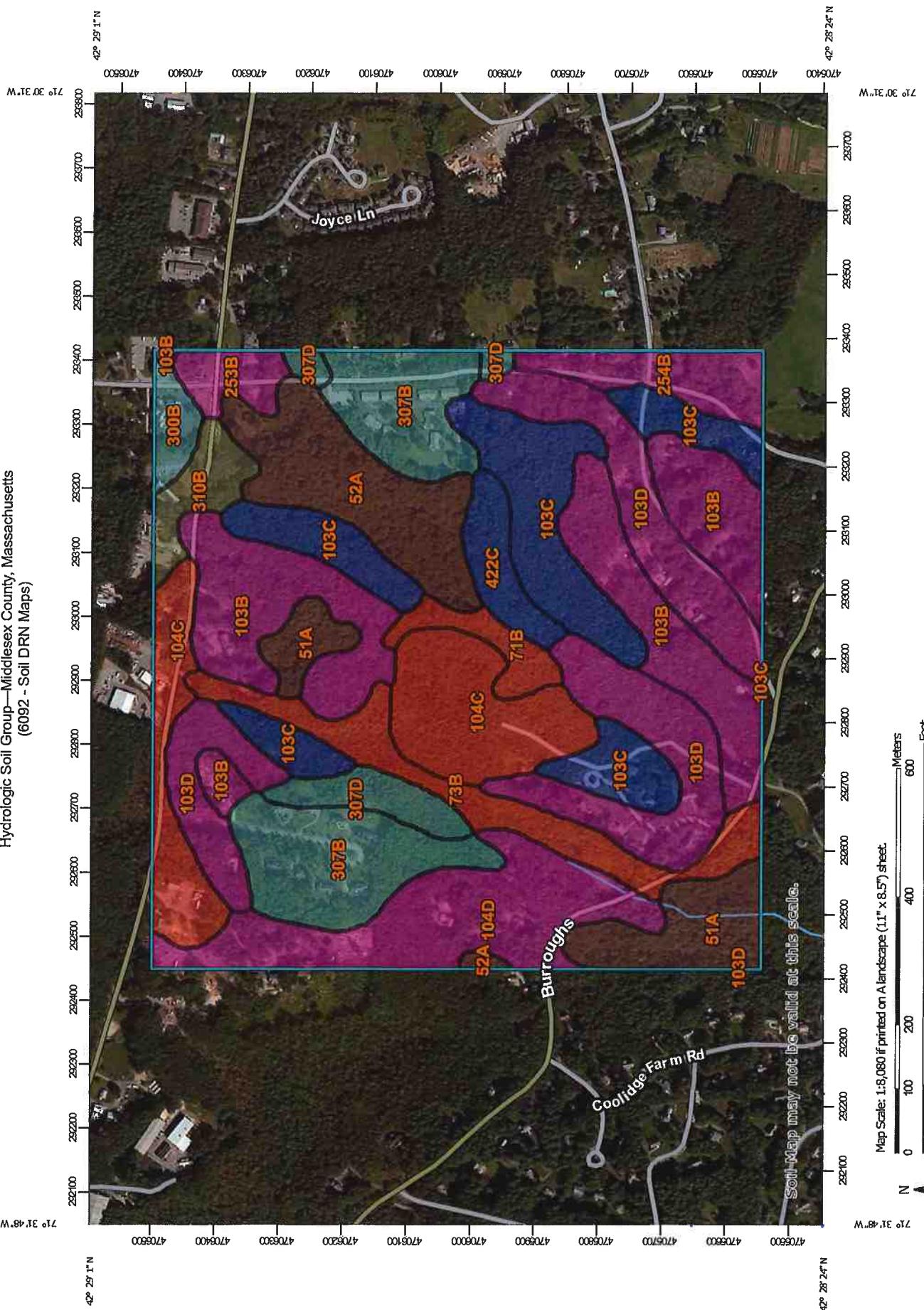
### Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted *prior to* the discharge of any stormwater to post-construction BMPs.

**Appendix C - Soils Data**

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## Hydrologic Soil Group—Middlesex County, Massachusetts (6092 - Soil DRN Maps)



Map Scale: 1:8,080 if printed on A landscape (11" x 8.5") sheet \_\_\_\_\_ Meters \_\_\_\_\_

Map projection: Web Mercator   Corner coordinates: WGS84   Edge tiles: UTM Zone 19N WGS84

Natural Resources  
Conservation Service

Web Soil Survey  
National Cooperative Soil Survey

## MAP LEGEND

Area of Interest (AOI)		C	C/D
	Area of Interest (AOI)		
Soils		D	
	A		
	A/D		
	B		
	B/D		
	C		
	C/D		
	D		
Soil Rating Polygons		Not rated or not available	
Soil Rating Lines			
Background			
Transportation			
Water Features			
Streams and Canals			
Railroads			
Interstate Highways			
US Routes			
Major Roads			
Local Roads			
Aerial Photography			
Soil Rating Points			
	A		
	A/D		
	B		
	B/D		
	C		
	C/D		
	D		
Soil Rating Polygons		Not rated or not available	
Soil Rating Lines			
Background			
Transportation			
Water Features			
Streams and Canals			
Railroads			
Interstate Highways			
US Routes			
Major Roads			
Local Roads			
Aerial Photography			
Soil Rating Points			
	A		
	A/D		
	B		
	B/D		
	C		
	C/D		
	D		

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape, but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Middlesex County, Massachusetts  
Survey Area Data: Version 18, Sep 7, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 12, 2014—Sep 28, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
51A	Swansea muck, 0 to 1 percent slopes	B/D	11.3	4.9%
52A	Freetown muck, 0 to 1 percent slopes	B/D	15.6	6.8%
71B	Ridgebury fine sandy loam, 3 to 8 percent slopes, extremely stony	D	3.7	1.6%
73B	Whitman fine sandy loam, 0 to 3 percent slopes, extremely stony	D	11.8	5.1%
103B	Charlton-Hollis-Rock outcrop complex, 3 to 8 percent slopes	A	35.3	15.4%
103C	Charlton-Hollis-Rock outcrop complex, 8 to 15 percent slopes	B	27.8	12.1%
103D	Charlton-Hollis-Rock outcrop complex, 15 to 25 percent slopes	A	31.5	13.8%
104C	Hollis-Rock outcrop-Charlton complex, 0 to 15 percent slopes	D	21.7	9.5%
104D	Hollis-Rock outcrop-Charlton complex, 15 to 25 percent slopes	A	20.1	8.8%
253B	Hinckley loamy sand, 3 to 8 percent slopes	A	3.5	1.5%
254B	Merrimac fine sandy loam, 3 to 8 percent slopes	A	6.6	2.9%
300B	Montauk fine sandy loam, 3 to 8 percent slopes	C	2.7	1.2%
307B	Paxton fine sandy loam, 0 to 8 percent slopes, extremely stony	C	22.6	9.9%
307D	Paxton fine sandy loam, 15 to 25 percent slopes, extremely stony	C	5.6	2.4%
310B	Woodbridge fine sandy loam, 3 to 8 percent slopes	C/D	4.5	2.0%

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
422C	Canton fine sandy loam, 8 to 15 percent slopes, extremely stony	B	4.7	2.0%
<b>Totals for Area of Interest</b>			<b>228.9</b>	<b>100.0%</b>

## Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

**Group A.** Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

**Group B.** Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

**Group C.** Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

**Group D.** Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

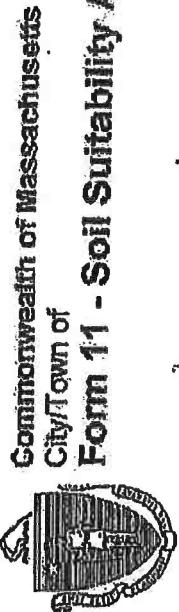
## Rating Options

*Aggregation Method:* Dominant Condition

*Component Percent Cutoff:* None Specified

*Tie-break Rule:* Higher





STAMSKI AND MCNARY, INC.  
1000 Main Street  
ACTION, MA 01720  
Engineering • Planning • Surveying

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

DEP has provided this form for use by on-site professionals and local Boards of Health. Other forms may be used, but the information must be substantially the same as provided here. Before using this form, check with your local Board of Health to determine the form they use.

### A. Facility Information

#### 1. Facility Information BROOKSBRIDGE TOWN CENTER, LLC

Owner Name \_\_\_\_\_  
Street Address 4270 N \_\_\_\_\_  
City/Town Acton \_\_\_\_\_  
State \_\_\_\_\_  
Map/Lot \_\_\_\_\_  
Zip Code \_\_\_\_\_

Storm Water  
Sanitary On-Site  
Abolates 10-15  
9/21/2015 10-16-31  
9/30/2016 16-35 11-56

### B. Site Information

1. (Check one) New Construction  Upgrade  Repair

2. Published Soil Survey available? Yes  No  If yes: \_\_\_\_\_

Year Published \_\_\_\_\_ Publication Scale \_\_\_\_\_ Soil Map Unit \_\_\_\_\_

Soil Name \_\_\_\_\_ Soil Limitations \_\_\_\_\_

3. Surficial Geological Report available? Yes  No  If yes: \_\_\_\_\_

Year Published \_\_\_\_\_ Publication Scale \_\_\_\_\_ Map Unit \_\_\_\_\_

Geologic Material \_\_\_\_\_

Landform \_\_\_\_\_

### 4. Flood Rate Insurance Map:

Above the 500 year flood boundary? Yes  No   
Within the 500 year flood boundary? Yes  No  Within a Velocity Zone? Yes  No

5. Wetland Area: National Wetland Inventory Map  
Wetlands - WETLANDS CONFLINED ON SITE  
Map Unit \_\_\_\_\_ Name \_\_\_\_\_



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN COMMISSIONERS  
100 & 1800 MARYLAND AVENUE, AVE

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-1      Depth: 9'      Time: 20

Overcast 70°  
Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) Within SWIZ

## 2. Land Use:

(e.g. woodland, agricultural field, vacant lot, etc.)  
PINE, HICKORYWOOD

Surface Stones

Depth (ft)

Inches

Position on landscape (check one)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area 200' \_\_\_\_\_  
Property Line 100' \_\_\_\_\_ Drinking Water Well 60' \_\_\_\_\_ Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL      Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from PI: \_\_\_\_\_ Depth Standing Water in Hole: \_\_\_\_\_

Estimated Depth to High Groundwater: 60'

Deep Observation Hole Number: 16-1

Depth (in.)	Soil Horizons/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (moisture)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
4	A	10YR 3/2	—	—	—	SL	—	—	M	F	20% GLEYED
20	B	10YR 5/6	—	—	—	SL	—	—	M	R	
46	C <sub>1</sub>	2.5Y 7/4	60	5YR 5/8	75	LS	20	M	F	Z	
170	C <sub>2</sub>	2.5Y 6/5				SL	55	M	F	GRANULAR	

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HUMMINGBIRD, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN CENTER, LLC.  
700-1 BODD MANUFACTURING AVE

STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-2 9/20 2014  
Date TimeOrientation 30°  
Wester

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use:

WILDLAND  
(e.g. woodland, agricultural field, vacant lot, etc.)  
MAPLE, PINE  
Vegetation Landform Surface Slope Surface Slope  
Position on landscape (slope sheet)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area > 100  
Property Line foot foot foot  
Drinking Water Well foot Other \_\_\_\_\_

4. Parent Material: GLACIATE TILL Unsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 30'

Deep Observation Hole Number: 16-2

Depth (in)	Soil Horizon/ Layer	Soil Matrix/ Color-Moist (Munsell)	Redoximorphic Features (moisture)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Porosity		Grovel	Cobbles & Boulders			
26	A <sub>1</sub> FILL	FILL	-	-	-	SL	-	10%	M	F	BRADLEY RESISTANT
30	B <sub>1</sub> SIL	10YR 5/6	-	-	-	SL	-	-	M	F	
48	C <sub>1</sub> 6/4	2.5Y 6/4	30	5YR 5/8	>5	SL	-	5%	M	F	ZO BLDRS
120	C <sub>2</sub> 6/3	2.5Y 6/3	-	-	-	SL	-	-	M	F	

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNAH, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN CEMETERY, LLC.

704-1, 800 MULBERRY ST., AVE.

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number 16-3 Date 9/20/2014 Time Overcast Weather 70°

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope (%)

PINE, HANOVER

Landform

Position on Landform (e.g. slope)

## 3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area &gt; 100

Property Line \_\_\_\_\_

Drinking Water Well \_\_\_\_\_

Other \_\_\_\_\_

feet

feet

feet

4. Parent Material: GLACIAL TILLUnsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 6.0"Deep Observation Hole Number: 16-3

Depth (in)	Soil Horizon/ Layer	Soil Matrix/ Color-Matrix (Munsell)	Redoximorphic Features (metiles)			Soil Texture (USDA)	Coarse Fragments % by Volume	Soil Structure	Soil Conductance (Metres)	Other
			Depth	Color	Percent					
6	A	10YR 4/2 3/2	--	--	--	SL	--	M	F	LARGE BOULDERS
22	B	10YR 5/2 5/2	--	--	--	SL	--	M	F	
60	C <sub>1</sub>	2.5Y 6/2	60	5YR 5/2	>5	SL	5%	M	F	
96	C <sub>2</sub>	2.5Y 6/3				SL		M	P	
		(BOULDERS)								

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNAH, P.E., MA SE 1012



City/Town of  
Form 11. Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN CENTER, LLC.  
700 1/2 BDO NEWTON STREET, MA

STAMSKI AND McNARY, INC.  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-4 Date: 9/20/2014 Time: Overset 7:00  
Weather:

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes \_\_\_\_\_

Slope (%) \_\_\_\_\_

HARDWOODS

Vegetation

Landform

Position on Landscape (slope sheet)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area > 100' 120' 150' \_\_\_\_\_

ft

ft

ft

ft

ft

ft

4. Parent Material: GLACIAL TILL

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 55

Deep Observation Hole Number: 16-4

Depth (in)	Soil Horizon Layer	Soil Matrix Color-Moist (Munsell)	Redoximorphic Features (metres)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Mohs)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
4	A	10YR 3/2	—	—	—	SL	—	55	M	F	LARGE Boulders
20	B	10YR 5/1	—	—	—	SL	—	—	M	F	
120	C	2.5Y 4/3	55	5YR 5/8	>5	LS	2G	5C	M	F	Boulders

Additional Notes: STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINEN, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN CENTRAL, LLC  
700 & 800 MARYLAND AVENUE

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-S 9/20 2014 Date Time Weather Overcast 70°

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND (e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes Slope (%)

Vegetation	Landform	Position on land slope (North, South, East, West)
------------	----------	---

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area >100 feet  
Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 48 \_\_\_\_\_

Deep Observation Hole Number: 16-5

Depth (in)	Soil Horizon/Layer	Soil Matrix Color-Moist (Munsell)	Radiximorphic Features (moist)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
5	A	10YR 3/2	-	-	-	SL	-	-	M	F	20 BOULDERS
22	B	10YR 5/6	-	-	-	SL	-	-	M	F	LARGE
36	C <sub>1</sub>	2.5Y 7/6	-	-	-	LS	-	5C	M	F	
120	C <sub>2</sub>	2.5Y 6/3	40	5YR 5/6	>5	SL	3G	10S	M	F	
							2C				

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. MANNAIONI, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBURY TOWNSHIP CENTRAL, LLC  
700 1/2 BDO MARYLAND ST., ALEXSTAMSKI AND McNARY, INC.  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-6 Date: 9/20/14 Time: OVERCAST 70°  
Weather:

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND  
(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation Landform Position on Landscape (block sheet)

3. Distances from: Open Water Body (ft) Draining Way (ft) Possible Wet Area (ft) >100  
Property Line (ft) Drinking Water Well (ft) Other \_\_\_\_\_4. Parent Material: GLACIAL TILL Unstable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pt. \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 50"

Deep Observation Hole Number: 16-6

Depth (in.)	Soil Horizon Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (moisture)			Soil Texture (USDA)	Degree Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other				
			Depth	Color	Percent		Gravel	Dobbles & Stones							
5	A	10YR 3/2	-	-	+	SL	-	M	F	LARGE BAULDERS					
21	B	10YR 5/6	--	--	--	SL	-	M	F						
44	C <sub>1</sub>	2.5Y 7/4	--	--	-	LS	6S 2C	M	F						
130	C <sub>2</sub>	2.5Y 6/3	SD	5YR 5/8	>5	SL	3G	M	F	LARGE BAULDERS					

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINGTON, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Buxborough TOWN CENTER, LLC

700 1/800 MARYLAND AVENUE, AVE

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-7 9/20/2014 Date Time Overcast Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND (e.g. woodland, agricultural field, vacant lot, etc.) Slope (%) Surface Slopes

Vegetation Landform Position on Landscape (slope show) 3. Distances from: Open Water Body foot Possible Wet Area &gt; 100 Drainage Way foot Property Line foot Drinking Water Well foot Others \_\_\_\_\_

4. Parent Material: GLACIUM TILL Unsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pt: \_\_\_\_\_ Depth Standing Water in Hole: \_\_\_\_\_

Estimated Depth to High Groundwater: 50 \_\_\_\_\_

Deep Observation Hole Number: 16-7

Depth (in)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (inches)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Consist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
4	A	10YR 3/2	-	-	-	SL	-	-	M	F	LARGE Boulders
18	B	10YR 5/6	-	-	-	SL	-	-	M	F	Boulders
102	C	2.5Y 6/3	50	5YR 5/8	>5	LS	36	2 C SS	M	F	GRANULAR Boulders
102	R										

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINGTON, P.E., MA SE 1012



City/Town of  
Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN COUNCIL, LLC  
700 4 Boro Mfg. Center, MA 01720

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-B 9/20 2014      Date      Time      Orientation: N 70°  
Weather:

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes      Slope (%)

Vegetation      Condition      Position on landscape (pitch sheet)

3. Distances from: Open Water Body      Drainage Way      Possible Wet Area >100

Property Line      feet      feet      feet

Drinking Water Well      feet      feet      Other \_\_\_\_\_

4. Parent Material: GLENVILLE TILL

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Freccured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pt. \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 5 ft

Deep Observation Hole Number: 16-B

Depth (in)	Soil Horizon/Layer	Soil Matrix; Color-Moist (moisture)	Redoximorphic Features (moisture)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Note)	Other
			Depth	Color	Percent		Grovel	Cobbles & Stones			
4	A	10YL 3/2	-	-	-	SL			M	F	
18	B	10YL 5/6	-	-	-	SL			M	F	
108	C	2.5Y 54	54	5YR 5/8	>5	LS	SG	2C 2S	M	very friable	
108	R					poorly drained sand					

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANLICKER, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN CENTER, LLC

700 &amp; 800 MARYACHIN'S AVE

STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-9 Date: 9/20 Time: 2pm Weather: Overcast 70°

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Subsoil/Sediments

Expo (%)

Vegetation

Landform

Position on landscape (check one)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area &gt; 100

Property Line \_\_\_\_\_

Foot

Foot

Drinking Water Well \_\_\_\_\_

Foot

Foot

Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL Unusable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 44

Deep Observation Hole Number: 16-9

Depth (in)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (notches)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Notes)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
4	A	10YR 5/2	-	-	-	SL	-	-	M	F	
23	B	10YR 5/6	-	-	-	SL	-	-	M	F	LAWNS 100% GRASS
108	C	2.5Y 4/3	44			LS	5G	5S 2C	M	F	

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANLICKER, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBURY TOWN CENTER, LLC.

100 BUD MCKEEAN STREET, MA

C. On-Site Review

STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

Deep Observation Hole Number: 16-10 Date: 9/20/2014

Time: 7:00

Weather:

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use:

WOODLAND

(e.g. woodland, agriculture field, vacant lot, etc.)

Surface Slope(s)

Slope (%)

Vegetation

Landscape

Position on landscape (high, mid, low)

3. Distances from: Open Water Body (ft) Drainage Way (ft) Possible Well Area (ft) &gt; 100

Property Line (ft)

Drinking Water Well (ft)

Other (ft)

4. Parent Material: GLACIAL TILL Unstable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 53

Deep Observation Hole Number: 16-10

Depth (in)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (metres)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consolidation (Mohr)	Other				
			Depth	Color	Percent		Gravel	Cobbles & Slentes							
4	A	10 1/2 3 1/2	—	—	—	SL			M	F	Lignite				
19	B	10 1/2 5 1/2	—	—	—	GL			M	F	Iron pyrite				
94	C	2.54 6 1/3	5 3	5 1/2 5 1/8	75	LS	5G	2C SS	M	F					
94	R														

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANLINGTON, P.E., MA SE 1012

311M DEANE 892 EXAMINER



City/Town of  
Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Buxbury Town Center, LLC.

Two & 800 Mission St., Inc.

G. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-11 Date: 9/20/2014 Time: Overcast 70° Weather:

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slope

Slope (W)

Vegetation

Landform

Position on landscape (check sheet)

3. Distances from: Open Water Body foot Drainage Way foot Possible Wet Area foot >100

Property Line foot Drinking Water Well foot Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pt. \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 60

Deep Observation Hole Number: 16-11

Depth (in)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (moist)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
4	A	10YL 312	-	-	-	SL	-	-	M	F	LARGE
16	B	10YL 51V	-	-	-	SL	-	-	M	F	BOULDERS
100	C	2-5Y 613	60	5YL S18	>5	LS	5G 2-5	5C 2-5	M	F	very fragile
100	R										

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. McNARY, P.E., MA SE 101Z



City/Town of  
Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWNSHIP, LLC.  
700 & 800 MASTERS INN SKYLINE AVE.

STANISKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-12 Date: 9/20/2014 Time: OVERCAST Weather:

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation \_\_\_\_\_ Landform \_\_\_\_\_ Position on landscape (which slope) \_\_\_\_\_

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area >100 foot  
Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 4.4

Deep Observation Hole Number: 16-12

Depth (in.)	Soil Horizon/ Layer	Soil Matrix; Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Wet)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
3	A	10YR 3/2	-	-	-	SL	-	-	M	F	
15	B	10YR 5/6	-	-	-	SL	-	-	M	F	
72	C	2.5Y 6/3	44	5YR 5/8	>5	LS	5g	2C 2S	M	Very Firm	
72	R										

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINGTON, P.E., MA SE 101Z



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN CENTER, LLC.

700 L 800 MARYANCHICKA RD., AVE.

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-13 9' 20' 2011 Date Time Overall 70° Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Elev. (ft)

Vegetation

Landform

Position on Landform (Rock shelf),

## 3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area &gt;100

foot

foot

foot

Property Line

Drinking Water Well

Other

foot

foot

4. Parent Material: GLACIAL TILL Unsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 40

Deep Observation Hole Number: 16-13

Depth (in)	Soil Horizon/ Layer	Soil Matrix: Color/Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
3	A	10 YR 3/2	-	-	-	SL	-	-	M	F	LARGE
16	B	10 YR 5/6	-	-	-	SL	-	-	M	F	SCATTERED
34	C	2.5Y 4/3	40	5YR 5/6	>5	LS	-	2C 2S	M	F	
34	R										

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNAH, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN CENTER, LLC

Job # BOD MATTAGAMICETT AVE

**STAMSKI AND McNARY, INC.**  
 1000 Main Street  
 ACTON, MA 01720  
 Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-14 9/20/2011 Date      Overcast Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Soil Slopes

Slope (%)

Vegetation      Condition      Position on Landscape (ditch, slope)

3. Distances from: Open Water Body \_\_\_\_\_ feet      Drainage Way \_\_\_\_\_ feet      Possible Wet Area &gt; 100 feet

Property Line \_\_\_\_\_ feet      Drinking Water Well \_\_\_\_\_ feet      Other \_\_\_\_\_ feet

4. Parent Material: COLAVCAZ TILL Unstable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from PII \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: \_\_\_\_\_

Deep Observation Hole Number: 16-14

Depth (in)	Soil Horizon Layer	Soil Matrix: Color-Matrix (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fractions % by Volume		Soil Structure	Soil Consistency (Mohr)	Other
			Depth	Color	Percent		Gravel	Dobbles & Stones			
5	A	10YR 3/2	—	—	—	SL	—	—	M	F	LARGE PARTICLES
14	B	10YR 5/6	—	—	—	SL	—	—	M	F	
106	C	2.5Y 4/3	42	5YR 5/8	>5	LS	—	3C 2.S	M	F	BLDRS
106	R										

Additional Notes: STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNAH, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Buckborough TOWN CENTER, LLC  
700 L. 800 MARYANNE STREET, AVE

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 6-15 9/20 2014 Date Time OVERCAST 70° Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes Slope (%)

Vegetation Landform Position on Landscape (Slope shown)

3. Distances from: Open Water Body foot Drainage Way foot Possible Wet Area > 100 foot  
Property Line foot Drinking Water Well foot Other \_\_\_\_\_4. Parent Material: GLACIAL TILL Unsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Weathered Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 42 \_\_\_\_\_

Deep Observation Hole Number: 16-15

Depth (in)	Soil Horizon/ Layer	Soil Matrix; Color-Moist (Munsell)	Redoximorphic Features (moisture)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
4	A	10YR 3/2	—	—	—	SL	—	—	M	F	LARGE
15	B	10YR 5/6	—	—	—	SL	—	—	M	F	SWIMMERS
10 <sup>9</sup>	C	2.5Y 41/2	42	5YR 5/8	>5	LS	—	2S 2C	M	F	
10 <sup>9</sup>	R	L									

Additional Notes

STORM-WATER SOIL OBSERVATIONS

RICHARD J. HANNIBAL, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN CENTER, LLC  
700 + 800 MASSACHUSETTS AVE

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-169212016  
Date: \_\_\_\_\_ Time: \_\_\_\_\_SM RU  
Wenber

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope (%)

Vegetation	Landform	Position on landscape (inches above)
Open Water Body	Drainage Way	Possible Wet Area
Property Line	Drinking Water Well	foot
feet	feet	feet
Other		

4. Parent Material: GLACIAL TILL Unusable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from PH \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 40Deep Observation Hole Number: 16-169

Depth (in.)	Soil Horizon/ Layer	Soil Matrix/ Color/Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
7	A	10YL 3/2	-	-	-	SL	-	-	M	F	23 BLDR
20	B	10YL 5/6	-	-	-	SL	-	-	M	F	
100	C	2.5Y 4/4	40	5YL 5/8	>5	LS	-	SC	M	F	
		Boulders									

Additional Notes

STORM-WATER SOIL OBSERVATIONS

RICHTERS HAMPTON, P.E., MA SE 1017



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Buxbury Town Center, LLC

700 &amp; 800 Miss. Ave.

## STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-17-21-204 Date: Sun 4/0 Weather:

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope (%)

Vegetation

Landform

Position on landscape (Wet/dry)

3. Distances from: Open Water Body \_\_\_\_\_ foot Drainage Way \_\_\_\_\_ foot Possible Wet Area >100 foot  
Property Line \_\_\_\_\_ foot Drinking Water Well \_\_\_\_\_ foot Other \_\_\_\_\_ foot4. Parent Material: GLACIAL TILL Unstable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 24 \_\_\_\_\_

Deep Observation Hole Number: 16-17

Depth (in)	Soil Horizons Layer	Soil Matrix Color-Moist (Munsell)	Redoximorphic Features (moist)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Mohs)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
6	A	brown 3R	-	-	-	SL	-	-	M	F	
15	B	brown 5/6	-	-	-	SL	-	-	M	P	
50	C <sub>1</sub>	2.5Y 4/4	24	5YR 5/6	>10	LS	-	-	M	F	
100	C <sub>2</sub>	2.5Y 1/3				SL	-	-	M	F	
		No refusal									

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. THOMAS, P.E., MA SE 101Z



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXTONBROUGH TOWN CENTER, LLC.  
100-1800 MASSACHUSETTS, AND

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-18 9 2 | 2-14, Date: 5/20, Time: 9:00 AM, Weather:

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND  
(e.g. woodland, agricultural field, vacant lot, etc.) Surface Stones Slope (%)3. Distances from: Open Water Body foot Drainage Way foot Possible Wet Area foot Position on landscape (inches above)  
Property Line foot Drinking Water Well foot Other \_\_\_\_\_4. Parent Material: GRANULITE TILL Unsuitable Materials Present; Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: \_\_\_\_\_

Deep Observation Hole Number: 16-18

Depth (in)	Soil Horizon/ Layer	Soil Matrix; Color-Moist (Munsell)	Redoximorphic Features (moisture)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Gonbles			
5	A	10YR 3/1	—	—	—	SL	—	—	M	F	LO SLDR
16	B	10YR 7/2	—	—	—	SL	—	—	M	F	
55	C <sub>1</sub>	2.5Y 6/3	18"	High 2H Brown	20%	LS	Pockers SAND	—	M	F	
96	C <sub>2</sub>	2.5Y 6/4				SL	2C SS				
		NO REACTION									

Additional Notes: STORMWATER SOIL OBSERVATIONS  
RICHARD J. HAWLEY, P.E., MA SE 1017



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUSCEBLOWA H TOWNSHIP CENTER, LLC.  
700 & 800 MUNICIPALITY AVE

## STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-19 | Z1 | Z14 | Sun | Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND Surface Slopes \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation	Landform	Position on land slope (Rock sheet)
------------	----------	-------------------------------------

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area >100 foot  
Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

4. Parent Material: GLENWELL TILL Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 24

Deep Observation Hole Number: 16-19

Depth (in.)	Soil Horizon/ Layer	Soil Matrix Color-Moist (Munsell)	Redoximorphic Features (metres)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
6	A	10YL 31Y	-	-	-	SL	-	-	M	F	Lo ST(MO)
14	B	10YL 31Y	-	-	-	SL	-	-	M	F	
30	C1	2.5Y 31Y	Z4	5YL 51Y	>20	LS	-	-	M	F	
100	C2	2.5Y 31Y	Z12			LS	Pebbles FINE GRAINED	M	M	F	
	NO	REFUSAL									

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINGTON P.E., MA SE 1012



Commonwealth of Massachusetts

City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXTONBURY TOWN CENTRE, LLC.

700-1800 MARYLAND STREET, ALEX

STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16 → 00 9 | 2 | 2014 Date

Sun 40  
Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope (%)

Vegetation

Landform

Position on Landform (Sketch Area)

## 3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area &gt; 100' foot

Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

foot

foot

foot

4. Parent Material: Glacial Till Unstable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractioned Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from PH \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 76 \_\_\_\_\_

Deep Observation Hole Number: 16 → 00

Depth (in)	Soil Horizon/Layer	Soil Matrix: Color-Model (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Mohr)	Other
			Depth	Color	Percent		Gravel	Dobbles & Stomos			
5	A	10YR 3/4	—	—	—	SL	—	—	M	P	Z BLDRS
16	B	10YR 5/6	—	—	—	SL	—	—	M	F	
60	C <sub>1</sub>	2.5Y 6/4	—	—	—	LS	2G	5S 2C	M	F	
76	C <sub>2</sub>	2.5Y 5/4	—	—	—	GRANULAR SAND	15G	2C	M	VERY FIRM	
120	C <sub>3</sub>	2.5Y 6/2	76	5YR 5/8	>5	SL	—	—			

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINGTON, P.E., MA SE 1012



COMMONWEALTH OF MASSACHUSETTS

City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Buxtonborough Town Central, LLC.

100 &amp; 800 Main Street, Acton, MA

STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-Z1 9 Z1 2016 Date

Sun 80 Time Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND (e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes Slope (%)

Vegetation Condition Position on landscape (elevation)

3. Distances from: Open Water Body foot Drainage Way foot Possible Wet Area 200 foot  
Property Line foot Drinking Water Well foot Other \_\_\_\_\_4. Parent Material: GLACIAL TILL Unsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from PII \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 20 \_\_\_\_\_

Deep Observation Hole Number: 16-Z1

Depth (in.)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Refractimorphia Features (moist)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
4	A	10YR 3/2	-	-	-	SL			M	F	10 BLDRS
16	B	10YR 5/6				SL			M	F	
76	C	2.5Y 6/9	20	5YR 5/6	>5	LS	30C 10S		M		
		NO REFUSAL									

Additional Notes

STORMWATER SITE OBSERVATIONS

RICHTER J. HANNING, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BROOKLYN TOWN CENTRE, LLC

700 1800 MARYLAND AVENUE, ALEX

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-20921 Date: 2014

Time: 8:00  
Weather:

## STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope (%)

Vegetation

Landform

Position on landscape (back slope)

3. Distances from: Open Water Body feet Drainage Way feet Possible Wet Area feet

Property Line feet Drinking Water Well feet Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL Unstable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 24 \_\_\_\_\_

Deep Observation Hole Number: 16-22

Depth (in)	Soil Horizon Layer	Soil Matrix; Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stumps			
7	A <sub>1</sub>	10YL 312	—	—	—	SL	—	—	M	F	BROWN
18	A <sub>2</sub>	10YL 516	—	—	—	SL	—	—	M	F	
120	C	2.5Y 4/4	24	5YL 518	>5	LS	ZG	10 <sup>c</sup> ZS	M	F	

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINGTON, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BRYCEBROOKS (TOWN) CENTER, LLC.  
700 + 800 MARYACHUSETTS AVE

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-239 Z1 Soil 4 Sm 90  
Date: \_\_\_\_\_ Time: \_\_\_\_\_ Weather: \_\_\_\_\_

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND  
(e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes Slope (5%)

Vegetation Landform Position on Landscape (attach sketch)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area >100  
Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 24

Deep Observation Hole Number: 16-23

Depth (in)	Soil Horizon/Layer	Soil Matrix/Color-Moist (Munsell)	Redoximorphic Features (oxiholes)			Soil Texture (USDA)	Coarse Fractions % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Precipitate		Gravel	Cobbles & Stones			
6	A	12YL 3/2	—	+	—	SL	—	—	M	F	20 BLDR
16	B	10Y 5/1	—	—	—	SL	—	—	M	F	
84	C	2.5Y 7/2 24	5YR 5/8	>10	LS FINE	SC	—	—	M	F	BLDR
		NO REFUSAL									

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINGTON, P.E., MA SE 1012



COMMONWEALTH OF MASSACHUSETTS

City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BEDFORD TOWN CENTER, LLC

100 BOSTON AVENUE, ACTON, MA 01720

C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-24921 Date: 2014 Time: SM 90 Weather:

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation \_\_\_\_\_ Landform \_\_\_\_\_ Position on Landscape (attach sheet) \_\_\_\_\_

## 3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area &gt;100 foot

Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

## 4. Parent Material: Glaciated TILL

Unsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from PI \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 18 \_\_\_\_\_

Deep Observation Hole Number: 16-249

Depth (in)	Soil Horizon/Layer	Soil Matrix: Color/Molal (Munsell)	Radiximorphic Features (moisture)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Molal)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
3	A	tan 3/1	—	—	—	SL			M	F	LARGE BLOCKS
12	B	tan R 4/2	—	—	—	SL			M	F	
60	C <sub>1</sub>	2.5Y 6/2	18	high wet	>10	LS	Pebbles GRANUL.	4%	M	F	BWZL
130	C <sub>2</sub>			constant		SL					

Additional Notes:

STORM WATER SOIL OBSERVATIONS

RICHARD J. McNARY, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BEDFORD TOWN CENTER, LLC  
700 & 800 MASSACHUSETTS AVE

## STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-259 Date: 21 Time: 2014 Weather: Sun 40

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND (e.g. woodland, agricultural field, vacant lot, etc.) Surface Slope \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation \_\_\_\_\_ Condition \_\_\_\_\_ Position on landscape (attach sheet) \_\_\_\_\_

3. Distances from: Open Water Body \_\_\_\_\_ foot Drainage Way \_\_\_\_\_ foot Possible Wet Area >100 foot  
Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ foot Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 26

Deep Observation Hole Number: 16-25

Depth (in)	Soil Horizon/ Layer	Soil Matrix Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Mohs)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
16	A	10YR 312	—	—	—	SL	—	—	M	F	LARGE Boulders
24	B	10YR 6/4	—	—	—	SL	—	—	M	F	
84	C	2.5Y 1/3	26	Low Oxidants	>5	LS	—	SS	M	F	BLW

Additional Notes

STORM WATER SOIL OBSERVATIONS

RICHARD J. HANNINING, P.E., MA SE 1017



COMMONWEALTH OF MASSACHUSETTS

City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBURYLAND TRUST COMPANY, LLC

700-1 BOSTON MASSACHUSETTS, MA 02116

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-26912 | Date: 7/16 | Time:

SM 80  
Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope (%)

Vegetation

Landform

Position on Landform (back, side, front)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area &gt;100

Property Line \_\_\_\_\_ foot      Drinking Water Well \_\_\_\_\_ foot      Other \_\_\_\_\_ foot

## 4. Parent Material: GLENWELL TUL

Unsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 40

Deep Observation Hole Number: 16-26

Depth (in)	Soil Horizon/Layer	Soil Matrix: Color-Texture (Munsell)	Redoximorphic Features (mettles)			Soil Texture (USDA)	Cobbles/Fragments % by Volume		Soil Structure	Soil Consistence (Mold)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
3	A	10YR 3/2	—	—	—	SL	—	—	M	R	
15	B	10YR 5/6	—	—	—	SL	—	—	M	F	
100	C	2.5Y 6/4	40	STR 5/8	25	LS	—	10S	M	F	
100	R										

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HAMILTON, P.E., MA SE 101Z



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BENGBURG TOWN CENTER, LLC  
700 & 800 MASSACHUSETTS AVE

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-27-9 | 21 | 2014

Date

Time

Sun 90

Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slope

Slope (%)

Vegetation

Landform

Position on Landscape (Hill, Valley)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area &gt;100 feet

Property Line

feet

Drinking Water Well

feet

feet

Other \_\_\_\_\_

4. Parent Material: Glacial Till Unstable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 4 ft

Deep Observation Hole Number: 16-27

Depth (in)	Soil Horizon/ Layer	Soil Matrix/ Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Inital)	Other				
			Depth	Color	Percent		Gravel	Cobbles & Stones							
3	A	10YR 31Z	-	-	-	SL	-	-	M	F	20 BLDR				
14	B	20YR 51G	-	-	-	SL	-	-	M	F					
63	C	2.5Y 41Y	40	5YR 51G	75	LS	-	-	M	F					
63	R														

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINGTON, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN COUNCIL, LLC  
1000 Main Street, Acton, MA 01720

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-289 / Z1 Date 2-14 Time 8:00  
Weather Sun

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND  
(e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation \_\_\_\_\_ Landform \_\_\_\_\_ Position on landscape (elevation) \_\_\_\_\_

3. Distances from: Open Water Body \_\_\_\_\_ Dripping Way \_\_\_\_\_ Possible Wet Area >100' (ft)  
Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from PI \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 40

Deep Observation Hole Number: 16-28

Depth (in)	Soil Horizon/ Layer	Soil Matrix: Color-Motol (Munsell)	Redox/Morpho Features (motiles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Conductance (Motol)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stonies			
6	A	10YL 3/2	—	—	—	SL	—	—	M	F	20 below
12	B	10YL 5/6	—	—	—	SL	—	—	M	F	
40	C <sub>1</sub>	2.5Y 6/4	—	—	—	LS	—	—	M	F	
84	C <sub>2</sub>	2.5Y 6/2	40	5YL 5/8	>5	FINE SAND	PELLETS GRAVEL		M	VERY PRIMER	
108	C <sub>3</sub>					LS			M	F	

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNIKAINEN, P.E., MA SE 1012





City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BRYCEBOROUGH TOWN CENTER, LLC  
700 A BLDG MARYWOOD CENTER, AVE

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-309 Date 21 Time 2014 Weather Sun

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND

(e.g. Woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope (%)

Vegetation

Elevation

Position on Landscape (elevated/level)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area >100  
 Property Line foot Drinking Water Well foot Other foot

4. Parent Material: GLACIAL TILLUnsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Freatured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 38Deep Observation Hole Number: 16-30

Depth (in)	Soil Horizon/ Layer	Soil Matrix/ Color-Moist (Aunsold)	Redoximorphic Features (moisture)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
6	A	10YL 31Z	—	—	—	SL	—	—	M	F	20 Boulders
14	B	10YL 51Z	—	—	—	SL	—	—	M	F	
76	C <sub>1</sub>	2.5Y 61Z	30 <sup>+</sup>	5YL 5/E	75	LS	—	—	M	F	20 GLOR.
1070	C <sub>2</sub>	2.5Y 41Z				LS	SG	10 S	M	F	

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANWINTING, P.E., M.A.S.E. 1012



City/Town of  
Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Bucksborough Town Center, LLC.  
700 A Ave., Wiscasset, ME 04578

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-31 Date: 9/21/2016 Time: Sun 9:00 Weather:

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND (e.g. woodland, agricultural field, wetland, etc.) Surface Slopes Slope (%)

Vegetation Landform Position on landscape (pitch shoot)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area >100 foot  
Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pit: \_\_\_\_\_ Depth Standing Water in Hole: \_\_\_\_\_

Estimated Depth to High Groundwater: 30'

Deep Observation Hole Number: 16-31

Depth (ft)	Soil Horizon/ Layer	Soil Matrix Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Mohs)	Other
			Depth	Color	Percent		GroVol	Cobbles & Stones			
4	A	Tan/Brown 31/2	-	-	-	SL	-	-	M	F	SURFACE BOULDERS
16	B	Tan/Yellow 5/6	-	-	-	SL	-	-	M	F	
70	C <sub>1</sub>	2.5Y 6/2	30'	5/6	25	LS			M	F	
120	C <sub>2</sub>	2.5Y 6/3				LS			M	F	

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HARRINGTON, P.E., MA SE 1012



City/Town of  
Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Buxtonborough "T"bush" Central, LLC  
700 1/8th Massachusetts Ave

STAMSKI AND McNARY, INC.  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-329 Z1 Zoli, Time: Sun 90  
Dated Weather:

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND (e.g. woodland, agricultural field, vacant lot, etc.) Slope %  
Slope (ft) Surface Slopes

Vegetation Condition Position on Landscape (inclined slope)

3. Distances from: Open Water Body feet Drainage Way feet Possible Wet Area >100 feet  
Property Line feet Drinking Water Well feet Other \_\_\_\_\_

4. Parent Material: Glacial till Unsuitable Material(s) Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: \_\_\_\_\_

Deep Observation Hole Number: 16-32

Depth (in)	Soil Horizon Layer	Soil Matrix Color-Model (Munsell)	Redoximorphic Features (moisture)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Molal)	Other
			Depth	Color	Percent		Grovel	Dobble & Stones			
5	A	10YR 3/2				SL			M	F	
15	B	10YR 5/6				SL			M	F	
59	C	2.5Y 5/5	31		75	LS			M	F	
59	R										

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNAH, P.E., MA SE 1012



City/Town of

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUXBOROUGH TOWN CENTRE, LLC.

700 &amp; 800 MARYCKIN SETTE, AVE

STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-339 Z1 Zolc Sm. So.  
Date: \_\_\_\_\_ Time: \_\_\_\_\_ Weather: \_\_\_\_\_

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Stones

Expo (%)

Vegetation

Landform

Position on landscape (check one)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area &gt;100' \_\_\_\_\_

Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

foot

## 4. Parent Material: PROGLACIAL

Unsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 170 OR 35.10M

Deep Observation Hole Number: 16-33

Depth (in.)	Soil Horizon/ Layer	Soil Matrix/ Color-Moist (Munsell)	Redoximorphic Features (moline)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
6	A	10YL 3/2	—	—	—	SL			M	F	
18	B	10YL 5/6	—	—	—	SL			M	F	
130	C	2.5Y 6/3	—	5AE GHC WELL	—	medium SAND	25	Sq	loose		

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINGTON, P.E., MA SE 1017



**City/Town of**

**Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal**

BUXBURY TOWN COUNCIL, H.C.

700-1800 MyMallorysets, NY

**C. On-Site Review** (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-34 9/21 2014

Sun 10

## 1. Looping

**Ground Elevation at Surface of Hole** \_\_\_\_\_

**Location (Identify on Plan) \_\_\_\_\_**

2. Land Use: Wooded Area

(e.g. woodland, agricultural field, woodland lot, etc.)

#### Syndic Stamps

Slana (3)

#### Vegetables.

Lanthanum

### Position on long-term (post-1990) climate

### 3. Distances from: Open Water Body

## Drainage Way

Possible Wet Area

Property Line

1

4. Parent Material: GLO-CM-1-THU

Unpublished Atelopis Gasteru - Vol. □ No. □

If Yes: Disturbed Soil  Material  Impervious Layer(s)  Weathered/Eroded Rock  Bedrock

#### REFERENCES AND NOTES

If Yes: Depth Wearing form (2) \_\_\_\_\_ Depth Standing Watch List \_\_\_\_\_

#### Ballasted Pavement Construction

Deep Observation Hole Number: 162-34

### **Additional Notes**

## STORMWATER SOIL OBSERVATIONS

RICHARD J. HAMILTON P.E. MA SE 1012



City/Town or

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Burlington W. Town Control, LLC  
700-1 800 Main Street, Acton, MA

STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-35 9/30/2014 Sun No Weather

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use:

(e.g. woodland, agricultural land, vacant lot, etc.)

Surface Stones \_\_\_\_\_

Slope (%) \_\_\_\_\_

Vegetation \_\_\_\_\_

Erosion

Position on landscape (attach sketch)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area  $\frac{2}{100}$  foot

Property Line \_\_\_\_\_

Drinking Water Well \_\_\_\_\_

Other \_\_\_\_\_

## 4. Parent Material:

GLACIAL TILL

Unsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 14"

Deep Observation Hole Number: 16-35

Depth (ft)	Soil Horizon/ Layer	Soil Matrix: Coarse-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Garnet Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Calibers & Sizes			
10	A	Loamy 2/2	-	-	-	SL			M	F	Lo stones
19	B	Loamy 5/6	14	Loam chromatic	>5	SL			M	F	
41	C <sub>1</sub>	2/5Y 6/2			>20	SL	58		M	F	
120	C <sub>2</sub>	2/5Y 6/4				Calcareous to mineral WATER bearing sand					

Additional Notes

STORM WATER SOIL OBSERVATIONS

RICHARD J. KARWICK, P.E., MA SE 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BUSINESS UNIT: TOWN CENTER, LLC  
700 & 800 MASSACHUSETTS AVE.

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

**C. On-Site Review** (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-369-30 Date: 2/14 Time: 5:15 PM Weather:

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use:

WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope (%)

Vegetation

Landform

Position on landscape (pitch sheet)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area >100

foot

foot

foot

Property Line

Drinking Water Well

Other

foot

foot

4. Parent Material: GLENVILLE TILL

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 14

Deep Observation Hole Number: 16-369

Depth (in)	Soil Horizon/ Layer	Soil Matrix Color-Molot (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Mold)	Other
			Depth	Color	Percent		Gravel	Cobbles & Boulders			
10	A	10YR 2/2	—	—	—	SL	—	—	M	F	5 BAULDERS
17	B	10YR 5/6	14	LAW CHROMA	>5	SL	—	—	M	F	
36	C <sub>1</sub>	2.5Y 4/2		5YR 5/8	>20	SL			M	F	
84	C <sub>2</sub>	2.5Y 6/4				STAND VARICATED	3G 5G	5S 5C	M	VERY FIRM	
170	C <sub>3</sub>	2.5Y 4/2				MEADOW TO CAVANEE			M	F	20 B LDR
						C <sub>3</sub> SL					

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. MCNARY, P.E., M.A.S.E. 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

**STAMSKI AND McNARY, INC.**

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

BEDBROOK TOWN CENTRE, LLC

700 L BOSTON HIGHWAY, ACTON, MA 01720

C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-37930 Z-14 Date: 5/20/02 Time: 9:00 Weather:

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use:

WOODLAND (e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes Slope (%)

Vegetation Condition Position on hillside (steepest)

3. Distances from: Open Water Body feet Drainage Way feet Possible Wet Area feet

Property Line feet Drinking Water Well feet Other feet

4. Parent Material:

GLACIAL TILL

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Water from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 44

Deep Observation Hole Number: 16-37-

Depth (in)	Soil Horizon/ Layer	Soil Matrix Color-Molot (Munsell)	Redoximorphic Features (notches)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Mold)	Other
			Depth	Color	Percent		Gravel	Cobbles & Boulders			
6	A	10YL 312	-	-	-	SL	-	-	M	F	
12	B	10YL 516	-	-	-	SL	-	-	M	F	
38	C <sub>1</sub>	2.5Y .64	-	-	-	COARSE SAND	10G	5C	M	VERY FIRM	
72	C <sub>2</sub>	2.5Y 613	44	5YL 518	>5	WET SAND			M	F	
112	C <sub>3</sub>	2.5Y 612				SANDY LOAM			M	F	
		BOULDER									

Additional Notes

STORM WATER SOIL OBSERVATIONS

RICHARD J. MANNING, P.E., M.A.S.E. 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

**STAMSKI AND McNARY, INC.**

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

Buckbee & W. TOWN CENTER, LLC.

700 R. 800 MASSACHUSETTS, AVE

C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: (n-3) 9/30/2017

SM 9/30  
Wenthol

1. Location

Ground Elevation of Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes \_\_\_\_\_

Slope (%) \_\_\_\_\_

Vegetation \_\_\_\_\_

Landform \_\_\_\_\_

Position on landscape (hilltop, slope, etc.) \_\_\_\_\_

3. Distances from: Open Water Body \_\_\_\_\_

Drainage Way \_\_\_\_\_

Possible Wet Area >100 foot

Property Line \_\_\_\_\_

Drinking Water Well \_\_\_\_\_

Other \_\_\_\_\_

4. Parent Material: ROCKWELL

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractioned Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pt. \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 17

Deep Observation Hole Number: 16-39

Depth (in)	Soil Horizon/ Layer	Soil Matrix/ Color-Moist (Munsell)	Redoximorphic Features (inches)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
6	A	10YR 3/2	—	—	—	SL	—	—	M	F	
12	B	10YR 5/6	—	—	—	SL	—	—	M	F	
48	C <sub>1</sub>	2.5Y 6/6	—	—	—	LS	—	—	M	F	
72	C <sub>2</sub>	2.5Y 6/4	—	—	—	COARSE SAND	15G	2C	M	VERY LOOSE	
112	C <sub>3</sub>	2.5Y 6/6				MEDIUM SAND	VARIOUS COLORS	2C	M	VERY LOOSE	
120	C <sub>4</sub>	2.5Y 6/2				SL					

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNAH, P.E., MA SE 1012



Commonwealth of Massachusetts

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

Surveyor working for TOWN OF CONCORD, MASS.

700 &amp; 800 MULBERRY STREETS, VINEYARD

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-394 | 30 | Z014 | Date: 10/14 | Time: 10:00 AM | Worker: SM PW

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODS, AGRICULTURE (e.g. woodland, agricultural field, vacant lot, etc.) | Surface Slopes \_\_\_\_\_ | Slope (%) \_\_\_\_\_

Vegetation | Erosion | Position on Landscape (Block) \_\_\_\_\_

3. Distances from: Open Water Body \_\_\_\_\_ foot | Drainage Way \_\_\_\_\_ foot | Possible Wet Area \_\_\_\_\_ foot |  
Property Line \_\_\_\_\_ foot | Drinking Water Well \_\_\_\_\_ foot | Other: \_\_\_\_\_4. Parent Material: REGOLITH | Unsuitable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 96

Deep Observation Hole Number: 16-39

Depth (in)	Soil Horizon Layer	Soil Matrix: Color-Moisture (Munsell)	Redoximorphic Features (moist)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
6	A	10YR 3/2	~	-	-	SL	--	--	M	F	
12	B	10YR 5/6	~	-	-	LS	--	--	M	F	
60	C <sub>1</sub>	2.5Y 6/4	-	-	-	COARSE SAND	15G	2C	M	VERY LOOSE	
96	C <sub>2</sub>	2.5Y 6/4	-	-	-	medium SAND	-	-	M	VERY FIRM	
120	C <sub>3</sub>	2.5Y 6/2	96		>5	SL	-	5S	M	F	

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINGTON, P.E., M.A.SCE L012



Commonwealth of Massachusetts

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Beverlyport Park Treatment Center, L.L.C.  
700 N. River Massachusetts, MA

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-409/30 Date: 7-16-90 Time: Sun 90  
Weather:

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: Wetland Area Surface Slope \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation	Landform	Position on landscape (ininch slope)
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3. Distances from: Open Water Body 100 feet Drainage Way 100 feet Possible Wet Area > 100 feet  
Property Line 100 feet Drinking Water Well 100 feet Other: \_\_\_\_\_

4. Parent Material: Proglacial till Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 140+

Deep Observation Hole Number: 16-40

Depth (in)	Soil Horizon/Layer	Soil Matrix/Color/Moist (Munsell)	Radiximorphic Features (millimeters)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Matal)	Other				
			Depth	Color	Percent		Gravel	Cobbles & Stands							
4	A	10YR 3/2	—	—	—		—	—	M						
10	B	10YR 5/6	—	—	—		—	—	M						
70	C <sub>1</sub>	2.5Y 6/3	—	—	—	COARSE SAND	15G	3C	M	VERY WET					
140	C <sub>2</sub>	2.5Y 6/6	—	—	—	MEDIUM SAND	—	—	M	VERY LOOSE					

Additional Notes: STORM WATER SOIL OBSERVATIONS

RICHARD J. MCNARY, P.E., MA SE 1012



## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Buxton Brook WTRWTR Control, LLC  
700 A Blvd, Maynard, MA 01754

**STAMSKI AND McNARY, INC.**

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: (6-4) 9 30 Zol 4 Sm 80 Weather

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: (e.g. woodland, agricultural field, vacant lot, etc.) **Wetland Area** Surface Slope \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation Erosion Position on Landscape (attach sheet)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area > 100 feet  
Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

4. Parent Material: **Bedrock** Unstable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: **104**

Deep Observation Hole Number: **(6-4)**

Depth (in)	Soil Horizon/Layer	Soil Matrix Color-Moist (Munsell)	Redox/Morphic Features (metres)			Soil Texture (USDA)	Coarse Fragments % by Volume	Soil Structure	Soil Consistency (Mohs)	Other
			Depth	Color	Percent					
4	A	104R 31L	—	—	—	SL	—	M		
9	B	104R 51G	—	—	—	LS	—	M		
94	C	2.5Y 41Y	—	—	—	MEDIUM COARSE SAND	10G	M	Very Firm	
132	C2	2.5 61G	104	54R 51G	75	FINE LOAMY SOIL		M		

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNAH, P.E., MA SE 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BURGBOROUGH TOWNSHIP CENTER, LLC  
700 1/800 MASSACHUSETTS AVE

STAMSKI AND McNARY, INC.  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

**C. On-Site Review** (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-429/30 Date 10/14 Time 10:00 Weather Sun 80

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use:

WOODLAND  
(e.g. woodland, agricultural field, vacant lot, etc.)

Ground Slope

Slope (%)

Vegetation

Landform

Position on landscape (pitch direction)

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area 21(%) foot foot foot

Property Line foot Drinking Water Well foot Other foot

4. Parent Material: GLENWELL TILL Unusable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from PII \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 22

Deep Observation Hole Number: 16-429

Depth (ft)	Soil Horizon/ Layer	Soil Matrix/ Color-Moist (Munsell)	Radiximorphic Features (inches)			Soil Texture (USDA)	Coarse Fractional % by Volume		Soil Structure	Soil Consistency (Mohs)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stnches			
6	A <sub>p</sub>	WEYL 3/2	—	—	—	SL	—	—	M	R	
10	B	WEYL 5/6	—	—	—	LS	—	—	M	R	
36	C <sub>1</sub>	2.5Y 6/6	22	STYL 5/8	>5	FINE LS	—	—	M	R	
72	C <sub>2</sub>	2.5Y 6/4				MEDIUM SAND			M	VERY FRIABLE	
105	C <sub>3</sub>	2.5Y 6/2				SL			M	R	
		BOULDER									

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNAH, P.E., M.A.S.E. 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Bucksborough Town Center, LLC  
700 + 800 MARYACHAUX AVE.

**STAMSKI AND McNARY, INC.**

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

**C. On-Site Review** (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-434 Date: 30 Time: 7:04 Weather: Sun

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WILDLAND

(e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation \_\_\_\_\_ Condition \_\_\_\_\_ Position on landscape (attach sketch) \_\_\_\_\_

3. Distances from: Open Water Body \_\_\_\_\_ Drainage Way \_\_\_\_\_ Possible Wet Area >100' \_\_\_\_\_

Property Line \_\_\_\_\_ Drinking Water Well \_\_\_\_\_ Other \_\_\_\_\_

4. Perennial Material: GRASS

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractionated Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 54

Deep Observation Hole Number: 16-43

Depth (ft)	Soil Horizon/Layer	Soil Matrix: Color/Moist (Munsell)	Redoximorphic Features (Infiltration)			Soil Texture (USDA)	Coarse Fragments % by Volume	Soil Structure	Soil Consistency (Mohr)	Other
			Depth	Color	Percent					
6	A <sub>p</sub>	10YR 31/2	-	-	-	SL		M	F	
12	B <sub>w</sub>	10YR 5/6	-	-	-	SL		M	F	
12-24	B <sub>c</sub>	2.5Y 6/6	-	-	-	FINE LS		M	F	
114	C <sub>1</sub>	2.5Y 4/4	54	5YL 518	>5	MEDIUM FINE SANDY	56 25		VERY FIRM	
124	C <sub>2</sub>	2.5Y 6/2				SL				

Additional Notes:

STORMWATER SOIL OBSERVATIONS

RICHARD J. MARINARO, P.E., N.J. SE 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

**STAMSKI AND McNARY, INC.**

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

BUXBURYBROUGH TOWNSHIP CENTER, LLC.

700 N BOSTON TURNpike, Acton, MA 01720

## C. On-Site Review

(minimum of two plots required at every proposed disposal area)

Deep Observation Hole Number: 16-44 Date: 30/2/14 Time: 10:00 AM

Weather: Sunny

### 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

### 2. Land Use:

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes \_\_\_\_\_

Slope (%) \_\_\_\_\_

Vegetation \_\_\_\_\_

Landform \_\_\_\_\_

Position on Landscapes (ininch sheet) \_\_\_\_\_

### 3. Distances from:

Open Water Body \_\_\_\_\_ foot Drainage Way \_\_\_\_\_ foot Possible Wet Area >100 foot

Property Line \_\_\_\_\_ foot

Drinking Water Well \_\_\_\_\_ foot

Other \_\_\_\_\_ foot

### 4. Parent Material:

PROGLACIAL

Uncultivable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

### 5. Groundwater Observed: Yes No

If Yes: Depth Weeping from PII \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 42

Deep Observation Hole Number: 16-44

Depth (in)	Soil Horizon/ Layer	Soil Matrix/ Color-Moist (Munsell)	Radixlimorphic Features (moisture)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Grunel.	Cobbles & Stones			
7	A	low 3/2	-	-	-	SL	-	-	M	F	10 BLDR
18	B	10YR 5/6	-	-	-	SL	-	-	M	F	
72	C1	2.5Y 6/4	42	low Chlorine	>5	medium SAND	-	-	M	very fractured	
100	C2	2.5Y 6/2				SL			M	F	
		No Reversal									

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HAMMING, P.E., M.A.S.E. 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Buxton, MA 01513  
700-1800 Massachusetts Avenue

**STAMSKI AND McNARY, INC.**

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

**C. On-Site Review** (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-459 Date 30 Time 2014

SM No.  
Weather

**1. Location**

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

**2. Land Use:** WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope(%)

Vegetation

Landform

Position on landscape (attach sketch)

**3. Distances from:** Open Water Body foot Drainage Way foot Possible Wet Area >100

Property Line foot Drinking Water Well foot Other foot

**4. Parent Material:** GLACIAL TILL

Unstable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

**5. Groundwater Observed:** Yes  No

If Yes: Depth Weeping from Pill — Depth Standing Water in Hole —

Estimated Depth to High Groundwater: 24

Deep Observation Hole Number: 16-45

Depth (in.)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Radixmorphia Features (molluscs)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gavel	Cobbles & Stones			
6	A <sub>p</sub>	LUVR 31L	—	—	—	SL	—	—	M	F	
24	B <sub>w</sub>	LUVR 516	—	—	—	SL	—	—	M	F	
72	C	2.74 412	24		>10	SL	—	—	M	Frm	
		16/20-35, AC									

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNAH, P.E., MA SE 1012



COMMONWEALTH OF

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BENSONBURG & THOMAS CENTER, L.L.C.  
700 A 800 MARYLAND AVENUE, ALEX.

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-46 Date 9/30/14 Time PM Weather Sunny

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WILDERNESS (e.g. woodland, agricultural field, vacant lot, etc.) Surface Slope \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation	Landform	Position on landscape (attach sheet)
------------	----------	--------------------------------------

3. Distances from: Open Water Body 1000 foot Drainage Way 1000 foot Possible Well Area 1000 foot  
Property Line 1000 foot Drinking Water Well 1000 foot Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 24

Deep Observation Hole Number: 16-46

Depth (in)	Soil Horizon/ Layer	Soil Minikit: Color-Moist (Munsell)	Redoximorphic Features (moths)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Constituents (Mineral)	Other
			Depth	Color	Percent		Gravel	Cobble & Stone			
9	A <sub>1</sub>	LOW 3/2	-	-	-	SL	-	-	M	F	
15	B <sub>1</sub>	LOW 1/2	-	-	-	SL	-	-	M	F	
24	C	2.5 1/3	24		>10	SL	-	-	M	Frm	

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. THORNTON, P.E., MA SE 1012



City of Uxbridge

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BryceBorloue & Towne Control, LLC  
700 A 800 Main Street, Suite 100  
Acton, MA 01720

STAMSKI AND McNARY, INC.

1000 Main Street  
ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-479/30 Date: 2011 Time: 5:15 PM Weather: Partly Cloudy

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use:

Woodland (e.g. woodland, agricultural field, vacant lot, etc.) Surface Stones Slope (%)

Vegetation Landform Position on landscape (attach sketch)

3. Distances from: Open Water Body (ft) Drainage Way (ft) Possible Wet Area (ft) 100' (ft)

Property Line (ft) Drinking Water Well (ft) Other \_\_\_\_\_

4. Parent Material: GRANULAR TILL Unstable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 32 \_\_\_\_\_

Deep Observation Hole Number: 16-47

Depth (in.)	Soil Horizon/layer	Soil Matrix: Color-Moist (Munsell)	Redox/morphological Features (mention)			Soil Texture (USDA)	Coarse Fragments % by Volume	Soil Structure	Soil Consistency (Mohs)	Other
			Depth	Color	Percent					
10	A <sub>p</sub>	10YL 3/2	-	-	-	SL	-	M	F	10 STONES
24	B <sub>w</sub>	10YL 5/6	-	-	-	SL	-	M	F	
100	C	2.5Y 3/2 6/13	Low cation exchange	>10	SL	5G	-	M	Firm	
		NO REFUSAL								

Additional Notes: STORMWATER SOIL OBSERVATIONS  
RICHARD J. MCNARY, P.E., MA SE 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BRYNGORLWYD TOWNSHIP COUNCIL, LLC  
700 A BEE MANAGEMENT, INC.

STAMSKI AND McNARY, INC.

2000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

**C. On-Site Review** (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-18930 Date: 7-24 Time: PM Weather: SM

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: WOODLAND (e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope(%)

Vegetation

Condition

Position on Landscapes (Inclined or flat)

3. Distances from: Open Water Body 100 foot Drainage Way 100 foot Possible Wet Area 100 foot

Property Line 100 foot

Drinking Water Well 100 foot

Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Eroded Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Water from Pit \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 36

Deep Observation Hole Number: 16-189

Depth (in)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (moist)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stnches			
7	A <sub>v</sub>	10YL 3/2	-	-	-	SL	.	.	M	F	20 VSLS, OLS
14	B	10YL 5/6	-	-	-	SL	.	.	M	F	
36	C <sub>1</sub>	2.5Y 4/4	36	5YL 5/8	YS	LS	.	.	M	Firm	
100+	C <sub>2</sub>	2.5Y 6/3	.	.	.	SL	.	.	.	.	
		NO REVERSE									

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. FRANKE, P.E., M.A.S.E. 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

Subdivision: "T-Bunk" Condo, LLC  
700 & 800 Mass Ave., Acton, MA

C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-499 Date: 30/2016 Time: 5:00 PM

S.M.  
Wuhrer

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use: Wetland (e.g. woodland, agricultural land, vacant lot, etc.) Surface Stones: None Slope (%): 0%

Vegetation: None Condition: Good Position on landscape (ditch edge) \_\_\_\_\_

3. Distances from: Open Water Body 100 feet Drainage Way 100 feet Possible Wet Area >100 feet  
Property Line 100 feet Drinking Water Well 100 feet Other \_\_\_\_\_

4. Parent Material: Glacial Till Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 30

Deep Observation Hole Number: 16-499

Depth (in)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (mention)			Soil Texture (USDA)	Cairns Fragments % by Volume	Soil Structure	Soil Consistency (Moist)	Other
			Drapes	Color	Percent					
7	A	10YR 3/2	-	-	-	SL	-	M	F	20 BSWP
14	B	10YR 5/6	-	-	-	SL	-	M	F	
48	C	2.5Y 6/6	30	low CHROMA	>5	LS	-	M	Firm	
58	C2	2.5Y 6/3				SL	-	M	Firm	
110	C3	2.5Y 6/2				SL	5g	M	Firm	
		MA BRASH								

Additional Notes: STORM WATER SOIL OBSERVATIONS  
RICHARD J. WANNINGER, P.E., MA SE 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Beverlytonia Town Center, LLC  
700 L St., Beverly, MA 01915

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

**C. On-Site Review** (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-50 9/30/2014

Date

Time

Sept 6/14  
Weather

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

**OVERCAST**

2. Land Use: WOODLAND (e.g. woodland, agricultural field, vacant lot, etc.) Surface Slopes \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation Elevation Position on landscape (hillside, slope)

3. Distances from: Open Water Body foot Drainage Way foot Possible Wet Area >100 foot  
Property Line foot Drinking Water Well foot Other \_\_\_\_\_

4. Parent Material: GLACIAL TILL Unuseable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pt. \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 30

Deep Observation Hole Number: 16-50

Depth (in)	Soil Horizon/Layer	Soil Matrix: Color/Moist (Munsell)	Redoximorphic Features (if hollow)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
7	A	10YR 3/2	—	—	—	SL	—	—	M	F	soil
24	B	10YR 5/6	—	—	—	SL	—	—	M	F	BLDLV
30	C	2.5Y 6/5	30	SMR 5/8	>5	SL	—	—	M	Firm	
		NO FROST									

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANNINEN, P.E., M.A.SCE 1017



## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BURGESSVILLE TOWNSHIP COUNCIL, LLC  
700 1/2 BROAD MARYLAND AVENUE, AVE

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

**C. On-Site Review** (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 6-51 Date 9/30/2014 Time 10:00 Weather Sunny

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use:

Wetland Area Surface Stones Slope (%)  
(e.g. Woodland, agricultural land, vacant lot, etc.)

Vegetation Landform Position on Landform (attach sketch)

3. Distances from: Open Water Body 100' feet Discharge Way 100' feet Possible Wet Area >100' feet

Property Line 100' feet Drinking Water Well 100' feet Other \_\_\_\_\_

4. Parent Material:

Glacial till

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: \_\_\_\_\_

51'

Deep Observation Hole Number: 6-51

Depth (in)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Radix/morpho Features (moisture)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Boulders			
10	A	10YR 3/2	—	—	—	SL			M	F	
20	B	10YR 5/6	—	—	—	SL			M	F	
50	C	2.5Y 4/3	24		>20	SL			M	Firm	
50	R					SL					

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. MCNARY, P.E., N.H. SE 1012



City/Town

## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BURLINGTON TOWN CENTER, LLC.

700 &amp; 800 MASSACHUSETTS AVE

STAMSKI AND McNARY, INC.

1000 Main Street

ACTON, MA 01720

Engineering • Planning • Surveying

## C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-529/30 Date: 2/14/04 Thru: 5M No. Washer:

## 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

## 2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope (%)

Vegetation

Landform

Position on Landscape (ininch slope)

## 3. Distances from: Open Water Body foot Drainage Way foot Possible Wet Area foot

Property Line

Drinking Water Well

Other

foot

foot

foot

foot

4. Parent Material: GLACIAR TILL Unstable Materials Present: Yes  No If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock 5. Groundwater Observed: Yes  No 

If Yes: Depth Weeping from Pt. \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 30

Deep Observation Hole Number: 16-52

Depth (in)	Soil Horizon/ Layer	Soil Matrix: Color/Molst (Munsell)	Radixomorphic Features (minerals)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Mold)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
8	A	tan 312	—	—	—	SL	—	—	M	F	20 WATER
22	B	tan 516	—	—	—	SL	—	—	M	F	
60	C	2.5Y 4/3	30.	—	75	SL	—	—	M	F	
60	R		30.								

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. HANLINTON, P.E., MA SE 1012



## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BurkeBoring in Town Control, LLC  
700 S. Blvd MURFREESBORO, TN 37130

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

### C. On-Site Review (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-53 Date: 9/30/14 Time: 2:11 PM Weather: Sunny

#### 1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

#### 2. Land Use:

WOODLAND

(e.g. woodland, agricultural field, natural lot, etc.)

Surface Slopes

Slope (%)

Vegetation

Landform

Position on landscape (attach sketch)

3. Distances from: Open Water Body None Drainage Way foot Possible Wet Area >100 foot

Property Line foot

Drinking Water Well foot

Other \_\_\_\_\_

4. Parent Material: Glacial Till Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from PII \_\_\_\_\_ Depth Standing Water in Hole \_\_\_\_\_

Estimated Depth to High Groundwater: 30

Deep Observation Hole Number: 16-53

Depth (in.)	Soil Horizon / Layer	Soil Matrix / Color-Moist (Munsell)	Redoximorphic Features (inches)			Soil Texture (ISODA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Mohr)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
10	A	10YR 3/2	-	-	-	SL	-	-	M	F	20 BLDR
20	B	10YR 5/6	-	-	-	SL	-	-	M	F	
100	C	2.5Y 3/0				SL			M	F	
100	R										
-											

Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. MURKIN, P.E., MA SE 1013



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BURLINGTON TWENTY CENTER, LLC  
700 A. 800 MELROSE AVENUE, AVE

**STAMSKI AND McNARY, INC.**

1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

**C. On-Site Review** (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-54 Date: 9/30/14 Time: 5pm Weather: Wet

1. Location

Ground Elevation at Surface of Hole: \_\_\_\_\_

Location (Identify on Plan): \_\_\_\_\_

2. Land Use: WOODLAND

(e.g. woodland, agricultural field, vacant lot, etc.)

Surface Slope(s): \_\_\_\_\_

Slope(%): \_\_\_\_\_

Vegetation: \_\_\_\_\_

Condition: \_\_\_\_\_

Position on Inclines (attach sketch): \_\_\_\_\_

3. Distances from: Open Water Body \_\_\_\_\_ feet Drainage Way \_\_\_\_\_ feet Possible Well Area >100' \_\_\_\_\_ feet

Property Line \_\_\_\_\_ feet Drinking Water Well \_\_\_\_\_ feet Other: \_\_\_\_\_ feet

4. Parent Material: Glacial Till

Unstable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from Pill: \_\_\_\_\_ Depth Standing Water in Hole: \_\_\_\_\_

Estimated Depth to High Groundwater: \_\_\_\_\_

Deep Observation Hole Number: 16-54

Depth (in)	Soil Horizon/Layer	Soil Matrix Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Mohs)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
6	A	Tan 3/2	-	-	-	SL	-	-	M	F	
11	B	Tan 5/6	-	-	-	SL	-	-	M	F	
80	C	2.5y 6/3	28		>5	LS	50	55	V1	F	FAWN
80	R										

Additional Notes: STORMWATER SOIL OBSERVATIONS

RICHARD J. HANLON, P.E., M.A.S.E. 1012



## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BEDFORD, MASSACHUSETTS  
TOWN OF BEDFORD, MASSACHUSETTS

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

### C. On-Site Review (Minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-559-30 Date: 2016 Time: 5:45 PM

Weather: Sunny

#### 1. Location

Ground Elevation at Surface of Hole: \_\_\_\_\_

Location (Identify on Plan): \_\_\_\_\_

#### 2. Land Use: WOODLAND

(e.g., woodland, agricultural field, vacant lot, etc.)

Surface Slopes

Slope(%): \_\_\_\_\_

Vegetation: \_\_\_\_\_ Landform: \_\_\_\_\_ Position on landscape (tenth sheet): \_\_\_\_\_

3. Distances from: Open Water Body: 100 feet Drainage Way: 100 feet Possible Wet Area: >100 feet  
Property Line: 100 feet Drinking Water Well: 100 feet Other: \_\_\_\_\_

#### 4. Parent Material: PROGLACIAL

Unstable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

#### 5. Groundwater Observed: Yes No

If Yes: Depth Weeping from Pit: \_\_\_\_\_ Depth Standing Water in Hole: \_\_\_\_\_

Estimated Depth to High Groundwater: 114

Deep Observation Hole Number: 16-55

Depth (in)	Soil Horizon/Layer	Soil Matrix Color-Moist (Munsell)	Redoximorphic Features (indicators)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
7	A	10YR 5/6	—	—	—		—	—			
20	B	10YR 5/6	—	—	—		—	—			
114	C	2.5Y 6/4	—	—	—	FMS SMWD	—	—	M	VERY FIRM AERATED	
120	C2	2.5Y 6/4	114			LS	—	—			

Additional Notes:

STORMWATER SOIL OBSERVATIONS

RICHARD J. MCNARY, P.E., MA SE 1012



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

BENGBOROUGH TOWN COUNCIL, LLC  
700-1-800 MASSACHUSETTS, MA

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
ACTON, MA 01720  
Engineering • Planning • Surveying

**C. On-Site Review** (minimum of two holes required at every proposed disposal area)

Deep Observation Hole Number: 16-569-30 Date: 10/14/14 Time: 10:00 AM Weather: Sunny

1. Location

Ground Elevation at Surface of Hole \_\_\_\_\_

Location (Identify on Plan) \_\_\_\_\_

2. Land Use:

WOODLAND (e.g. woodland, agriculture field, vacant lot, etc.) Surface Slopes \_\_\_\_\_ Slope (%) \_\_\_\_\_

Vegetation

Landform

Position on landscape (attach sketch)

3. Distances from: Open Water Body \_\_\_\_\_ feet Drainage Way \_\_\_\_\_ feet Possible Wet Area > 100' \_\_\_\_\_ feet

Property Line \_\_\_\_\_ feet

Drinking Water Well \_\_\_\_\_ feet

Other \_\_\_\_\_ feet

4. Parent Material:

Glaciol till

Unsuitable Materials Present: Yes  No

If Yes: Disturbed Soil  Fill Material  Impervious Layer(s)  Weathered/Fractured Rock  Bedrock

5. Groundwater Observed: Yes  No

If Yes: Depth Weeping from PH \_\_\_\_\_ feet Depth Standing Water in Hole \_\_\_\_\_ feet

Estimated Depth to High Groundwater: 132' GNL (BGSW)

Deep Observation Hole Number: 16-56

Depth (in)	Soil Horizon/ Layer	Soil Matrix/ Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments % by Volume		Soil Structure	Soil Consistency (Molot)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
6	A	Loamy 31/2	—	—	—	SL	—	—	M	F	10 BLDR
19	B	Loamy 5/6	—	—	—	SL	—	—	M	F	
132	C	2.5y 6/3	—	—	—	LS	20-55	—	M	F	

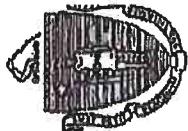
Additional Notes

STORMWATER SOIL OBSERVATIONS

RICHARD J. THAYER, P.E., M.A.S.E. 1012

**STAMSKI AND McNARY, INC.**  
1000 Main Street  
AFCTON, MA 01720  
Engineering • Planning • Surveying

Commonwealth of Massachusetts  
City/Town of \_\_\_\_\_  
**Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal**



#### D. Determination of High Groundwater Elevation

1. Method used:
  - Depth observed standing water in observation hole A. \_\_\_\_\_ inches      B. \_\_\_\_\_ inches
  - Depth weeping from side of observation hole A. \_\_\_\_\_ inches      B. \_\_\_\_\_ inches
  - Depth to soil redoximorphic features (mottles) A. \_\_\_\_\_ inches      B. \_\_\_\_\_ inches
  - Groundwater adjustment (USGS methodology) A. \_\_\_\_\_ inches      B. \_\_\_\_\_ inches

2. Index Well Number \_\_\_\_\_ Reading Date \_\_\_\_\_ Index Well Level \_\_\_\_\_

Adjustment Factor \_\_\_\_\_ Adjusted Groundwater Level \_\_\_\_\_

#### E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

1. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system? Yes  No  *Soil Evaluation Soil Observations*

b. If yes, at what depth was it observed? Upper boundary: \_\_\_\_\_ inches  
Lower boundary: \_\_\_\_\_ inches

#### F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

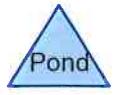
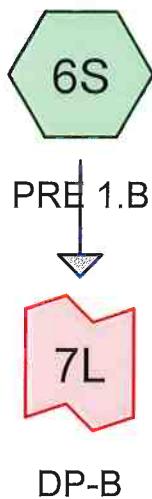
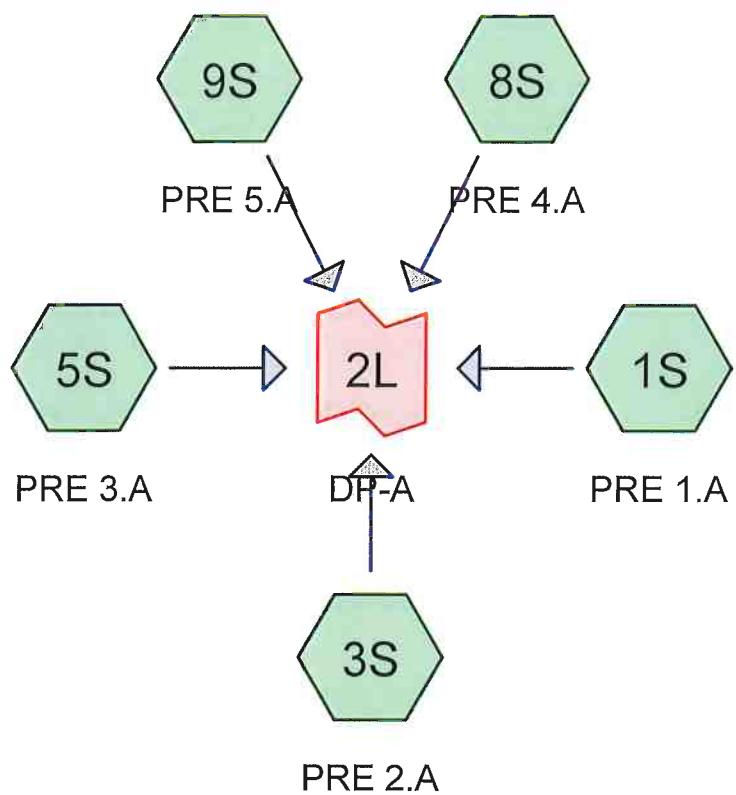
*[Signature]*  
Signature of Soil Evaluator  
E. C. Tiareo, S.E. 1012  
Typed or Printed Name of Soil Evaluator/License Number  
*Soil Evaluation Soil Observations*  
Name of Board of Health Witness  
*UNWITNESSED*  
Board of Health

Date 10/25/1994  
\*Date of Soil Evaluator Exam

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with Percolation Test Form 12.

**Appendix D - Existing Conditions Hydrologic Calculations**

---



#### Routing Diagram for 6092 - PRE

Prepared by {enter your company name here}, Printed 7/16/2019  
 HydroCAD® 10.00-21 s/n 03590 © 2018 HydroCAD Software Solutions LLC

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment 1S: PRE 1.A**

Runoff Area=499,537 sf 0.23% Impervious Runoff Depth=0.13"  
Flow Length=923' Tc=19.0 min CN=51 Runoff=0.24 cfs 0.123 af

**Subcatchment 3S: PRE 2.A**

Runoff Area=202,863 sf 0.00% Impervious Runoff Depth=0.20"  
Flow Length=572' Tc=19.6 min CN=54 Runoff=0.26 cfs 0.076 af

**Subcatchment 5S: PRE 3.A**

Runoff Area=188,718 sf 0.00% Impervious Runoff Depth=0.92"  
Flow Length=320' Tc=16.7 min CN=73 Runoff=3.11 cfs 0.332 af

**Subcatchment 6S: PRE 1.B**

Runoff Area=91,035 sf 0.00% Impervious Runoff Depth=0.00"  
Flow Length=251' Tc=10.2 min CN=33 Runoff=0.00 cfs 0.000 af

**Subcatchment 8S: PRE 4.A**

Runoff Area=17,402 sf 0.00% Impervious Runoff Depth=1.26"  
Tc=6.0 min CN=79 Runoff=0.57 cfs 0.042 af

**Subcatchment 9S: PRE 5.A**

Runoff Area=45,444 sf 0.00% Impervious Runoff Depth=0.37"  
Tc=6.0 min CN=60 Runoff=0.24 cfs 0.032 af

**Link 2L: DP-A**

Inflow=3.66 cfs 0.605 af  
Primary=3.66 cfs 0.605 af

**Link 7L: DP-B**

Inflow=0.00 cfs 0.000 af  
Primary=0.00 cfs 0.000 af

**Total Runoff Area = 23.990 ac Runoff Volume = 0.605 af Average Runoff Depth = 0.30"**  
**99.89% Pervious = 23.963 ac 0.11% Impervious = 0.027 ac**

**6092 - PRE**

Type III 24-hr 2-year Rainfall=3.10"

Prepared by {enter your company name here}

Printed 7/16/2019

HydroCAD® 10.00-21 s/n 03590 © 2018 HydroCAD Software Solutions LLC

Page 3

**Summary for Subcatchment 1S: PRE 1.A**

Runoff = 0.24 cfs @ 12.72 hrs, Volume= 0.123 af, Depth= 0.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
88,048	30	Woods, Good, HSG A
404,937	55	Woods, Good, HSG B
5,390	70	Woods, Good, HSG C
1,162	98	Roofs, HSG B
499,537	51	Weighted Average
498,375		99.77% Pervious Area
1,162		0.23% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.6	50	0.0700	0.11		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
11.4	873	0.0650	1.27		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
19.0	923	Total			

**Summary for Subcatchment 3S: PRE 2.A**

Runoff = 0.26 cfs @ 12.58 hrs, Volume= 0.076 af, Depth= 0.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
9,883	30	Woods, Good, HSG A
188,641	55	Woods, Good, HSG B
4,339	77	Woods, Good, HSG D
202,863	54	Weighted Average
202,863		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	50	0.0200	0.07		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
7.1	522	0.0600	1.22		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
19.6	572	Total			

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Type III 24-hr 2-year Rainfall=3.10"

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Page 4

**Summary for Subcatchment 5S: PRE 3.A**

Runoff = 3.11 cfs @ 12.26 hrs, Volume= 0.332 af, Depth= 0.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description			
154,569	77	Woods, Good, HSG D			
34,149	55	Woods, Good, HSG B			
188,718	73	Weighted Average			
188,718		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.7	50	0.0160	0.06		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
3.0	270	0.0900	1.50		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
16.7	320	Total			

**Summary for Subcatchment 6S: PRE 1.B**

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description			
79,057	30	Woods, Good, HSG A			
11,978	55	Woods, Good, HSG B			
91,035	33	Weighted Average			
91,035		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.1	50	0.0600	0.10		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
2.1	201	0.1000	1.58		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
10.2	251	Total			

**Summary for Subcatchment 8S: PRE 4.A**

Runoff = 0.57 cfs @ 12.10 hrs, Volume= 0.042 af, Depth= 1.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

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Type III 24-hr 2-year Rainfall=3.10"

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Page 5

Area (sf)	CN	Description
9,066	74	>75% Grass cover, Good, HSG C
5,485	96	Gravel surface, HSG C
2,851	61	>75% Grass cover, Good, HSG B
17,402	79	Weighted Average
17,402		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 9S: PRE 5.A**

Runoff = 0.24 cfs @ 12.15 hrs, Volume= 0.032 af, Depth= 0.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
15,021	30	Woods, Good, HSG A
3,541	55	Woods, Good, HSG B
26,882	77	Woods, Good, HSG D
45,444	60	Weighted Average
45,444		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Link 2L: DP-A**

Inflow Area = 21.900 ac, 0.12% Impervious, Inflow Depth = 0.33" for 2-year event

Inflow = 3.66 cfs @ 12.25 hrs, Volume= 0.605 af

Primary = 3.66 cfs @ 12.25 hrs, Volume= 0.605 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Summary for Link 7L: DP-B**

Inflow Area = 2.090 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-year event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 10-year Rainfall=4.60"

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Page 6

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment 1S: PRE 1.A**

Runoff Area=499,537 sf 0.23% Impervious Runoff Depth=0.58"  
Flow Length=923' Tc=19.0 min CN=51 Runoff=3.32 cfs 0.558 af

**Subcatchment 3S: PRE 2.A**

Runoff Area=202,863 sf 0.00% Impervious Runoff Depth=0.73"  
Flow Length=572' Tc=19.6 min CN=54 Runoff=1.95 cfs 0.285 af

**Subcatchment 5S: PRE 3.A**

Runoff Area=188,718 sf 0.00% Impervious Runoff Depth=1.97"  
Flow Length=320' Tc=16.7 min CN=73 Runoff=7.11 cfs 0.712 af

**Subcatchment 6S: PRE 1.B**

Runoff Area=91,035 sf 0.00% Impervious Runoff Depth=0.01"  
Flow Length=251' Tc=10.2 min CN=33 Runoff=0.00 cfs 0.002 af

**Subcatchment 8S: PRE 4.A**

Runoff Area=17,402 sf 0.00% Impervious Runoff Depth=2.46"  
Tc=6.0 min CN=79 Runoff=1.13 cfs 0.082 af

**Subcatchment 9S: PRE 5.A**

Runoff Area=45,444 sf 0.00% Impervious Runoff Depth=1.07"  
Tc=6.0 min CN=60 Runoff=1.12 cfs 0.093 af

**Link 2L: DP-A**

Inflow=12.72 cfs 1.730 af  
Primary=12.72 cfs 1.730 af

**Link 7L: DP-B**

Inflow=0.00 cfs 0.002 af  
Primary=0.00 cfs 0.002 af

**Total Runoff Area = 23.990 ac Runoff Volume = 1.733 af Average Runoff Depth = 0.87"**  
**99.89% Pervious = 23.963 ac 0.11% Impervious = 0.027 ac**

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Type III 24-hr 10-year Rainfall=4.60"

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Page 7

**Summary for Subcatchment 1S: PRE 1.A**

Runoff = 3.32 cfs @ 12.41 hrs, Volume= 0.558 af, Depth= 0.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
88,048	30	Woods, Good, HSG A
404,937	55	Woods, Good, HSG B
5,390	70	Woods, Good, HSG C
1,162	98	Roofs, HSG B
499,537	51	Weighted Average
498,375		99.77% Pervious Area
1,162		0.23% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.6	50	0.0700	0.11		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
11.4	873	0.0650	1.27		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
19.0	923	Total			

**Summary for Subcatchment 3S: PRE 2.A**

Runoff = 1.95 cfs @ 12.37 hrs, Volume= 0.285 af, Depth= 0.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
9,883	30	Woods, Good, HSG A
188,641	55	Woods, Good, HSG B
4,339	77	Woods, Good, HSG D
202,863	54	Weighted Average
202,863		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	50	0.0200	0.07		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
7.1	522	0.0600	1.22		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
19.6	572	Total			

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Type III 24-hr 10-year Rainfall=4.60"

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Page 8

**Summary for Subcatchment 5S: PRE 3.A**

Runoff = 7.11 cfs @ 12.24 hrs, Volume= 0.712 af, Depth= 1.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
154,569	77	Woods, Good, HSG D
34,149	55	Woods, Good, HSG B
188,718	73	Weighted Average
188,718		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.7	50	0.0160	0.06		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
3.0	270	0.0900	1.50		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
16.7	320	Total			

**Summary for Subcatchment 6S: PRE 1.B**

Runoff = 0.00 cfs @ 22.21 hrs, Volume= 0.002 af, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
79,057	30	Woods, Good, HSG A
11,978	55	Woods, Good, HSG B
91,035	33	Weighted Average
91,035		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.1	50	0.0600	0.10		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
2.1	201	0.1000	1.58		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
10.2	251	Total			

**Summary for Subcatchment 8S: PRE 4.A**

Runoff = 1.13 cfs @ 12.09 hrs, Volume= 0.082 af, Depth= 2.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

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Type III 24-hr 10-year Rainfall=4.60"

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Page 9

Area (sf)	CN	Description			
9,066	74	>75% Grass cover, Good, HSG C			
5,485	96	Gravel surface, HSG C			
2,851	61	>75% Grass cover, Good, HSG B			
17,402	79	Weighted Average			
17,402		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 9S: PRE 5.A**

Runoff = 1.12 cfs @ 12.11 hrs, Volume= 0.093 af, Depth= 1.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description			
15,021	30	Woods, Good, HSG A			
3,541	55	Woods, Good, HSG B			
26,882	77	Woods, Good, HSG D			
45,444	60	Weighted Average			
45,444		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Link 2L: DP-A**Inflow Area = 21.900 ac, 0.12% Impervious, Inflow Depth = 0.95" for 10-year event  
Inflow = 12.72 cfs @ 12.29 hrs, Volume= 1.730 af  
Primary = 12.72 cfs @ 12.29 hrs, Volume= 1.730 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Summary for Link 7L: DP-B**Inflow Area = 2.090 ac, 0.00% Impervious, Inflow Depth = 0.01" for 10-year event  
Inflow = 0.00 cfs @ 22.21 hrs, Volume= 0.002 af  
Primary = 0.00 cfs @ 22.21 hrs, Volume= 0.002 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment 1S: PRE 1.A**

Runoff Area=499,537 sf 0.23% Impervious Runoff Depth=0.92"  
Flow Length=923' Tc=19.0 min CN=51 Runoff=6.33 cfs 0.884 af

**Subcatchment 3S: PRE 2.A**

Runoff Area=202,863 sf 0.00% Impervious Runoff Depth=1.12"  
Flow Length=572' Tc=19.6 min CN=54 Runoff=3.37 cfs 0.434 af

**Subcatchment 5S: PRE 3.A**

Runoff Area=188,718 sf 0.00% Impervious Runoff Depth=2.60"  
Flow Length=320' Tc=16.7 min CN=73 Runoff=9.46 cfs 0.938 af

**Subcatchment 6S: PRE 1.B**

Runoff Area=91,035 sf 0.00% Impervious Runoff Depth=0.08"  
Flow Length=251' Tc=10.2 min CN=33 Runoff=0.02 cfs 0.014 af

**Subcatchment 8S: PRE 4.A**

Runoff Area=17,402 sf 0.00% Impervious Runoff Depth=3.15"  
Tc=6.0 min CN=79 Runoff=1.44 cfs 0.105 af

**Subcatchment 9S: PRE 5.A**

Runoff Area=45,444 sf 0.00% Impervious Runoff Depth=1.54"  
Tc=6.0 min CN=60 Runoff=1.71 cfs 0.134 af

**Link 2L: DP-A**

Inflow=20.15 cfs 2.494 af  
Primary=20.15 cfs 2.494 af

**Link 7L: DP-B**

Inflow=0.02 cfs 0.014 af  
Primary=0.02 cfs 0.014 af

**Total Runoff Area = 23.990 ac Runoff Volume = 2.509 af Average Runoff Depth = 1.25"**  
**99.89% Pervious = 23.963 ac 0.11% Impervious = 0.027 ac**

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Type III 24-hr 25-year Rainfall=5.40"

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Page 11

**Summary for Subcatchment 1S: PRE 1.A**

Runoff = 6.33 cfs @ 12.34 hrs, Volume= 0.884 af, Depth= 0.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
88,048	30	Woods, Good, HSG A
404,937	55	Woods, Good, HSG B
5,390	70	Woods, Good, HSG C
1,162	98	Roofs, HSG B
499,537	51	Weighted Average
498,375		99.77% Pervious Area
1,162		0.23% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.6	50	0.0700	0.11		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
11.4	873	0.0650	1.27		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
19.0	923	Total			

**Summary for Subcatchment 3S: PRE 2.A**

Runoff = 3.37 cfs @ 12.33 hrs, Volume= 0.434 af, Depth= 1.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
9,883	30	Woods, Good, HSG A
188,641	55	Woods, Good, HSG B
4,339	77	Woods, Good, HSG D
202,863	54	Weighted Average
202,863		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	50	0.0200	0.07		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
7.1	522	0.0600	1.22		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
19.6	572	Total			

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Type III 24-hr 25-year Rainfall=5.40"

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Page 12

**Summary for Subcatchment 5S: PRE 3.A**

Runoff = 9.46 cfs @ 12.24 hrs, Volume= 0.938 af, Depth= 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description		
154,569	77	Woods, Good, HSG D		
34,149	55	Woods, Good, HSG B		
188,718	73	Weighted Average		
188,718		100.00% Pervious Area		
Tc (min)	Length (feet)	Slope (ft/ft) Velocity (ft/sec) Capacity (cfs) Description		
13.7	50	0.0160	0.06	<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
3.0	270	0.0900	1.50	<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
16.7	320	Total		

**Summary for Subcatchment 6S: PRE 1.B**

Runoff = 0.02 cfs @ 15.30 hrs, Volume= 0.014 af, Depth= 0.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description		
79,057	30	Woods, Good, HSG A		
11,978	55	Woods, Good, HSG B		
91,035	33	Weighted Average		
91,035		100.00% Pervious Area		
Tc (min)	Length (feet)	Slope (ft/ft) Velocity (ft/sec) Capacity (cfs) Description		
8.1	50	0.0600	0.10	<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
2.1	201	0.1000	1.58	<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
10.2	251	Total		

**Summary for Subcatchment 8S: PRE 4.A**

Runoff = 1.44 cfs @ 12.09 hrs, Volume= 0.105 af, Depth= 3.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

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Type III 24-hr 25-year Rainfall=5.40"

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Page 13

Area (sf)	CN	Description			
9,066	74	>75% Grass cover, Good, HSG C			
5,485	96	Gravel surface, HSG C			
2,851	61	>75% Grass cover, Good, HSG B			
17,402	79	Weighted Average			
17,402		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 9S: PRE 5.A**

Runoff = 1.71 cfs @ 12.10 hrs, Volume= 0.134 af, Depth= 1.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description			
15,021	30	Woods, Good, HSG A			
3,541	55	Woods, Good, HSG B			
26,882	77	Woods, Good, HSG D			
45,444	60	Weighted Average			
45,444		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Link 2L: DP-A**Inflow Area = 21.900 ac, 0.12% Impervious, Inflow Depth = 1.37" for 25-year event  
Inflow = 20.15 cfs @ 12.27 hrs, Volume= 2.494 af  
Primary = 20.15 cfs @ 12.27 hrs, Volume= 2.494 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Summary for Link 7L: DP-B**Inflow Area = 2.090 ac, 0.00% Impervious, Inflow Depth = 0.08" for 25-year event  
Inflow = 0.02 cfs @ 15.30 hrs, Volume= 0.014 af  
Primary = 0.02 cfs @ 15.30 hrs, Volume= 0.014 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 100-year Rainfall=7.00"

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Page 14

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment 1S: PRE 1.A**

Runoff Area=499,537 sf 0.23% Impervious Runoff Depth=1.76"  
Flow Length=923' Tc=19.0 min CN=51 Runoff=14.23 cfs 1.678 af

**Subcatchment 3S: PRE 2.A**

Runoff Area=202,863 sf 0.00% Impervious Runoff Depth=2.03"  
Flow Length=572' Tc=19.6 min CN=54 Runoff=6.89 cfs 0.788 af

**Subcatchment 5S: PRE 3.A**

Runoff Area=188,718 sf 0.00% Impervious Runoff Depth=3.94"  
Flow Length=320' Tc=16.7 min CN=73 Runoff=14.42 cfs 1.421 af

**Subcatchment 6S: PRE 1.B**

Runoff Area=91,035 sf 0.00% Impervious Runoff Depth=0.37"  
Flow Length=251' Tc=10.2 min CN=33 Runoff=0.21 cfs 0.065 af

**Subcatchment 8S: PRE 4.A**

Runoff Area=17,402 sf 0.00% Impervious Runoff Depth=4.58"  
Tc=6.0 min CN=79 Runoff=2.09 cfs 0.153 af

**Subcatchment 9S: PRE 5.A**

Runoff Area=45,444 sf 0.00% Impervious Runoff Depth=2.60"  
Tc=6.0 min CN=60 Runoff=3.04 cfs 0.226 af

**Link 2L: DP-A**

Inflow=37.55 cfs 4.266 af  
Primary=37.55 cfs 4.266 af

**Link 7L: DP-B**

Inflow=0.21 cfs 0.065 af  
Primary=0.21 cfs 0.065 af

**Total Runoff Area = 23.990 ac Runoff Volume = 4.331 af Average Runoff Depth = 2.17"  
99.89% Pervious = 23.963 ac 0.11% Impervious = 0.027 ac**

**6092 - PRE**

Type III 24-hr 100-year Rainfall=7.00"

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Page 15

**Summary for Subcatchment 1S: PRE 1.A**

Runoff = 14.23 cfs @ 12.30 hrs, Volume= 1.678 af, Depth= 1.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
88,048	30	Woods, Good, HSG A
404,937	55	Woods, Good, HSG B
5,390	70	Woods, Good, HSG C
1,162	98	Roofs, HSG B
499,537	51	Weighted Average
498,375		99.77% Pervious Area
1,162		0.23% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.6	50	0.0700	0.11		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
11.4	873	0.0650	1.27		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
19.0	923	Total			

**Summary for Subcatchment 3S: PRE 2.A**

Runoff = 6.89 cfs @ 12.30 hrs, Volume= 0.788 af, Depth= 2.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
9,883	30	Woods, Good, HSG A
188,641	55	Woods, Good, HSG B
4,339	77	Woods, Good, HSG D
202,863	54	Weighted Average
202,863		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	50	0.0200	0.07		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
7.1	522	0.0600	1.22		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
19.6	572	Total			

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Type III 24-hr 100-year Rainfall=7.00"

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Page 16

**Summary for Subcatchment 5S: PRE 3.A**

Runoff = 14.42 cfs @ 12.23 hrs, Volume= 1.421 af, Depth= 3.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description			
154,569	77	Woods, Good, HSG D			
34,149	55	Woods, Good, HSG B			
188,718	73	Weighted Average			
188,718		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.7	50	0.0160	0.06		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
3.0	270	0.0900	1.50		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
16.7	320	Total			

**Summary for Subcatchment 6S: PRE 1.B**

Runoff = 0.21 cfs @ 12.48 hrs, Volume= 0.065 af, Depth= 0.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description			
79,057	30	Woods, Good, HSG A			
11,978	55	Woods, Good, HSG B			
91,035	33	Weighted Average			
91,035		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.1	50	0.0600	0.10		<b>Sheet Flow,</b> Woods: Light underbrush n= 0.400 P2= 3.10"
2.1	201	0.1000	1.58		<b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
10.2	251	Total			

**Summary for Subcatchment 8S: PRE 4.A**

Runoff = 2.09 cfs @ 12.09 hrs, Volume= 0.153 af, Depth= 4.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

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Type III 24-hr 100-year Rainfall=7.00"

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Page 17

Area (sf)	CN	Description
9,066	74	>75% Grass cover, Good, HSG C
5,485	96	Gravel surface, HSG C
2,851	61	>75% Grass cover, Good, HSG B
17,402	79	Weighted Average
17,402		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 9S: PRE 5.A

Runoff = 3.04 cfs @ 12.10 hrs, Volume= 0.226 af, Depth= 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
15,021	30	Woods, Good, HSG A
3,541	55	Woods, Good, HSG B
26,882	77	Woods, Good, HSG D
45,444	60	Weighted Average
45,444		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Link 2L: DP-A

Inflow Area = 21.900 ac, 0.12% Impervious, Inflow Depth = 2.34" for 100-year event

Inflow = 37.55 cfs @ 12.26 hrs, Volume= 4.266 af

Primary = 37.55 cfs @ 12.26 hrs, Volume= 4.266 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### Summary for Link 7L: DP-B

Inflow Area = 2.090 ac, 0.00% Impervious, Inflow Depth = 0.37" for 100-year event

Inflow = 0.21 cfs @ 12.48 hrs, Volume= 0.065 af

Primary = 0.21 cfs @ 12.48 hrs, Volume= 0.065 af, Atten= 0%, Lag= 0.0 min

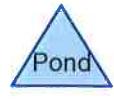
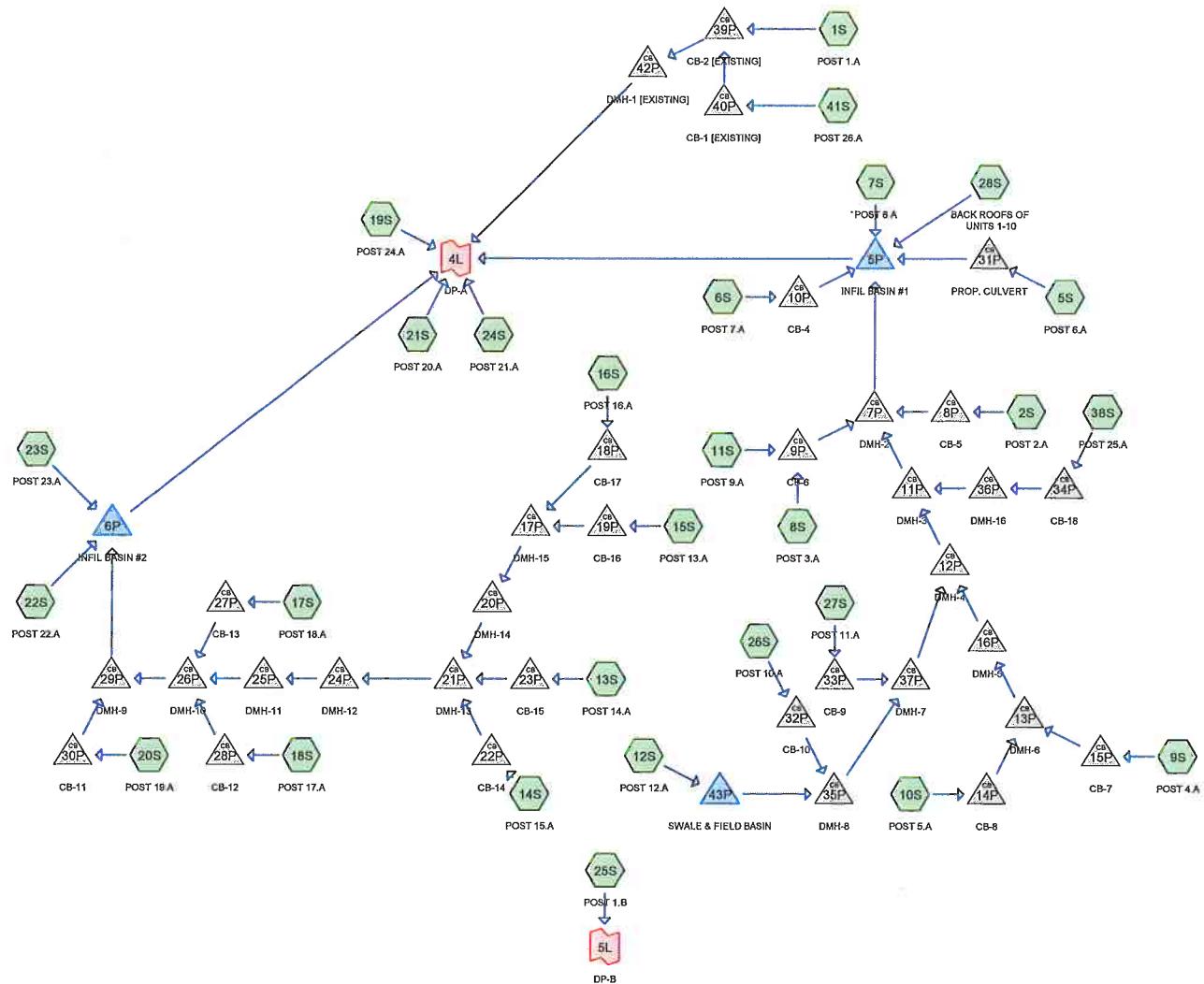
Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



**Appendix E - Proposed Conditions Hydrologic Calculations**

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**Routing Diagram for 6092 - POST Toll Rev4 (current)**  
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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment 1S: POST 1.A</b>	Runoff Area=11,232 sf 80.47% Impervious Runoff Depth=2.35" Tc=6.0 min CN=93 Runoff=0.67 cfs 0.050 af
<b>Subcatchment 2S: POST 2.A</b>	Runoff Area=23,362 sf 69.71% Impervious Runoff Depth=1.83" Tc=6.0 min CN=87 Runoff=1.12 cfs 0.082 af
<b>Subcatchment 5S: POST 6.A</b>	Runoff Area=101,865 sf 19.03% Impervious Runoff Depth=0.48" Flow Length=1,092' Tc=22.1 min CN=63 Runoff=0.59 cfs 0.093 af
<b>Subcatchment 6S: POST 7.A</b>	Runoff Area=21,169 sf 82.05% Impervious Runoff Depth=2.16" Tc=6.0 min CN=91 Runoff=1.19 cfs 0.088 af
<b>Subcatchment 7S: POST 8.A</b>	Runoff Area=21,105 sf 5.32% Impervious Runoff Depth=0.55" Tc=6.0 min UI Adjusted CN=65 Runoff=0.24 cfs 0.022 af
<b>Subcatchment 8S: POST 3.A</b>	Runoff Area=5,197 sf 100.00% Impervious Runoff Depth=2.87" Tc=6.0 min CN=98 Runoff=0.35 cfs 0.029 af
<b>Subcatchment 9S: POST 4.A</b>	Runoff Area=25,089 sf 74.55% Impervious Runoff Depth=1.60" Tc=6.0 min CN=84 Runoff=1.06 cfs 0.077 af
<b>Subcatchment 10S: POST 5.A</b>	Runoff Area=14,640 sf 81.36% Impervious Runoff Depth=1.99" Tc=6.0 min CN=89 Runoff=0.76 cfs 0.056 af
<b>Subcatchment 11S: POST 9.A</b>	Runoff Area=12,906 sf 81.22% Impervious Runoff Depth=2.16" Tc=6.0 min CN=91 Runoff=0.72 cfs 0.053 af
<b>Subcatchment 12S: POST 12.A</b>	Runoff Area=96,627 sf 18.52% Impervious Runoff Depth=0.48" Tc=8.0 min UI Adjusted CN=63 Runoff=0.77 cfs 0.088 af
<b>Subcatchment 13S: POST 14.A</b>	Runoff Area=7,009 sf 98.59% Impervious Runoff Depth=2.76" Tc=6.0 min CN=97 Runoff=0.47 cfs 0.037 af
<b>Subcatchment 14S: POST 15.A</b>	Runoff Area=16,242 sf 76.57% Impervious Runoff Depth=1.60" Tc=6.0 min CN=84 Runoff=0.69 cfs 0.050 af
<b>Subcatchment 15S: POST 13.A</b>	Runoff Area=15,308 sf 48.14% Impervious Runoff Depth=1.26" Tc=6.0 min CN=79 Runoff=0.50 cfs 0.037 af
<b>Subcatchment 16S: POST 16.A</b>	Runoff Area=6,688 sf 98.52% Impervious Runoff Depth=2.76" Tc=6.0 min CN=97 Runoff=0.44 cfs 0.035 af
<b>Subcatchment 17S: POST 18.A</b>	Runoff Area=7,933 sf 98.50% Impervious Runoff Depth=2.76" Tc=6.0 min CN=97 Runoff=0.53 cfs 0.042 af
<b>Subcatchment 18S: POST 17.A</b>	Runoff Area=21,158 sf 76.42% Impervious Runoff Depth=1.99" Tc=6.0 min CN=89 Runoff=1.10 cfs 0.081 af

<b>Subcatchment 19S: POST 24.A</b>	Runoff Area=45,444 sf 0.00% Impervious Runoff Depth=0.92" Tc=6.0 min CN=73 Runoff=1.03 cfs 0.080 af
<b>Subcatchment 20S: POST 19.A</b>	Runoff Area=31,969 sf 74.09% Impervious Runoff Depth=1.91" Tc=6.0 min CN=88 Runoff=1.60 cfs 0.117 af
<b>Subcatchment 21S: POST 20.A</b>	Runoff Area=54,029 sf 13.37% Impervious Runoff Depth=0.72" Tc=6.0 min CN=69 Runoff=0.90 cfs 0.075 af
<b>Subcatchment 22S: POST 22.A</b>	Runoff Area=165,736 sf 13.02% Impervious Runoff Depth=0.40" Flow Length=884' Tc=15.2 min CN=61 Runoff=0.82 cfs 0.128 af
<b>Subcatchment 23S: POST 23.A</b>	Runoff Area=176,164 sf 0.00% Impervious Runoff Depth=0.97" Tc=16.7 min CN=74 Runoff=3.11 cfs 0.328 af
<b>Subcatchment 24S: POST 21.A</b>	Runoff Area=74,028 sf 1.61% Impervious Runoff Depth=0.40" Tc=6.0 min UI Adjusted CN=61 Runoff=0.46 cfs 0.057 af
<b>Subcatchment 25S: POST 1.B</b>	Runoff Area=38,793 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=106' Tc=11.1 min CN=37 Runoff=0.00 cfs 0.000 af
<b>Subcatchment 26S: POST 10.A</b>	Runoff Area=4,620 sf 100.00% Impervious Runoff Depth=2.87" Tc=6.0 min CN=98 Runoff=0.31 cfs 0.025 af
<b>Subcatchment 27S: POST 11.A</b>	Runoff Area=5,180 sf 100.00% Impervious Runoff Depth=2.87" Tc=6.0 min CN=98 Runoff=0.35 cfs 0.028 af
<b>Subcatchment 28S: BACK ROOFS OF</b>	Runoff Area=13,690 sf 100.00% Impervious Runoff Depth=2.87" Tc=6.0 min CN=98 Runoff=0.92 cfs 0.075 af
<b>Subcatchment 38S: POST 25.A</b>	Runoff Area=20,255 sf 62.98% Impervious Runoff Depth=1.60" Tc=6.0 min CN=84 Runoff=0.85 cfs 0.062 af
<b>Subcatchment 41S: POST 26.A</b>	Runoff Area=7,581 sf 81.24% Impervious Runoff Depth=2.35" Tc=6.0 min CN=93 Runoff=0.45 cfs 0.034 af
<b>Pond 5P: INFIL BASIN #1</b>	Peak Elev=312.80' Storage=17,080 cf Inflow=8.51 cfs 0.763 af Discarded=0.30 cfs 0.450 af Primary=0.35 cfs 0.313 af Outflow=0.65 cfs 0.763 af
<b>Pond 6P: INFIL BASIN #2</b>	Peak Elev=316.02' Storage=18,450 cf Inflow=7.45 cfs 0.854 af Discarded=0.60 cfs 0.778 af Primary=0.14 cfs 0.076 af Outflow=0.74 cfs 0.854 af
<b>Pond 7P: DMH-2</b>	Peak Elev=315.13' Inflow=6.08 cfs 0.485 af 24.0" Round Culvert n=0.013 L=60.0' S=0.0200 '/' Outflow=6.08 cfs 0.485 af
<b>Pond 8P: CB-5</b>	Peak Elev=315.89' Inflow=1.12 cfs 0.082 af 12.0" Round Culvert n=0.013 L=20.0' S=0.0180 '/' Outflow=1.12 cfs 0.082 af
<b>Pond 9P: CB-6</b>	Peak Elev=316.27' Inflow=1.07 cfs 0.082 af 12.0" Round Culvert n=0.013 L=20.0' S=0.0375 '/' Outflow=1.07 cfs 0.082 af

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Page 4

**Pond 10P: CB-4**Peak Elev=315.48' Inflow=1.19 cfs 0.088 af  
12.0" Round Culvert n=0.013 L=25.0' S=0.0168 '/' Outflow=1.19 cfs 0.088 af**Pond 11P: DMH-3**Peak Elev=318.06' Inflow=3.90 cfs 0.321 af  
24.0" Round Culvert n=0.013 L=63.0' S=0.0349 '/' Outflow=3.90 cfs 0.321 af**Pond 12P: DMH-4**Peak Elev=322.79' Inflow=3.05 cfs 0.259 af  
24.0" Round Culvert n=0.013 L=99.0' S=0.0481 '/' Outflow=3.05 cfs 0.259 af**Pond 13P: DMH-6**Peak Elev=335.95' Inflow=1.82 cfs 0.132 af  
18.0" Round Culvert n=0.013 L=124.0' S=0.0576 '/' Outflow=1.82 cfs 0.132 af**Pond 14P: CB-8**Peak Elev=337.06' Inflow=0.76 cfs 0.056 af  
12.0" Round Culvert n=0.013 L=14.0' S=0.0571 '/' Outflow=0.76 cfs 0.056 af**Pond 15P: CB-7**Peak Elev=336.56' Inflow=1.06 cfs 0.077 af  
12.0" Round Culvert n=0.013 L=10.0' S=0.0200 '/' Outflow=1.06 cfs 0.077 af**Pond 16P: DMH-5**Peak Elev=328.71' Inflow=1.82 cfs 0.132 af  
18.0" Round Culvert n=0.013 L=91.0' S=0.0599 '/' Outflow=1.82 cfs 0.132 af**Pond 17P: DMH-15**Peak Elev=336.12' Inflow=0.94 cfs 0.072 af  
12.0" Round Culvert n=0.013 L=69.0' S=0.0100 '/' Outflow=0.94 cfs 0.072 af**Pond 18P: CB-17**Peak Elev=336.25' Inflow=0.44 cfs 0.035 af  
12.0" Round Culvert n=0.013 L=16.0' S=0.0100 '/' Outflow=0.44 cfs 0.035 af**Pond 19P: CB-16**Peak Elev=336.27' Inflow=0.50 cfs 0.037 af  
12.0" Round Culvert n=0.013 L=16.0' S=0.0100 '/' Outflow=0.50 cfs 0.037 af**Pond 20P: DMH-14**Peak Elev=335.33' Inflow=0.94 cfs 0.072 af  
12.0" Round Culvert n=0.013 L=88.0' S=0.0100 '/' Outflow=0.94 cfs 0.072 af**Pond 21P: DMH-13**Peak Elev=334.14' Inflow=2.09 cfs 0.159 af  
18.0" Round Culvert n=0.013 L=136.0' S=0.0200 '/' Outflow=2.09 cfs 0.159 af**Pond 22P: CB-14**Peak Elev=338.69' Inflow=0.69 cfs 0.050 af  
12.0" Round Culvert n=0.013 L=17.0' S=0.0124 '/' Outflow=0.69 cfs 0.050 af**Pond 23P: CB-15**Peak Elev=338.70' Inflow=0.47 cfs 0.037 af  
12.0" Round Culvert n=0.013 L=28.0' S=0.0111 '/' Outflow=0.47 cfs 0.037 af**Pond 24P: DMH-12**Peak Elev=331.32' Inflow=2.09 cfs 0.159 af  
18.0" Round Culvert n=0.013 L=99.0' S=0.0275 '/' Outflow=2.09 cfs 0.159 af**Pond 25P: DMH-11**Peak Elev=328.50' Inflow=2.09 cfs 0.159 af  
18.0" Round Culvert n=0.013 L=76.0' S=0.0753 '/' Outflow=2.09 cfs 0.159 af**Pond 26P: DMH-10**Peak Elev=322.99' Inflow=3.72 cfs 0.281 af  
18.0" Round Culvert n=0.013 L=174.0' S=0.0294 '/' Outflow=3.72 cfs 0.281 af

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Page 5

Pond 27P: CB-13	Peak Elev=323.92' Inflow=0.53 cfs 0.042 af 12.0" Round Culvert n=0.013 L=14.0' S=0.0200 '/' Outflow=0.53 cfs 0.042 af
Pond 28P: CB-12	Peak Elev=324.14' Inflow=1.10 cfs 0.081 af 12.0" Round Culvert n=0.013 L=14.0' S=0.0200 '/' Outflow=1.10 cfs 0.081 af
Pond 29P: DMH-9	Peak Elev=317.53' Inflow=5.33 cfs 0.398 af 24.0" Round Culvert n=0.013 L=62.0' S=0.0050 '/' Outflow=5.33 cfs 0.398 af
Pond 30P: CB-11	Peak Elev=317.96' Inflow=1.60 cfs 0.117 af 18.0" Round Culvert n=0.013 L=49.0' S=0.0100 '/' Outflow=1.60 cfs 0.117 af
Pond 31P: PROP. CULVERT	Peak Elev=314.41' Inflow=0.59 cfs 0.093 af 15.0" Round Culvert n=0.013 L=75.8' S=0.0132 '/' Outflow=0.59 cfs 0.093 af
Pond 32P: CB-10	Peak Elev=323.30' Inflow=0.31 cfs 0.025 af 18.0" Round Culvert n=0.013 L=55.0' S=0.0049 '/' Outflow=0.31 cfs 0.025 af
Pond 33P: CB-9	Peak Elev=323.29' Inflow=0.35 cfs 0.028 af 18.0" Round Culvert n=0.013 L=7.0' S=0.0686 '/' Outflow=0.35 cfs 0.028 af
Pond 34P: CB-18	Peak Elev=320.54' Inflow=0.85 cfs 0.062 af 12.0" Round Culvert n=0.013 L=16.0' S=0.0600 '/' Outflow=0.85 cfs 0.062 af
Pond 35P: DMH-8	Peak Elev=323.21' Inflow=0.99 cfs 0.099 af 24.0" Round Culvert n=0.013 L=21.0' S=0.0052 '/' Outflow=0.99 cfs 0.099 af
Pond 36P: DMH-16	Peak Elev=319.48' Inflow=0.85 cfs 0.062 af 12.0" Round Culvert n=0.013 L=21.0' S=0.0829 '/' Outflow=0.85 cfs 0.062 af
Pond 37P: DMH-7	Peak Elev=323.07' Inflow=1.27 cfs 0.127 af 24.0" Round Culvert n=0.013 L=78.1' S=0.0059 '/' Outflow=1.27 cfs 0.127 af
Pond 39P: CB-2 [EXISTING]	Peak Elev=310.86' Inflow=1.13 cfs 0.085 af 12.0" Round Culvert n=0.013 L=49.0' S=0.0010 '/' Outflow=1.13 cfs 0.085 af
Pond 40P: CB-1 [EXISTING]	Peak Elev=311.13' Inflow=0.45 cfs 0.034 af 12.0" Round Culvert n=0.013 L=17.0' S=0.0353 '/' Outflow=0.45 cfs 0.034 af
Pond 42P: DMH-1 [EXISTING]	Peak Elev=310.64' Inflow=1.13 cfs 0.085 af 12.0" Round Culvert n=0.013 L=92.0' S=0.0215 '/' Outflow=1.13 cfs 0.085 af
Pond 43P: SWALE & FIELD BASIN	Peak Elev=328.07' Storage=44 cf Inflow=0.77 cfs 0.088 af Discarded=0.02 cfs 0.015 af Primary=0.76 cfs 0.073 af Outflow=0.78 cfs 0.088 af
Link 4L: DP-A	Inflow=3.66 cfs 0.686 af Primary=3.66 cfs 0.686 af
Link 5L: DP-B	Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af

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Type III 24-hr 2-year Rainfall=3.10"

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Page 6

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Total Runoff Area = 23.990 ac   Runoff Volume = 1.928 af   Average Runoff Depth = 0.96"  
73.13% Pervious = 17.545 ac   26.87% Impervious = 6.446 ac

**Summary for Subcatchment 1S: POST 1.A**

Runoff = 0.67 cfs @ 12.09 hrs, Volume= 0.050 af, Depth= 2.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
4,679	98	Paved parking, HSG C
1,673	98	Paved parking, HSG B
2,343	98	Unconnected pavement, HSG C
343	98	Unconnected pavement, HSG B
395	61	>75% Grass cover, Good, HSG B
1,799	74	>75% Grass cover, Good, HSG C

11,232	93	Weighted Average
2,194		19.53% Pervious Area
9,038		80.47% Impervious Area
2,686		29.72% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 2S: POST 2.A**

Runoff = 1.12 cfs @ 12.09 hrs, Volume= 0.082 af, Depth= 1.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
757	98	Unconnected pavement, HSG B
8,275	98	Paved parking, HSG B
7,076	61	>75% Grass cover, Good, HSG B

23,362	87	Weighted Average
7,076		30.29% Pervious Area
16,286		69.71% Impervious Area
757		4.65% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 5S: POST 6.A

Runoff = 0.59 cfs @ 12.42 hrs, Volume= 0.093 af, Depth= 0.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
3,460	70	Woods, Good, HSG C
1,929	74	>75% Grass cover, Good, HSG C
11,762	55	Woods, Good, HSG B
43,358	61	>75% Grass cover, Good, HSG B
10,565	98	Roofs, HSG B
1,187	98	Unconnected pavement, HSG B
6,891	98	Roofs, HSG A
741	98	Unconnected pavement, HSG A
21,972	39	>75% Grass cover, Good, HSG A
101,865	63	Weighted Average
82,481		80.97% Pervious Area
19,384		19.03% Impervious Area
1,928		9.95% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3	50	0.0060	0.09		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 3.10"
11.8	859	0.0300	1.21		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
0.9	100	0.0700	1.85		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
0.1	83	0.0200	10.18	31.99	<b>Pipe Channel,</b> 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
22.1	1,092	Total			

### Summary for Subcatchment 6S: POST 7.A

Runoff = 1.19 cfs @ 12.09 hrs, Volume= 0.088 af, Depth= 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
9,604	98	Paved parking, HSG B
511	98	Unconnected pavement, HSG B
3,800	61	>75% Grass cover, Good, HSG B
21,169	91	Weighted Average
3,800		17.95% Pervious Area
17,369		82.05% Impervious Area
511		2.94% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 7S: POST 8.A**

Runoff = 0.24 cfs @ 12.11 hrs, Volume= 0.022 af, Depth= 0.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Adj	Description
120	98		Roofs, HSG B
1,579	96		Gravel surface, HSG B
1,003	98		Unconnected pavement, HSG B
18,403	61		>75% Grass cover, Good, HSG B
21,105	66	65	Weighted Average, UI Adjusted
19,982			94.68% Pervious Area
1,123			5.32% Impervious Area
1,003			89.31% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 8S: POST 3.A**

Runoff = 0.35 cfs @ 12.09 hrs, Volume= 0.029 af, Depth= 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
5,197	98	Paved parking, HSG B
5,197		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 9S: POST 4.A**

Runoff = 1.06 cfs @ 12.09 hrs, Volume= 0.077 af, Depth= 1.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

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Type III 24-hr 2-year Rainfall=3.10"

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Page 10

Area (sf)	CN	Description
2,372	98	Roofs, HSG B
150	98	Unconnected pavement, HSG B
2,521	98	Paved parking, HSG B
1,636	61	>75% Grass cover, Good, HSG B
6,091	98	Roofs, HSG A
649	98	Unconnected pavement, HSG A
6,920	98	Paved parking, HSG A
4,750	39	>75% Grass cover, Good, HSG A
25,089	84	Weighted Average
6,386		25.45% Pervious Area
18,703		74.55% Impervious Area
799		4.27% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 10S: POST 5.A**

Runoff = 0.76 cfs @ 12.09 hrs, Volume= 0.056 af, Depth= 1.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
2,418	98	Roofs, HSG B
338	98	Unconnected pavement, HSG B
2,578	98	Paved parking, HSG B
1,011	61	>75% Grass cover, Good, HSG B
2,418	98	Roofs, HSG A
494	98	Unconnected pavement, HSG A
3,665	98	Paved parking, HSG A
1,718	39	>75% Grass cover, Good, HSG A
14,640	89	Weighted Average
2,729		18.64% Pervious Area
11,911		81.36% Impervious Area
832		6.99% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 11S: POST 9.A**

Runoff = 0.72 cfs @ 12.09 hrs, Volume= 0.053 af, Depth= 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

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Type III 24-hr 2-year Rainfall=3.10"

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Page 11

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
7,363	98	Paved parking, HSG B			
701	98	Unconnected pavement, HSG B			
2,424	61	>75% Grass cover, Good, HSG B			
12,906	91	Weighted Average			
2,424		18.78% Pervious Area			
10,482		81.22% Impervious Area			
701		6.69% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 12S: POST 12.A**

Runoff = 0.77 cfs @ 12.16 hrs, Volume= 0.088 af, Depth= 0.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Adj	Description		
9,152	98		Roofs, HSG B		
2,471	98		Unconnected pavement, HSG B		
59,845	61		>75% Grass cover, Good, HSG B		
4,539	98		Roofs, HSG A		
1,734	98		Unconnected pavement, HSG A		
18,886	39		>75% Grass cover, Good, HSG A		
96,627	64	63	Weighted Average, UI Adjusted		
78,731			81.48% Pervious Area		
17,896			18.52% Impervious Area		
4,205			23.50% Unconnected		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry,

**Summary for Subcatchment 13S: POST 14.A**

Runoff = 0.47 cfs @ 12.09 hrs, Volume= 0.037 af, Depth= 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

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Type III 24-hr 2-year Rainfall=3.10"

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Page 12

Area (sf)	CN	Description			
1,784	98	Roofs, HSG A			
3,387	98	Paved parking, HSG A			
99	39	>75% Grass cover, Good, HSG A			
1,105	98	Paved parking, HSG B			
634	98	Roofs, HSG B			
7,009	97	Weighted Average			
99		1.41% Pervious Area			
6,910		98.59% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 14S: POST 15.A**

Runoff = 0.69 cfs @ 12.09 hrs, Volume= 0.050 af, Depth= 1.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description			
1,135	98	Paved parking, HSG B			
6,045	98	Roofs, HSG A			
496	98	Unconnected pavement, HSG A			
4,760	98	Paved parking, HSG A			
3,806	39	>75% Grass cover, Good, HSG A			
16,242	84	Weighted Average			
3,806		23.43% Pervious Area			
12,436		76.57% Impervious Area			
496		3.99% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 15S: POST 13.A**

Runoff = 0.50 cfs @ 12.10 hrs, Volume= 0.037 af, Depth= 1.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

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Type III 24-hr 2-year Rainfall=3.10"

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Page 13

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
1,547	98	Unconnected pavement, HSG B			
3,405	98	Paved parking, HSG B			
7,938	61	>75% Grass cover, Good, HSG B			
15,308	79	Weighted Average			
7,938		51.86% Pervious Area			
7,370		48.14% Impervious Area			
1,547		20.99% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 16S: POST 16.A**

Runoff = 0.44 cfs @ 12.09 hrs, Volume= 0.035 af, Depth= 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
4,171	98	Paved parking, HSG B			
99	61	>75% Grass cover, Good, HSG B			
6,688	97	Weighted Average			
99		1.48% Pervious Area			
6,589		98.52% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 17S: POST 18.A**

Runoff = 0.53 cfs @ 12.09 hrs, Volume= 0.042 af, Depth= 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
5,396	98	Paved parking, HSG B			
119	61	>75% Grass cover, Good, HSG B			
7,933	97	Weighted Average			
119		1.50% Pervious Area			
7,814		98.50% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 18S: POST 17.A**

Runoff = 1.10 cfs @ 12.09 hrs, Volume= 0.081 af, Depth= 1.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
7,406	98	Paved parking, HSG B
1,509	98	Unconnected pavement, HSG B
4,989	61	>75% Grass cover, Good, HSG B
21,158	89	Weighted Average
4,989		23.58% Pervious Area
16,169		76.42% Impervious Area
1,509		9.33% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 19S: POST 24.A**

Runoff = 1.03 cfs @ 12.10 hrs, Volume= 0.080 af, Depth= 0.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
16,251	77	Woods, Good, HSG D
5,201	96	Gravel surface, HSG D
5,436	80	>75% Grass cover, Good, HSG D
6,221	30	Woods, Good, HSG A
3,810	39	>75% Grass cover, Good, HSG A
4,986	96	Gravel surface, HSG A
1,269	61	>75% Grass cover, Good, HSG B
2,270	96	Gravel surface, HSG B
45,444	73	Weighted Average
45,444		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

### Summary for Subcatchment 20S: POST 19.A

Runoff = 1.60 cfs @ 12.09 hrs, Volume= 0.117 af, Depth= 1.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
15,358	98	Paved parking, HSG B
1,073	98	Unconnected pavement, HSG B
8,284	61	>75% Grass cover, Good, HSG B
31,969	88	Weighted Average
8,284		25.91% Pervious Area
23,685		74.09% Impervious Area
1,073		4.53% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 21S: POST 20.A

Runoff = 0.90 cfs @ 12.11 hrs, Volume= 0.075 af, Depth= 0.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
5,596	98	Roofs, HSG B
1,630	98	Unconnected pavement, HSG B
1,749	96	Gravel surface, HSG B
40,318	61	>75% Grass cover, Good, HSG B
1,861	96	Gravel surface, HSG D
2,875	80	>75% Grass cover, Good, HSG D
54,029	69	Weighted Average
46,803		86.63% Pervious Area
7,226		13.37% Impervious Area
1,630		22.56% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 22S: POST 22.A

Runoff = 0.82 cfs @ 12.32 hrs, Volume= 0.128 af, Depth= 0.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
6,845	98	Roofs, HSG A
819	98	Unconnected pavement, HSG A
5,708	30	Woods, Good, HSG A
30,256	39	>75% Grass cover, Good, HSG A
11,252	98	Roofs, HSG B
2,667	98	Unconnected pavement, HSG B
88,631	61	>75% Grass cover, Good, HSG B
14,680	55	Woods, Good, HSG B
2,251	96	Gravel surface, HSG B
831	96	Gravel surface, HSG D
1,334	80	>75% Grass cover, Good, HSG D
462	77	Woods, Good, HSG D
165,736	61	Weighted Average
144,153		86.98% Pervious Area
21,583		13.02% Impervious Area
3,486		16.15% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7	50	0.0200	0.15		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 3.10"
3.7	348	0.0500	1.57		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
5.8	486	0.0400	1.40		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
15.2	884	Total			

### Summary for Subcatchment 23S: POST 23.A

Runoff = 3.11 cfs @ 12.25 hrs, Volume= 0.328 af, Depth= 0.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
24,614	55	Woods, Good, HSG B
1,206	96	Gravel surface, HSG D
1,599	80	>75% Grass cover, Good, HSG D
148,745	77	Woods, Good, HSG D
176,164	74	Weighted Average
176,164		100.00% Pervious Area

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Type III 24-hr 2-year Rainfall=3.10"

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Page 17

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.7					Direct Entry,

**Summary for Subcatchment 24S: POST 21.A**

Runoff = 0.46 cfs @ 12.14 hrs, Volume= 0.057 af, Depth= 0.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Adj	Description
1,194	98		Unconnected pavement, HSG B
248	96		Gravel surface, HSG B
72,586	61		>75% Grass cover, Good, HSG B
74,028	62	61	Weighted Average, UI Adjusted
72,834			98.39% Pervious Area
1,194			1.61% Impervious Area
1,194			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 25S: POST 1.B**

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
7,624	55	Woods, Good, HSG B
176	61	>75% Grass cover, Good, HSG B
24,732	30	Woods, Good, HSG A
6,261	39	>75% Grass cover, Good, HSG A
38,793	37	Weighted Average
38,793		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	50	0.0300	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.10"
0.4	56	0.2700	2.60		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
11.1	106	Total			

**Summary for Subcatchment 26S: POST 10.A**

Runoff = 0.31 cfs @ 12.09 hrs, Volume= 0.025 af, Depth= 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
4,620	98	Paved parking, HSG B
4,620		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 27S: POST 11.A**

Runoff = 0.35 cfs @ 12.09 hrs, Volume= 0.028 af, Depth= 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
5,180	98	Paved parking, HSG B
5,180		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 28S: BACK ROOFS OF UNITS 1-10**

Runoff = 0.92 cfs @ 12.09 hrs, Volume= 0.075 af, Depth= 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
13,690	98	Roofs, HSG B
13,690		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 38S: POST 25.A

Runoff = 0.85 cfs @ 12.09 hrs, Volume= 0.062 af, Depth= 1.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
1,914	98	Roofs, HSG A
10,842	98	Unconnected pavement, HSG B
7,499	61	>75% Grass cover, Good, HSG B
20,255	84	Weighted Average
7,499		37.02% Pervious Area
12,756		62.98% Impervious Area
10,842		85.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 41S: POST 26.A

Runoff = 0.45 cfs @ 12.09 hrs, Volume= 0.034 af, Depth= 2.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2-year Rainfall=3.10"

Area (sf)	CN	Description
4,658	98	Paved parking, HSG C
1,501	98	Paved parking, HSG B
352	61	>75% Grass cover, Good, HSG B
1,070	74	>75% Grass cover, Good, HSG C
7,581	93	Weighted Average
1,422		18.76% Pervious Area
6,159		81.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Pond 5P: INFIL BASIN #1

Inflow Area = 8.395 ac, 42.27% Impervious, Inflow Depth = 1.09" for 2-year event

Inflow = 8.51 cfs @ 12.10 hrs, Volume= 0.763 af

Outflow = 0.65 cfs @ 14.44 hrs, Volume= 0.763 af, Atten= 92%, Lag= 140.7 min

Discarded = 0.30 cfs @ 14.44 hrs, Volume= 0.450 af

Primary = 0.35 cfs @ 14.44 hrs, Volume= 0.313 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 2-year Rainfall=3.10"

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Page 20

Peak Elev= 312.80' @ 14.44 hrs Surf.Area= 6,219 sf Storage= 17,080 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 386.4 min ( 1,212.2 - 825.8 )

Volume	Invert	Avail.Storage	Storage Description		
#1	309.00'	42,092 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
309.00	2,439	200.1	0	0	2,439
310.00	3,839	242.8	3,113	3,113	3,960
312.00	5,491	281.0	9,281	12,394	5,636
314.00	7,393	320.0	12,837	25,230	7,596
316.00	9,513	355.0	16,862	42,092	9,594

Device	Routing	Invert	Outlet Devices
#1	Primary	309.00'	<b>24.0" Round Culvert</b> L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 309.00' / 308.00' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Discarded	309.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 306.73'
#3	Primary	315.60'	<b>20.0' long x 12.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.57 2.62 2.70 2.67 2.66 2.67 2.66 2.64
#4	Device 1	315.00'	<b>16.0' long Sharp-Crested Rectangular Weir</b> 0 End Contraction(s)
#5	Device 1	310.45'	<b>3.0" Vert. Orifice/Grate</b> C= 0.600

Discarded OutFlow Max=0.30 cfs @ 14.44 hrs HW=312.80' (Free Discharge)

↑ 2=Exfiltration (Controls 0.30 cfs)

Primary OutFlow Max=0.35 cfs @ 14.44 hrs HW=312.80' TW=0.00' (Dynamic Tailwater)

↑ 1=Culvert (Passes 0.35 cfs of 19.99 cfs potential flow)  
 ↑ 4=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)  
 ↓ 5=Orifice/Grate (Orifice Controls 0.35 cfs @ 7.18 fps)  
 3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

**Summary for Pond 6P: INFIL BASIN #2**

Inflow Area = 10.289 ac, 22.88% Impervious, Inflow Depth = 1.00" for 2-year event

Inflow = 7.45 cfs @ 12.12 hrs, Volume= 0.854 af

Outflow = 0.74 cfs @ 14.79 hrs, Volume= 0.854 af, Atten= 90%, Lag= 160.1 min

Discarded = 0.60 cfs @ 14.79 hrs, Volume= 0.778 af

Primary = 0.14 cfs @ 14.79 hrs, Volume= 0.076 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 316.02' @ 14.79 hrs Surf.Area= 18,103 sf Storage= 18,450 cf

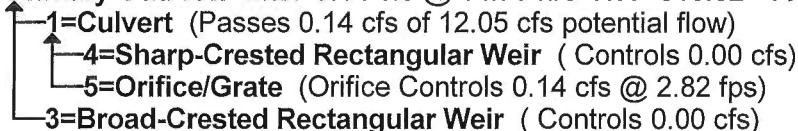
Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 375.9 min ( 1,226.3 - 850.4 )

Volume	Invert	Avail.Storage	Storage Description
#			Custom Stage Data (Irregular) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)
314.00	2,513	223.0	0
316.00	17,939	667.0	18,111
318.00	39,591	937.0	56,120
319.00	49,868	1,019.0	44,631
			Cum.Store (cubic-feet)
			118,862
			Wet.Area (sq-ft)
			2,513
			33,971
			68,472
			81,273
Device	Routing	Invert	Outlet Devices
#1	Primary	314.00'	<b>24.0" Round Culvert</b> L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.00' / 312.60' S= 0.0280 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Discarded	314.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 312.00'
#3	Primary	317.80'	<b>25.0' long x 20.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#4	Device 1	317.40'	<b>16.0' long Sharp-Crested Rectangular Weir</b> 0 End Contraction(s)
#5	Device 1	315.55'	<b>3.0" Vert. Orifice/Grate</b> C= 0.600

**Discarded OutFlow** Max=0.60 cfs @ 14.79 hrs HW=316.02' (Free Discharge)  
 ↗ 2=Exfiltration (Controls 0.60 cfs)

**Primary OutFlow** Max=0.14 cfs @ 14.79 hrs HW=316.02' TW=0.00' (Dynamic Tailwater)



### Summary for Pond 7P: DMH-2

Inflow Area = 4.772 ac, 49.56% Impervious, Inflow Depth = 1.22" for 2-year event  
 Inflow = 6.08 cfs @ 12.10 hrs, Volume= 0.485 af  
 Outflow = 6.08 cfs @ 12.10 hrs, Volume= 0.485 af, Atten= 0%, Lag= 0.0 min  
 Primary = 6.08 cfs @ 12.10 hrs, Volume= 0.485 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 315.13' @ 12.10 hrs

Flood Elev= 319.42'

Device	Routing	Invert	Outlet Devices
#1	Primary	313.90'	<b>24.0" Round Culvert</b> L= 60.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 313.90' / 312.70' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=6.06 cfs @ 12.10 hrs HW=315.13' TW=311.09' (Dynamic Tailwater)  
 ↑  
 1=Culvert (Inlet Controls 6.06 cfs @ 2.98 fps)

### Summary for Pond 8P: CB-5

Inflow Area = 0.536 ac, 69.71% Impervious, Inflow Depth = 1.83" for 2-year event  
 Inflow = 1.12 cfs @ 12.09 hrs, Volume= 0.082 af  
 Outflow = 1.12 cfs @ 12.09 hrs, Volume= 0.082 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.12 cfs @ 12.09 hrs, Volume= 0.082 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 315.89' @ 12.09 hrs  
 Flood Elev= 319.26'

Device	Routing	Invert	Outlet Devices
#1	Primary	315.26'	<b>12.0" Round Culvert</b> L= 20.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 315.26' / 314.90' S= 0.0180 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.10 cfs @ 12.09 hrs HW=315.89' TW=315.12' (Dynamic Tailwater)  
 ↑  
 1=Culvert (Inlet Controls 1.10 cfs @ 2.13 fps)

### Summary for Pond 9P: CB-6

Inflow Area = 0.416 ac, 86.61% Impervious, Inflow Depth = 2.37" for 2-year event  
 Inflow = 1.07 cfs @ 12.09 hrs, Volume= 0.082 af  
 Outflow = 1.07 cfs @ 12.09 hrs, Volume= 0.082 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.07 cfs @ 12.09 hrs, Volume= 0.082 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 316.27' @ 12.09 hrs  
 Flood Elev= 319.65'

Device	Routing	Invert	Outlet Devices
#1	Primary	315.65'	<b>12.0" Round Culvert</b> L= 20.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 315.65' / 314.90' S= 0.0375 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.05 cfs @ 12.09 hrs HW=316.26' TW=315.11' (Dynamic Tailwater)  
 ↑  
 1=Culvert (Inlet Controls 1.05 cfs @ 2.10 fps)

### Summary for Pond 10P: CB-4

Inflow Area = 0.486 ac, 82.05% Impervious, Inflow Depth = 2.16" for 2-year event  
 Inflow = 1.19 cfs @ 12.09 hrs, Volume= 0.088 af  
 Outflow = 1.19 cfs @ 12.09 hrs, Volume= 0.088 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.19 cfs @ 12.09 hrs, Volume= 0.088 af

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Page 23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 315.48' @ 12.09 hrs

Flood Elev= 318.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	314.82'	<b>12.0" Round Culvert</b> L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.82' / 314.40' S= 0.0168 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.16 cfs @ 12.09 hrs HW=315.47' TW=311.03' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 1.16 cfs @ 2.16 fps)

**Summary for Pond 11P: DMH-3**

Inflow Area = 3.820 ac, 42.71% Impervious, Inflow Depth = 1.01" for 2-year event  
 Inflow = 3.90 cfs @ 12.10 hrs, Volume= 0.321 af  
 Outflow = 3.90 cfs @ 12.10 hrs, Volume= 0.321 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.90 cfs @ 12.10 hrs, Volume= 0.321 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 318.06' @ 12.10 hrs

Flood Elev= 321.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	317.10'	<b>24.0" Round Culvert</b> L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 317.10' / 314.90' S= 0.0349 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.86 cfs @ 12.10 hrs HW=318.05' TW=315.13' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 3.86 cfs @ 2.62 fps)

**Summary for Pond 12P: DMH-4**

Inflow Area = 3.355 ac, 39.90% Impervious, Inflow Depth = 0.93" for 2-year event  
 Inflow = 3.05 cfs @ 12.11 hrs, Volume= 0.259 af  
 Outflow = 3.05 cfs @ 12.11 hrs, Volume= 0.259 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.05 cfs @ 12.11 hrs, Volume= 0.259 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 322.79' @ 12.11 hrs

Flood Elev= 326.96'

Device	Routing	Invert	Outlet Devices
#1	Primary	321.96'	<b>24.0" Round Culvert</b> L= 99.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 321.96' / 317.20' S= 0.0481 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.00 cfs @ 12.11 hrs HW=322.79' TW=318.05' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 3.00 cfs @ 2.45 fps)

### Summary for Pond 13P: DMH-6

Inflow Area = 0.912 ac, 77.06% Impervious, Inflow Depth = 1.74" for 2-year event  
 Inflow = 1.82 cfs @ 12.09 hrs, Volume= 0.132 af  
 Outflow = 1.82 cfs @ 12.09 hrs, Volume= 0.132 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.82 cfs @ 12.09 hrs, Volume= 0.132 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 335.95' @ 12.09 hrs  
 Flood Elev= 340.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.25'	<b>18.0" Round Culvert</b> L= 124.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.25' / 328.11' S= 0.0576 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=1.79 cfs @ 12.09 hrs HW=335.94' TW=328.70' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 1.79 cfs @ 2.24 fps)

### Summary for Pond 14P: CB-8

Inflow Area = 0.336 ac, 81.36% Impervious, Inflow Depth = 1.99" for 2-year event  
 Inflow = 0.76 cfs @ 12.09 hrs, Volume= 0.056 af  
 Outflow = 0.76 cfs @ 12.09 hrs, Volume= 0.056 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.76 cfs @ 12.09 hrs, Volume= 0.056 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 337.06' @ 12.09 hrs  
 Flood Elev= 340.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	336.55'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 336.55' / 335.75' S= 0.0571 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.75 cfs @ 12.09 hrs HW=337.05' TW=335.94' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 0.75 cfs @ 1.90 fps)

### Summary for Pond 15P: CB-7

Inflow Area = 0.576 ac, 74.55% Impervious, Inflow Depth = 1.60" for 2-year event  
 Inflow = 1.06 cfs @ 12.09 hrs, Volume= 0.077 af  
 Outflow = 1.06 cfs @ 12.09 hrs, Volume= 0.077 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.06 cfs @ 12.09 hrs, Volume= 0.077 af

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Page 25

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 336.56' @ 12.09 hrs

Flood Elev= 339.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.95'	<b>12.0" Round Culvert</b> L= 10.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.95' / 335.75' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.04 cfs @ 12.09 hrs HW=336.56' TW=335.94' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 1.04 cfs @ 2.09 fps)

**Summary for Pond 16P: DMH-5**

Inflow Area = 0.912 ac, 77.06% Impervious, Inflow Depth = 1.74" for 2-year event  
 Inflow = 1.82 cfs @ 12.09 hrs, Volume= 0.132 af  
 Outflow = 1.82 cfs @ 12.09 hrs, Volume= 0.132 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.82 cfs @ 12.09 hrs, Volume= 0.132 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 328.71' @ 12.09 hrs

Flood Elev= 332.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	328.01'	<b>18.0" Round Culvert</b> L= 91.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 328.01' / 322.56' S= 0.0599 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=1.79 cfs @ 12.09 hrs HW=328.70' TW=322.78' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 1.79 cfs @ 2.24 fps)

**Summary for Pond 17P: DMH-15**

Inflow Area = 0.505 ac, 63.46% Impervious, Inflow Depth = 1.72" for 2-year event  
 Inflow = 0.94 cfs @ 12.09 hrs, Volume= 0.072 af  
 Outflow = 0.94 cfs @ 12.09 hrs, Volume= 0.072 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.94 cfs @ 12.09 hrs, Volume= 0.072 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 336.12' @ 12.09 hrs

Flood Elev= 340.37'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.55'	<b>12.0" Round Culvert</b> L= 69.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.55' / 334.86' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.93 cfs @ 12.09 hrs HW=336.12' TW=335.33' (Dynamic Tailwater)  
 ↑  
 ↗1=Culvert (Inlet Controls 0.93 cfs @ 2.02 fps)

### Summary for Pond 18P: CB-17

Inflow Area = 0.154 ac, 98.52% Impervious, Inflow Depth = 2.76" for 2-year event  
 Inflow = 0.44 cfs @ 12.09 hrs, Volume= 0.035 af  
 Outflow = 0.44 cfs @ 12.09 hrs, Volume= 0.035 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.44 cfs @ 12.09 hrs, Volume= 0.035 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 336.25' @ 12.11 hrs  
 Flood Elev= 339.81'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.81'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.81' / 335.65' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.36 cfs @ 12.09 hrs HW=336.23' TW=336.11' (Dynamic Tailwater)  
 ↑  
 ↗1=Culvert (Outlet Controls 0.36 cfs @ 1.70 fps)

### Summary for Pond 19P: CB-16

Inflow Area = 0.351 ac, 48.14% Impervious, Inflow Depth = 1.26" for 2-year event  
 Inflow = 0.50 cfs @ 12.10 hrs, Volume= 0.037 af  
 Outflow = 0.50 cfs @ 12.10 hrs, Volume= 0.037 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.50 cfs @ 12.10 hrs, Volume= 0.037 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 336.27' @ 12.12 hrs  
 Flood Elev= 339.81'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.81'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.81' / 335.65' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.44 cfs @ 12.10 hrs HW=336.26' TW=336.12' (Dynamic Tailwater)  
 ↑  
 ↗1=Culvert (Outlet Controls 0.44 cfs @ 1.88 fps)

### Summary for Pond 20P: DMH-14

Inflow Area = 0.505 ac, 63.46% Impervious, Inflow Depth = 1.72" for 2-year event  
 Inflow = 0.94 cfs @ 12.09 hrs, Volume= 0.072 af  
 Outflow = 0.94 cfs @ 12.09 hrs, Volume= 0.072 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.94 cfs @ 12.09 hrs, Volume= 0.072 af

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Page 27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 335.33' @ 12.09 hrs

Flood Elev= 341.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	334.76'	<b>12.0" Round Culvert</b> L= 88.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 334.76' / 333.88' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.93 cfs @ 12.09 hrs HW=335.33' TW=334.13' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 0.93 cfs @ 2.02 fps)

**Summary for Pond 21P: DMH-13**

Inflow Area =	1.039 ac, 73.61% Impervious, Inflow Depth = 1.84"	for 2-year event
Inflow =	2.09 cfs @ 12.09 hrs, Volume=	0.159 af
Outflow =	2.09 cfs @ 12.09 hrs, Volume=	0.159 af, Atten= 0%, Lag= 0.0 min
Primary =	2.09 cfs @ 12.09 hrs, Volume=	0.159 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 334.14' @ 12.09 hrs

Flood Elev= 342.06'

Device	Routing	Invert	Outlet Devices
#1	Primary	333.38'	<b>18.0" Round Culvert</b> L= 136.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 333.38' / 330.66' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=2.05 cfs @ 12.09 hrs HW=334.13' TW=331.31' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.05 cfs @ 2.33 fps)

**Summary for Pond 22P: CB-14**

Inflow Area =	0.373 ac, 76.57% Impervious, Inflow Depth = 1.60"	for 2-year event
Inflow =	0.69 cfs @ 12.09 hrs, Volume=	0.050 af
Outflow =	0.69 cfs @ 12.09 hrs, Volume=	0.050 af, Atten= 0%, Lag= 0.0 min
Primary =	0.69 cfs @ 12.09 hrs, Volume=	0.050 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 338.69' @ 12.09 hrs

Flood Elev= 342.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	338.21'	<b>12.0" Round Culvert</b> L= 17.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 338.21' / 338.00' S= 0.0124 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.67 cfs @ 12.09 hrs HW=338.68' TW=334.13' (Dynamic Tailwater)  
 ↑  
 1=Culvert (Inlet Controls 0.67 cfs @ 1.85 fps)

### Summary for Pond 23P: CB-15

Inflow Area = 0.161 ac, 98.59% Impervious, Inflow Depth = 2.76" for 2-year event  
 Inflow = 0.47 cfs @ 12.09 hrs, Volume= 0.037 af  
 Outflow = 0.47 cfs @ 12.09 hrs, Volume= 0.037 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.47 cfs @ 12.09 hrs, Volume= 0.037 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 338.70' @ 12.09 hrs  
 Flood Elev= 342.31'

Device	Routing	Invert	Outlet Devices
#1	Primary	338.31'	<b>12.0" Round Culvert</b> L= 28.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 338.31' / 338.00' S= 0.0111 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.45 cfs @ 12.09 hrs HW=338.69' TW=334.12' (Dynamic Tailwater)  
 ↑  
 1=Culvert (Inlet Controls 0.45 cfs @ 1.66 fps)

### Summary for Pond 24P: DMH-12

Inflow Area = 1.039 ac, 73.61% Impervious, Inflow Depth = 1.84" for 2-year event  
 Inflow = 2.09 cfs @ 12.09 hrs, Volume= 0.159 af  
 Outflow = 2.09 cfs @ 12.09 hrs, Volume= 0.159 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.09 cfs @ 12.09 hrs, Volume= 0.159 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 331.32' @ 12.09 hrs  
 Flood Elev= 338.71'

Device	Routing	Invert	Outlet Devices
#1	Primary	330.56'	<b>18.0" Round Culvert</b> L= 99.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 330.56' / 327.84' S= 0.0275 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=2.05 cfs @ 12.09 hrs HW=331.31' TW=328.49' (Dynamic Tailwater)  
 ↑  
 1=Culvert (Inlet Controls 2.05 cfs @ 2.33 fps)

### Summary for Pond 25P: DMH-11

Inflow Area = 1.039 ac, 73.61% Impervious, Inflow Depth = 1.84" for 2-year event  
 Inflow = 2.09 cfs @ 12.09 hrs, Volume= 0.159 af  
 Outflow = 2.09 cfs @ 12.09 hrs, Volume= 0.159 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.09 cfs @ 12.09 hrs, Volume= 0.159 af

**6092 - POST Toll Rev4 (current)**

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Type III 24-hr 2-year Rainfall=3.10"

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Page 29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 328.50' @ 12.09 hrs

Flood Elev= 332.34'

Device	Routing	Invert	Outlet Devices
#1	Primary	327.74'	<b>18.0" Round Culvert</b> L= 76.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 327.74' / 322.02' S= 0.0753 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=2.05 cfs @ 12.09 hrs HW=328.49' TW=322.97' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.05 cfs @ 2.33 fps)

**Summary for Pond 26P: DMH-10**

Inflow Area = 1.707 ac, 77.06% Impervious, Inflow Depth = 1.98" for 2-year event  
 Inflow = 3.72 cfs @ 12.09 hrs, Volume= 0.281 af  
 Outflow = 3.72 cfs @ 12.09 hrs, Volume= 0.281 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.72 cfs @ 12.09 hrs, Volume= 0.281 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 322.99' @ 12.09 hrs

Flood Elev= 327.14'

Device	Routing	Invert	Outlet Devices
#1	Primary	321.92'	<b>18.0" Round Culvert</b> L= 174.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 321.92' / 316.81' S= 0.0294 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.64 cfs @ 12.09 hrs HW=322.97' TW=317.51' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 3.64 cfs @ 2.76 fps)

**Summary for Pond 27P: CB-13**

Inflow Area = 0.182 ac, 98.50% Impervious, Inflow Depth = 2.76" for 2-year event  
 Inflow = 0.53 cfs @ 12.09 hrs, Volume= 0.042 af  
 Outflow = 0.53 cfs @ 12.09 hrs, Volume= 0.042 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.53 cfs @ 12.09 hrs, Volume= 0.042 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 323.92' @ 12.09 hrs

Flood Elev= 327.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.51'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.51' / 323.23' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.51 cfs @ 12.09 hrs HW=323.92' TW=322.97' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 0.51 cfs @ 1.71 fps)

### Summary for Pond 28P: CB-12

Inflow Area =	0.486 ac, 76.42% Impervious, Inflow Depth = 1.99"	for 2-year event
Inflow =	1.10 cfs @ 12.09 hrs, Volume=	0.081 af
Outflow =	1.10 cfs @ 12.09 hrs, Volume=	0.081 af, Atten= 0%, Lag= 0.0 min
Primary =	1.10 cfs @ 12.09 hrs, Volume=	0.081 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 324.14' @ 12.09 hrs  
 Flood Elev= 327.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.51'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.51' / 323.23' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.08 cfs @ 12.09 hrs HW=324.13' TW=322.97' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 1.08 cfs @ 2.11 fps)

### Summary for Pond 29P: DMH-9

Inflow Area =	2.440 ac, 76.17% Impervious, Inflow Depth = 1.96"	for 2-year event
Inflow =	5.33 cfs @ 12.09 hrs, Volume=	0.398 af
Outflow =	5.33 cfs @ 12.09 hrs, Volume=	0.398 af, Atten= 0%, Lag= 0.0 min
Primary =	5.33 cfs @ 12.09 hrs, Volume=	0.398 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 317.53' @ 12.09 hrs  
 Flood Elev= 322.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	316.31'	<b>24.0" Round Culvert</b> L= 62.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 316.31' / 316.00' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=5.21 cfs @ 12.09 hrs HW=317.51' TW=315.05' (Dynamic Tailwater)  
 ↗1=Culvert (Barrel Controls 5.21 cfs @ 3.79 fps)

### Summary for Pond 30P: CB-11

Inflow Area =	0.734 ac, 74.09% Impervious, Inflow Depth = 1.91"	for 2-year event
Inflow =	1.60 cfs @ 12.09 hrs, Volume=	0.117 af
Outflow =	1.60 cfs @ 12.09 hrs, Volume=	0.117 af, Atten= 0%, Lag= 0.0 min
Primary =	1.60 cfs @ 12.09 hrs, Volume=	0.117 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 317.96' @ 12.10 hrs

Flood Elev= 321.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	317.30'	<b>18.0" Round Culvert</b> L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 317.30' / 316.81' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=1.47 cfs @ 12.09 hrs HW=317.95' TW=317.51' (Dynamic Tailwater)  
 ↑—1=Culvert (Outlet Controls 1.47 cfs @ 2.94 fps)

### Summary for Pond 31P: PROP. CULVERT

Inflow Area = 2.338 ac, 19.03% Impervious, Inflow Depth = 0.48" for 2-year event  
 Inflow = 0.59 cfs @ 12.42 hrs, Volume= 0.093 af  
 Outflow = 0.59 cfs @ 12.42 hrs, Volume= 0.093 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.59 cfs @ 12.42 hrs, Volume= 0.093 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 314.41' @ 12.42 hrs

Flood Elev= 318.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	314.00'	<b>15.0" Round Culvert</b> L= 75.8' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.00' / 313.00' S= 0.0132 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf

**Primary OutFlow** Max=0.59 cfs @ 12.42 hrs HW=314.41' TW=312.17' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 0.59 cfs @ 1.71 fps)

### Summary for Pond 32P: CB-10

Inflow Area = 0.106 ac, 100.00% Impervious, Inflow Depth = 2.87" for 2-year event  
 Inflow = 0.31 cfs @ 12.09 hrs, Volume= 0.025 af  
 Outflow = 0.31 cfs @ 12.09 hrs, Volume= 0.025 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.31 cfs @ 12.09 hrs, Volume= 0.025 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 323.30' @ 12.13 hrs

Flood Elev= 327.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.00'	<b>18.0" Round Culvert</b> L= 55.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.00' / 322.73' S= 0.0049 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.25 cfs @ 12.09 hrs HW=323.30' TW=323.09' (Dynamic Tailwater)  
 ↗1=Culvert (Outlet Controls 0.25 cfs @ 1.53 fps)

### Summary for Pond 33P: CB-9

Inflow Area = 0.119 ac, 100.00% Impervious, Inflow Depth = 2.87" for 2-year event  
 Inflow = 0.35 cfs @ 12.09 hrs, Volume= 0.028 af  
 Outflow = 0.35 cfs @ 12.09 hrs, Volume= 0.028 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.35 cfs @ 12.09 hrs, Volume= 0.028 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 323.29' @ 12.09 hrs  
 Flood Elev= 327.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.00'	<b>18.0" Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.00' / 322.52' S= 0.0686 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.34 cfs @ 12.09 hrs HW=323.29' TW=323.00' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 0.34 cfs @ 1.44 fps)

### Summary for Pond 34P: CB-18

Inflow Area = 0.465 ac, 62.98% Impervious, Inflow Depth = 1.60" for 2-year event  
 Inflow = 0.85 cfs @ 12.09 hrs, Volume= 0.062 af  
 Outflow = 0.85 cfs @ 12.09 hrs, Volume= 0.062 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.85 cfs @ 12.09 hrs, Volume= 0.062 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 320.54' @ 12.09 hrs  
 Flood Elev= 325.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	320.00'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 320.00' / 319.04' S= 0.0600 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.84 cfs @ 12.09 hrs HW=320.53' TW=319.47' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 0.84 cfs @ 1.97 fps)

### Summary for Pond 35P: DMH-8

Inflow Area = 2.324 ac, 22.24% Impervious, Inflow Depth = 0.51" for 2-year event  
 Inflow = 0.99 cfs @ 12.15 hrs, Volume= 0.099 af  
 Outflow = 0.99 cfs @ 12.15 hrs, Volume= 0.099 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.99 cfs @ 12.15 hrs, Volume= 0.099 af

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Page 33

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 323.21' @ 12.18 hrs

Flood Elev= 327.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	322.63'	<b>24.0" Round Culvert</b> L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 322.63' / 322.52' S= 0.0052 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.88 cfs @ 12.15 hrs HW=323.20' TW=323.07' (Dynamic Tailwater)  
 ↗1=Culvert (Outlet Controls 0.88 cfs @ 1.80 fps)

**Summary for Pond 36P: DMH-16**

Inflow Area =	0.465 ac, 62.98% Impervious, Inflow Depth = 1.60"	for 2-year event
Inflow =	0.85 cfs @ 12.09 hrs, Volume=	0.062 af
Outflow =	0.85 cfs @ 12.09 hrs, Volume=	0.062 af, Atten= 0%, Lag= 0.0 min
Primary =	0.85 cfs @ 12.09 hrs, Volume=	0.062 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 319.48' @ 12.09 hrs  
 Flood Elev= 326.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	318.94'	<b>12.0" Round Culvert</b> L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 318.94' / 317.20' S= 0.0829 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.84 cfs @ 12.09 hrs HW=319.47' TW=318.04' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 0.84 cfs @ 1.97 fps)

**Summary for Pond 37P: DMH-7**

Inflow Area =	2.443 ac, 26.02% Impervious, Inflow Depth = 0.62"	for 2-year event
Inflow =	1.27 cfs @ 12.13 hrs, Volume=	0.127 af
Outflow =	1.27 cfs @ 12.13 hrs, Volume=	0.127 af, Atten= 0%, Lag= 0.0 min
Primary =	1.27 cfs @ 12.13 hrs, Volume=	0.127 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 323.07' @ 12.15 hrs  
 Flood Elev= 327.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	322.42'	<b>24.0" Round Culvert</b> L= 78.1' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 322.42' / 321.96' S= 0.0059 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.26 cfs @ 12.13 hrs HW=323.06' TW=322.76' (Dynamic Tailwater)  
 ↑  
 1=Culvert (Outlet Controls 1.26 cfs @ 2.19 fps)

### **Summary for Pond 39P: CB-2 [EXISTING]**

Inflow Area = 0.432 ac, 80.78% Impervious, Inflow Depth = 2.35" for 2-year event  
 Inflow = 1.13 cfs @ 12.09 hrs, Volume= 0.085 af  
 Outflow = 1.13 cfs @ 12.09 hrs, Volume= 0.085 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.13 cfs @ 12.09 hrs, Volume= 0.085 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 310.86' @ 12.10 hrs  
 Flood Elev= 314.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.05'	<b>12.0" Round Culvert</b> L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.05' / 310.00' S= 0.0010 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.04 cfs @ 12.09 hrs HW=310.85' TW=310.63' (Dynamic Tailwater)  
 ↑  
 1=Culvert (Outlet Controls 1.04 cfs @ 2.12 fps)

### **Summary for Pond 40P: CB-1 [EXISTING]**

Inflow Area = 0.174 ac, 81.24% Impervious, Inflow Depth = 2.35" for 2-year event  
 Inflow = 0.45 cfs @ 12.09 hrs, Volume= 0.034 af  
 Outflow = 0.45 cfs @ 12.09 hrs, Volume= 0.034 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.45 cfs @ 12.09 hrs, Volume= 0.034 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 311.13' @ 12.09 hrs  
 Flood Elev= 314.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.75'	<b>12.0" Round Culvert</b> L= 17.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.75' / 310.15' S= 0.0353 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.44 cfs @ 12.09 hrs HW=311.13' TW=310.85' (Dynamic Tailwater)  
 ↑  
 1=Culvert (Inlet Controls 0.44 cfs @ 1.65 fps)

### **Summary for Pond 42P: DMH-1 [EXISTING]**

Inflow Area = 0.432 ac, 80.78% Impervious, Inflow Depth = 2.35" for 2-year event  
 Inflow = 1.13 cfs @ 12.09 hrs, Volume= 0.085 af  
 Outflow = 1.13 cfs @ 12.09 hrs, Volume= 0.085 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.13 cfs @ 12.09 hrs, Volume= 0.085 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 310.64' @ 12.09 hrs

Flood Elev= 315.33'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.00'	<b>12.0" Round Culvert</b> L= 92.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.00' / 308.02' S= 0.0215 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.10 cfs @ 12.09 hrs HW=310.63' TW=0.00' (Dynamic Tailwater)  
 ↳ 1=Culvert (Inlet Controls 1.10 cfs @ 2.13 fps)

### Summary for Pond 43P: SWALE & FIELD BASIN

Inflow Area =	2.218 ac, 18.52% Impervious, Inflow Depth = 0.48"	for 2-year event
Inflow =	0.77 cfs @ 12.16 hrs, Volume=	0.088 af
Outflow =	0.78 cfs @ 12.17 hrs, Volume=	0.088 af, Atten= 0%, Lag= 0.6 min
Discarded =	0.02 cfs @ 12.17 hrs, Volume=	0.015 af
Primary =	0.76 cfs @ 12.17 hrs, Volume=	0.073 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 328.07' @ 12.17 hrs Surf.Area= 663 sf Storage= 44 cf

Plug-Flow detention time= 1.5 min calculated for 0.088 af (100% of inflow)  
 Center-of-Mass det. time= 1.5 min ( 911.6 - 910.1 )

Volume	Invert	Avail.Storage	Storage Description		
#1	328.00'	1,150 cf	<b>Custom Stage Data (Irregular) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
328.00	600	119.4	0	0	600
329.00	1,808	444.3	1,150	1,150	15,177

Device	Routing	Invert	Outlet Devices
#1	Discarded	328.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 320.50'
#2	Primary	324.00'	<b>18.0" Round Culvert</b> L= 51.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 324.00' / 322.73' S= 0.0249 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#3	Device 2	328.00'	<b>48.0" Horiz. Orifice/Grate C= 0.600</b> Limited to weir flow at low heads

**Discarded OutFlow** Max=0.02 cfs @ 12.17 hrs HW=328.07' (Free Discharge)  
 ↳ 1=Exfiltration ( Controls 0.02 cfs)

**Primary OutFlow** Max=0.74 cfs @ 12.17 hrs HW=328.07' TW=323.20' (Dynamic Tailwater)  
 ↳ 2=Culvert (Passes 0.74 cfs of 12.24 cfs potential flow)  
 ↳ 3=Orifice/Grate (Weir Controls 0.74 cfs @ 0.86 fps)

**Summary for Link 4L: DP-A**

Inflow Area = 23.100 ac, 27.90% Impervious, Inflow Depth = 0.36" for 2-year event  
Inflow = 3.66 cfs @ 12.11 hrs, Volume= 0.686 af  
Primary = 3.66 cfs @ 12.11 hrs, Volume= 0.686 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Summary for Link 5L: DP-B**

Inflow Area = 0.891 ac, 0.00% Impervious, Inflow Depth = 0.00" for 2-year event  
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af  
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment 1S: POST 1.A</b>	Runoff Area=11,232 sf 80.47% Impervious Runoff Depth=3.81" Tc=6.0 min CN=93 Runoff=1.06 cfs 0.082 af
<b>Subcatchment 2S: POST 2.A</b>	Runoff Area=23,362 sf 69.71% Impervious Runoff Depth=3.19" Tc=6.0 min CN=87 Runoff=1.94 cfs 0.143 af
<b>Subcatchment 5S: POST 6.A</b>	Runoff Area=101,865 sf 19.03% Impervious Runoff Depth=1.26" Flow Length=1,092' Tc=22.1 min CN=63 Runoff=2.02 cfs 0.246 af
<b>Subcatchment 6S: POST 7.A</b>	Runoff Area=21,169 sf 82.05% Impervious Runoff Depth=3.59" Tc=6.0 min CN=91 Runoff=1.93 cfs 0.146 af
<b>Subcatchment 7S: POST 8.A</b>	Runoff Area=21,105 sf 5.32% Impervious Runoff Depth=1.39" Tc=6.0 min UI Adjusted CN=65 Runoff=0.73 cfs 0.056 af
<b>Subcatchment 8S: POST 3.A</b>	Runoff Area=5,197 sf 100.00% Impervious Runoff Depth=4.36" Tc=6.0 min CN=98 Runoff=0.52 cfs 0.043 af
<b>Subcatchment 9S: POST 4.A</b>	Runoff Area=25,089 sf 74.55% Impervious Runoff Depth=2.91" Tc=6.0 min CN=84 Runoff=1.91 cfs 0.140 af
<b>Subcatchment 10S: POST 5.A</b>	Runoff Area=14,640 sf 81.36% Impervious Runoff Depth=3.39" Tc=6.0 min CN=89 Runoff=1.27 cfs 0.095 af
<b>Subcatchment 11S: POST 9.A</b>	Runoff Area=12,906 sf 81.22% Impervious Runoff Depth=3.59" Tc=6.0 min CN=91 Runoff=1.17 cfs 0.089 af
<b>Subcatchment 12S: POST 12.A</b>	Runoff Area=96,627 sf 18.52% Impervious Runoff Depth=1.26" Tc=8.0 min UI Adjusted CN=63 Runoff=2.71 cfs 0.233 af
<b>Subcatchment 13S: POST 14.A</b>	Runoff Area=7,009 sf 98.59% Impervious Runoff Depth=4.25" Tc=6.0 min CN=97 Runoff=0.70 cfs 0.057 af
<b>Subcatchment 14S: POST 15.A</b>	Runoff Area=16,242 sf 76.57% Impervious Runoff Depth=2.91" Tc=6.0 min CN=84 Runoff=1.24 cfs 0.090 af
<b>Subcatchment 15S: POST 13.A</b>	Runoff Area=15,308 sf 48.14% Impervious Runoff Depth=2.46" Tc=6.0 min CN=79 Runoff=0.99 cfs 0.072 af
<b>Subcatchment 16S: POST 16.A</b>	Runoff Area=6,688 sf 98.52% Impervious Runoff Depth=4.25" Tc=6.0 min CN=97 Runoff=0.67 cfs 0.054 af
<b>Subcatchment 17S: POST 18.A</b>	Runoff Area=7,933 sf 98.50% Impervious Runoff Depth=4.25" Tc=6.0 min CN=97 Runoff=0.79 cfs 0.064 af
<b>Subcatchment 18S: POST 17.A</b>	Runoff Area=21,158 sf 76.42% Impervious Runoff Depth=3.39" Tc=6.0 min CN=89 Runoff=1.84 cfs 0.137 af

**Subcatchment 19S: POST 24.A**

Runoff Area=45,444 sf 0.00% Impervious Runoff Depth=1.97"  
Tc=6.0 min CN=73 Runoff=2.34 cfs 0.171 af

**Subcatchment 20S: POST 19.A**

Runoff Area=31,969 sf 74.09% Impervious Runoff Depth=3.29"  
Tc=6.0 min CN=88 Runoff=2.72 cfs 0.201 af

**Subcatchment 21S: POST 20.A**

Runoff Area=54,029 sf 13.37% Impervious Runoff Depth=1.67"  
Tc=6.0 min CN=69 Runoff=2.31 cfs 0.173 af

**Subcatchment 22S: POST 22.A**

Runoff Area=165,736 sf 13.02% Impervious Runoff Depth=1.14"  
Flow Length=884' Tc=15.2 min CN=61 Runoff=3.30 cfs 0.360 af

**Subcatchment 23S: POST 23.A**

Runoff Area=176,164 sf 0.00% Impervious Runoff Depth=2.05"  
Tc=16.7 min CN=74 Runoff=6.93 cfs 0.691 af

**Subcatchment 24S: POST 21.A**

Runoff Area=74,028 sf 1.61% Impervious Runoff Depth=1.14"  
Tc=6.0 min UI Adjusted CN=61 Runoff=1.97 cfs 0.161 af

**Subcatchment 25S: POST 1.B**

Runoff Area=38,793 sf 0.00% Impervious Runoff Depth=0.08"  
Flow Length=106' Tc=11.1 min CN=37 Runoff=0.01 cfs 0.006 af

**Subcatchment 26S: POST 10.A**

Runoff Area=4,620 sf 100.00% Impervious Runoff Depth=4.36"  
Tc=6.0 min CN=98 Runoff=0.47 cfs 0.039 af

**Subcatchment 27S: POST 11.A**

Runoff Area=5,180 sf 100.00% Impervious Runoff Depth=4.36"  
Tc=6.0 min CN=98 Runoff=0.52 cfs 0.043 af

**Subcatchment 28S: BACK ROOFS OF**

Runoff Area=13,690 sf 100.00% Impervious Runoff Depth=4.36"  
Tc=6.0 min CN=98 Runoff=1.38 cfs 0.114 af

**Subcatchment 38S: POST 25.A**

Runoff Area=20,255 sf 62.98% Impervious Runoff Depth=2.91"  
Tc=6.0 min CN=84 Runoff=1.54 cfs 0.113 af

**Subcatchment 41S: POST 26.A**

Runoff Area=7,581 sf 81.24% Impervious Runoff Depth=3.81"  
Tc=6.0 min CN=93 Runoff=0.72 cfs 0.055 af

**Pond 5P: INFIL BASIN #1**

Peak Elev=315.07' Storage=33,723 cf Inflow=16.62 cfs 1.483 af

Discarded=0.47 cfs 0.760 af Primary=1.47 cfs 0.723 af Outflow=1.94 cfs 1.483 af

**Pond 6P: INFIL BASIN #2**

Peak Elev=317.03' Storage=41,465 cf Inflow=15.66 cfs 1.727 af

Discarded=1.01 cfs 1.442 af Primary=0.27 cfs 0.286 af Outflow=1.28 cfs 1.727 af

**Pond 7P: DMH-2**

Peak Elev=315.87' Inflow=11.80 cfs 0.921 af

24.0" Round Culvert n=0.013 L=60.0' S=0.0200 '/' Outflow=11.80 cfs 0.921 af

**Pond 8P: CB-5**

Peak Elev=316.18' Inflow=1.94 cfs 0.143 af

12.0" Round Culvert n=0.013 L=20.0' S=0.0180 '/' Outflow=1.94 cfs 0.143 af

**Pond 9P: CB-6**

Peak Elev=316.48' Inflow=1.70 cfs 0.132 af

12.0" Round Culvert n=0.013 L=20.0' S=0.0375 '/' Outflow=1.70 cfs 0.132 af

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Page 39

Pond 10P: CB-4	Peak Elev=315.73' Inflow=1.93 cfs 0.146 af 12.0" Round Culvert n=0.013 L=25.0' S=0.0168 '/' Outflow=1.93 cfs 0.146 af
Pond 11P: DMH-3	Peak Elev=318.58' Inflow=8.19 cfs 0.646 af 24.0" Round Culvert n=0.013 L=63.0' S=0.0349 '/' Outflow=8.19 cfs 0.646 af
Pond 12P: DMH-4	Peak Elev=323.26' Inflow=6.67 cfs 0.534 af 24.0" Round Culvert n=0.013 L=99.0' S=0.0481 '/' Outflow=6.67 cfs 0.534 af
Pond 13P: DMH-6	Peak Elev=336.22' Inflow=3.19 cfs 0.234 af 18.0" Round Culvert n=0.013 L=124.0' S=0.0576 '/' Outflow=3.19 cfs 0.234 af
Pond 14P: CB-8	Peak Elev=337.23' Inflow=1.27 cfs 0.095 af 12.0" Round Culvert n=0.013 L=14.0' S=0.0571 '/' Outflow=1.27 cfs 0.095 af
Pond 15P: CB-7	Peak Elev=336.86' Inflow=1.91 cfs 0.140 af 12.0" Round Culvert n=0.013 L=10.0' S=0.0200 '/' Outflow=1.91 cfs 0.140 af
Pond 16P: DMH-5	Peak Elev=328.98' Inflow=3.19 cfs 0.234 af 18.0" Round Culvert n=0.013 L=91.0' S=0.0599 '/' Outflow=3.19 cfs 0.234 af
Pond 17P: DMH-15	Peak Elev=336.36' Inflow=1.66 cfs 0.126 af 12.0" Round Culvert n=0.013 L=69.0' S=0.0100 '/' Outflow=1.66 cfs 0.126 af
Pond 18P: CB-17	Peak Elev=336.45' Inflow=0.67 cfs 0.054 af 12.0" Round Culvert n=0.013 L=16.0' S=0.0100 '/' Outflow=0.67 cfs 0.054 af
Pond 19P: CB-16	Peak Elev=336.53' Inflow=0.99 cfs 0.072 af 12.0" Round Culvert n=0.013 L=16.0' S=0.0100 '/' Outflow=0.99 cfs 0.072 af
Pond 20P: DMH-14	Peak Elev=335.57' Inflow=1.66 cfs 0.126 af 12.0" Round Culvert n=0.013 L=88.0' S=0.0100 '/' Outflow=1.66 cfs 0.126 af
Pond 21P: DMH-13	Peak Elev=334.42' Inflow=3.60 cfs 0.274 af 18.0" Round Culvert n=0.013 L=136.0' S=0.0200 '/' Outflow=3.60 cfs 0.274 af
Pond 22P: CB-14	Peak Elev=338.88' Inflow=1.24 cfs 0.090 af 12.0" Round Culvert n=0.013 L=17.0' S=0.0124 '/' Outflow=1.24 cfs 0.090 af
Pond 23P: CB-15	Peak Elev=338.79' Inflow=0.70 cfs 0.057 af 12.0" Round Culvert n=0.013 L=28.0' S=0.0111 '/' Outflow=0.70 cfs 0.057 af
Pond 24P: DMH-12	Peak Elev=331.60' Inflow=3.60 cfs 0.274 af 18.0" Round Culvert n=0.013 L=99.0' S=0.0275 '/' Outflow=3.60 cfs 0.274 af
Pond 25P: DMH-11	Peak Elev=328.78' Inflow=3.60 cfs 0.274 af 18.0" Round Culvert n=0.013 L=76.0' S=0.0753 '/' Outflow=3.60 cfs 0.274 af
Pond 26P: DMH-10	Peak Elev=323.53' Inflow=6.24 cfs 0.475 af 18.0" Round Culvert n=0.013 L=174.0' S=0.0294 '/' Outflow=6.24 cfs 0.475 af

Pond 27P: CB-13	Peak Elev=324.03' Inflow=0.79 cfs 0.064 af 12.0" Round Culvert n=0.013 L=14.0' S=0.0200 '/' Outflow=0.79 cfs 0.064 af
Pond 28P: CB-12	Peak Elev=324.39' Inflow=1.84 cfs 0.137 af 12.0" Round Culvert n=0.013 L=14.0' S=0.0200 '/' Outflow=1.84 cfs 0.137 af
Pond 29P: DMH-9	Peak Elev=317.99' Inflow=8.95 cfs 0.677 af 24.0" Round Culvert n=0.013 L=62.0' S=0.0050 '/' Outflow=8.95 cfs 0.677 af
Pond 30P: CB-11	Peak Elev=318.31' Inflow=2.72 cfs 0.201 af 18.0" Round Culvert n=0.013 L=49.0' S=0.0100 '/' Outflow=2.72 cfs 0.201 af
Pond 31P: PROP. CULVERT	Peak Elev=315.09' Inflow=2.02 cfs 0.246 af 15.0" Round Culvert n=0.013 L=75.8' S=0.0132 '/' Outflow=2.02 cfs 0.246 af
Pond 32P: CB-10	Peak Elev=323.76' Inflow=0.47 cfs 0.039 af 18.0" Round Culvert n=0.013 L=55.0' S=0.0049 '/' Outflow=0.47 cfs 0.039 af
Pond 33P: CB-9	Peak Elev=323.60' Inflow=0.52 cfs 0.043 af 18.0" Round Culvert n=0.013 L=7.0' S=0.0686 '/' Outflow=0.52 cfs 0.043 af
Pond 34P: CB-18	Peak Elev=320.77' Inflow=1.54 cfs 0.113 af 12.0" Round Culvert n=0.013 L=16.0' S=0.0600 '/' Outflow=1.54 cfs 0.113 af
Pond 35P: DMH-8	Peak Elev=323.75' Inflow=3.09 cfs 0.256 af 24.0" Round Culvert n=0.013 L=21.0' S=0.0052 '/' Outflow=3.09 cfs 0.256 af
Pond 36P: DMH-16	Peak Elev=319.71' Inflow=1.54 cfs 0.113 af 12.0" Round Culvert n=0.013 L=21.0' S=0.0829 '/' Outflow=1.54 cfs 0.113 af
Pond 37P: DMH-7	Peak Elev=323.59' Inflow=3.58 cfs 0.299 af 24.0" Round Culvert n=0.013 L=78.1' S=0.0059 '/' Outflow=3.58 cfs 0.299 af
Pond 39P: CB-2 [EXISTING]	Peak Elev=311.16' Inflow=1.78 cfs 0.137 af 12.0" Round Culvert n=0.013 L=49.0' S=0.0010 '/' Outflow=1.78 cfs 0.137 af
Pond 40P: CB-1 [EXISTING]	Peak Elev=311.30' Inflow=0.72 cfs 0.055 af 12.0" Round Culvert n=0.013 L=17.0' S=0.0353 '/' Outflow=0.72 cfs 0.055 af
Pond 42P: DMH-1 [EXISTING]	Peak Elev=310.86' Inflow=1.78 cfs 0.137 af 12.0" Round Culvert n=0.013 L=92.0' S=0.0215 '/' Outflow=1.78 cfs 0.137 af
Pond 43P: SWALE & FIELD BASIN	Peak Elev=328.16' Storage=110 cf Inflow=2.71 cfs 0.233 af Discarded=0.02 cfs 0.016 af Primary=2.70 cfs 0.217 af Outflow=2.72 cfs 0.233 af
Link 4L: DP-A	Inflow=8.78 cfs 1.651 af Primary=8.78 cfs 1.651 af
Link 5L: DP-B	Inflow=0.01 cfs 0.006 af Primary=0.01 cfs 0.006 af

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Type III 24-hr 10-year Rainfall=4.60"

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Page 41

Total Runoff Area = 23.990 ac Runoff Volume = 3.874 af Average Runoff Depth = 1.94"  
73.13% Pervious = 17.545 ac 26.87% Impervious = 6.446 ac

### Summary for Subcatchment 1S: POST 1.A

Runoff = 1.06 cfs @ 12.09 hrs, Volume= 0.082 af, Depth= 3.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description			
4,679	98	Paved parking, HSG C			
1,673	98	Paved parking, HSG B			
2,343	98	Unconnected pavement, HSG C			
343	98	Unconnected pavement, HSG B			
395	61	>75% Grass cover, Good, HSG B			
1,799	74	>75% Grass cover, Good, HSG C			
11,232	93	Weighted Average			
2,194		19.53% Pervious Area			
9,038		80.47% Impervious Area			
2,686		29.72% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 2S: POST 2.A

Runoff = 1.94 cfs @ 12.09 hrs, Volume= 0.143 af, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description			
7,254	98	Roofs, HSG B			
757	98	Unconnected pavement, HSG B			
8,275	98	Paved parking, HSG B			
7,076	61	>75% Grass cover, Good, HSG B			
23,362	87	Weighted Average			
7,076		30.29% Pervious Area			
16,286		69.71% Impervious Area			
757		4.65% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 5S: POST 6.A

Runoff = 2.02 cfs @ 12.35 hrs, Volume= 0.246 af, Depth= 1.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
3,460	70	Woods, Good, HSG C
1,929	74	>75% Grass cover, Good, HSG C
11,762	55	Woods, Good, HSG B
43,358	61	>75% Grass cover, Good, HSG B
10,565	98	Roofs, HSG B
1,187	98	Unconnected pavement, HSG B
6,891	98	Roofs, HSG A
741	98	Unconnected pavement, HSG A
21,972	39	>75% Grass cover, Good, HSG A
101,865	63	Weighted Average
82,481		80.97% Pervious Area
19,384		19.03% Impervious Area
1,928		9.95% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3	50	0.0060	0.09		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 3.10"
11.8	859	0.0300	1.21		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
0.9	100	0.0700	1.85		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
0.1	83	0.0200	10.18	31.99	<b>Pipe Channel,</b> 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
22.1	1,092	Total			

### Summary for Subcatchment 6S: POST 7.A

Runoff = 1.93 cfs @ 12.09 hrs, Volume= 0.146 af, Depth= 3.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
9,604	98	Paved parking, HSG B
511	98	Unconnected pavement, HSG B
3,800	61	>75% Grass cover, Good, HSG B
21,169	91	Weighted Average
3,800		17.95% Pervious Area
17,369		82.05% Impervious Area
511		2.94% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 7S: POST 8.A**

Runoff = 0.73 cfs @ 12.10 hrs, Volume= 0.056 af, Depth= 1.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Adj	Description
120	98		Roofs, HSG B
1,579	96		Gravel surface, HSG B
1,003	98		Unconnected pavement, HSG B
18,403	61		>75% Grass cover, Good, HSG B
21,105	66	65	Weighted Average, UI Adjusted
19,982			94.68% Pervious Area
1,123			5.32% Impervious Area
1,003			89.31% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 8S: POST 3.A**

Runoff = 0.52 cfs @ 12.09 hrs, Volume= 0.043 af, Depth= 4.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
5,197	98	Paved parking, HSG B
5,197		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 9S: POST 4.A**

Runoff = 1.91 cfs @ 12.09 hrs, Volume= 0.140 af, Depth= 2.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

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Type III 24-hr 10-year Rainfall=4.60"

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Page 45

Area (sf)	CN	Description
2,372	98	Roofs, HSG B
150	98	Unconnected pavement, HSG B
2,521	98	Paved parking, HSG B
1,636	61	>75% Grass cover, Good, HSG B
6,091	98	Roofs, HSG A
649	98	Unconnected pavement, HSG A
6,920	98	Paved parking, HSG A
4,750	39	>75% Grass cover, Good, HSG A

25,089	84	Weighted Average
6,386		25.45% Pervious Area
18,703		74.55% Impervious Area
799		4.27% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 10S: POST 5.A**

Runoff = 1.27 cfs @ 12.09 hrs, Volume= 0.095 af, Depth= 3.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
2,418	98	Roofs, HSG B
338	98	Unconnected pavement, HSG B
2,578	98	Paved parking, HSG B
1,011	61	>75% Grass cover, Good, HSG B
2,418	98	Roofs, HSG A
494	98	Unconnected pavement, HSG A
3,665	98	Paved parking, HSG A
1,718	39	>75% Grass cover, Good, HSG A

14,640	89	Weighted Average
2,729		18.64% Pervious Area
11,911		81.36% Impervious Area
832		6.99% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 11S: POST 9.A**

Runoff = 1.17 cfs @ 12.09 hrs, Volume= 0.089 af, Depth= 3.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

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Type III 24-hr 10-year Rainfall=4.60"

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Page 46

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
7,363	98	Paved parking, HSG B			
701	98	Unconnected pavement, HSG B			
2,424	61	>75% Grass cover, Good, HSG B			
12,906	91	Weighted Average			
2,424		18.78% Pervious Area			
10,482		81.22% Impervious Area			
701		6.69% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 12S: POST 12.A**

Runoff = 2.71 cfs @ 12.13 hrs, Volume= 0.233 af, Depth= 1.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Adj	Description		
9,152	98		Roofs, HSG B		
2,471	98		Unconnected pavement, HSG B		
59,845	61		>75% Grass cover, Good, HSG B		
4,539	98		Roofs, HSG A		
1,734	98		Unconnected pavement, HSG A		
18,886	39		>75% Grass cover, Good, HSG A		
96,627	64	63	Weighted Average, UI Adjusted		
78,731			81.48% Pervious Area		
17,896			18.52% Impervious Area		
4,205			23.50% Unconnected		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry,

**Summary for Subcatchment 13S: POST 14.A**

Runoff = 0.70 cfs @ 12.09 hrs, Volume= 0.057 af, Depth= 4.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

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Page 47

Area (sf)	CN	Description
1,784	98	Roofs, HSG A
3,387	98	Paved parking, HSG A
99	39	>75% Grass cover, Good, HSG A
1,105	98	Paved parking, HSG B
634	98	Roofs, HSG B
7,009	97	Weighted Average
99		1.41% Pervious Area
6,910		98.59% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 14S: POST 15.A**

Runoff = 1.24 cfs @ 12.09 hrs, Volume= 0.090 af, Depth= 2.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
1,135	98	Paved parking, HSG B
6,045	98	Roofs, HSG A
496	98	Unconnected pavement, HSG A
4,760	98	Paved parking, HSG A
3,806	39	>75% Grass cover, Good, HSG A
16,242	84	Weighted Average
3,806		23.43% Pervious Area
12,436		76.57% Impervious Area
496		3.99% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 15S: POST 13.A**

Runoff = 0.99 cfs @ 12.09 hrs, Volume= 0.072 af, Depth= 2.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
1,547	98	Unconnected pavement, HSG B			
3,405	98	Paved parking, HSG B			
7,938	61	>75% Grass cover, Good, HSG B			
15,308	79	Weighted Average			
7,938		51.86% Pervious Area			
7,370		48.14% Impervious Area			
1,547		20.99% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 16S: POST 16.A

Runoff = 0.67 cfs @ 12.09 hrs, Volume= 0.054 af, Depth= 4.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
4,171	98	Paved parking, HSG B			
99	61	>75% Grass cover, Good, HSG B			
6,688	97	Weighted Average			
99		1.48% Pervious Area			
6,589		98.52% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 17S: POST 18.A

Runoff = 0.79 cfs @ 12.09 hrs, Volume= 0.064 af, Depth= 4.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
5,396	98	Paved parking, HSG B			
119	61	>75% Grass cover, Good, HSG B			
7,933	97	Weighted Average			
119		1.50% Pervious Area			
7,814		98.50% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 18S: POST 17.A**

Runoff = 1.84 cfs @ 12.09 hrs, Volume= 0.137 af, Depth= 3.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
7,406	98	Paved parking, HSG B
1,509	98	Unconnected pavement, HSG B
4,989	61	>75% Grass cover, Good, HSG B
21,158	89	Weighted Average
4,989		23.58% Pervious Area
16,169		76.42% Impervious Area
1,509		9.33% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 19S: POST 24.A**

Runoff = 2.34 cfs @ 12.10 hrs, Volume= 0.171 af, Depth= 1.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
16,251	77	Woods, Good, HSG D
5,201	96	Gravel surface, HSG D
5,436	80	>75% Grass cover, Good, HSG D
6,221	30	Woods, Good, HSG A
3,810	39	>75% Grass cover, Good, HSG A
4,986	96	Gravel surface, HSG A
1,269	61	>75% Grass cover, Good, HSG B
2,270	96	Gravel surface, HSG B
45,444	73	Weighted Average
45,444		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 20S: POST 19.A**

Runoff = 2.72 cfs @ 12.09 hrs, Volume= 0.201 af, Depth= 3.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description			
7,254	98	Roofs, HSG B			
15,358	98	Paved parking, HSG B			
1,073	98	Unconnected pavement, HSG B			
8,284	61	>75% Grass cover, Good, HSG B			
31,969	88	Weighted Average			
8,284		25.91% Pervious Area			
23,685		74.09% Impervious Area			
1,073		4.53% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 21S: POST 20.A**

Runoff = 2.31 cfs @ 12.10 hrs, Volume= 0.173 af, Depth= 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description			
5,596	98	Roofs, HSG B			
1,630	98	Unconnected pavement, HSG B			
1,749	96	Gravel surface, HSG B			
40,318	61	>75% Grass cover, Good, HSG B			
1,861	96	Gravel surface, HSG D			
2,875	80	>75% Grass cover, Good, HSG D			
54,029	69	Weighted Average			
46,803		86.63% Pervious Area			
7,226		13.37% Impervious Area			
1,630		22.56% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 22S: POST 22.A

Runoff = 3.30 cfs @ 12.24 hrs, Volume= 0.360 af, Depth= 1.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
6,845	98	Roofs, HSG A
819	98	Unconnected pavement, HSG A
5,708	30	Woods, Good, HSG A
30,256	39	>75% Grass cover, Good, HSG A
11,252	98	Roofs, HSG B
2,667	98	Unconnected pavement, HSG B
88,631	61	>75% Grass cover, Good, HSG B
14,680	55	Woods, Good, HSG B
2,251	96	Gravel surface, HSG B
831	96	Gravel surface, HSG D
1,334	80	>75% Grass cover, Good, HSG D
462	77	Woods, Good, HSG D
165,736	61	Weighted Average
144,153		86.98% Pervious Area
21,583		13.02% Impervious Area
3,486		16.15% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7	50	0.0200	0.15		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 3.10"
3.7	348	0.0500	1.57		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
5.8	486	0.0400	1.40		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
15.2	884	Total			

### Summary for Subcatchment 23S: POST 23.A

Runoff = 6.93 cfs @ 12.24 hrs, Volume= 0.691 af, Depth= 2.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
24,614	55	Woods, Good, HSG B
1,206	96	Gravel surface, HSG D
1,599	80	>75% Grass cover, Good, HSG D
148,745	77	Woods, Good, HSG D
176,164	74	Weighted Average
176,164		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.7	Direct Entry,				

**Summary for Subcatchment 24S: POST 21.A**

Runoff = 1.97 cfs @ 12.11 hrs, Volume= 0.161 af, Depth= 1.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Adj	Description
1,194	98		Unconnected pavement, HSG B
248	96		Gravel surface, HSG B
72,586	61		>75% Grass cover, Good, HSG B
74,028	62	61	Weighted Average, UI Adjusted
72,834			98.39% Pervious Area
1,194			1.61% Impervious Area
1,194			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 25S: POST 1.B**

Runoff = 0.01 cfs @ 15.20 hrs, Volume= 0.006 af, Depth= 0.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
7,624	55	Woods, Good, HSG B
176	61	>75% Grass cover, Good, HSG B
24,732	30	Woods, Good, HSG A
6,261	39	>75% Grass cover, Good, HSG A
38,793	37	Weighted Average
38,793		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	50	0.0300	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.10"
0.4	56	0.2700	2.60		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
11.1	106	Total			

### Summary for Subcatchment 26S: POST 10.A

Runoff = 0.47 cfs @ 12.09 hrs, Volume= 0.039 af, Depth= 4.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
4,620	98	Paved parking, HSG B
4,620		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 27S: POST 11.A

Runoff = 0.52 cfs @ 12.09 hrs, Volume= 0.043 af, Depth= 4.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
5,180	98	Paved parking, HSG B
5,180		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 28S: BACK ROOFS OF UNITS 1-10

Runoff = 1.38 cfs @ 12.09 hrs, Volume= 0.114 af, Depth= 4.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
13,690	98	Roofs, HSG B
13,690		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 38S: POST 25.A**

Runoff = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af, Depth= 2.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
1,914	98	Roofs, HSG A
10,842	98	Unconnected pavement, HSG B
7,499	61	>75% Grass cover, Good, HSG B
20,255	84	Weighted Average
7,499		37.02% Pervious Area
12,756		62.98% Impervious Area
10,842		85.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 41S: POST 26.A**

Runoff = 0.72 cfs @ 12.09 hrs, Volume= 0.055 af, Depth= 3.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description
4,658	98	Paved parking, HSG C
1,501	98	Paved parking, HSG B
352	61	>75% Grass cover, Good, HSG B
1,070	74	>75% Grass cover, Good, HSG C
7,581	93	Weighted Average
1,422		18.76% Pervious Area
6,159		81.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Pond 5P: INFIL BASIN #1**

Inflow Area = 8.395 ac, 42.27% Impervious, Inflow Depth = 2.12" for 10-year event

Inflow = 16.62 cfs @ 12.10 hrs, Volume= 1.483 af

Outflow = 1.94 cfs @ 13.16 hrs, Volume= 1.483 af, Atten= 88%, Lag= 63.8 min

Discarded = 0.47 cfs @ 13.16 hrs, Volume= 0.760 af

Primary = 1.47 cfs @ 13.16 hrs, Volume= 0.723 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**6092 - POST Toll Rev4 (current)**

Prepared by {enter your company name here}

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Type III 24-hr 10-year Rainfall=4.60"

Printed 7/17/2019

Page 55

Peak Elev= 315.07' @ 13.16 hrs Surf.Area= 8,494 sf Storage= 33,723 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 448.1 min ( 1,266.9 - 818.8 )

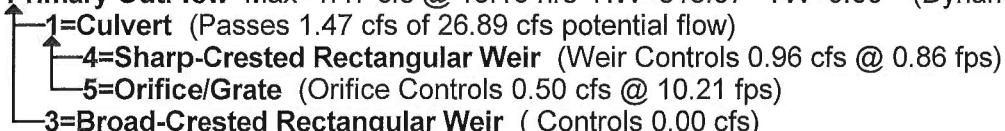
Volume	Invert	Avail.Storage	Storage Description		
#1	309.00'	42,092 cf	<b>Custom Stage Data (Irregular) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
309.00	2,439	200.1	0	0	2,439
310.00	3,839	242.8	3,113	3,113	3,960
312.00	5,491	281.0	9,281	12,394	5,636
314.00	7,393	320.0	12,837	25,230	7,596
316.00	9,513	355.0	16,862	42,092	9,594

Device	Routing	Invert	Outlet Devices
#1	Primary	309.00'	<b>24.0" Round Culvert</b> L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 309.00' / 308.00' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Discarded	309.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 306.73'
#3	Primary	315.60'	<b>20.0' long x 12.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.57 2.62 2.70 2.67 2.66 2.67 2.66 2.64
#4	Device 1	315.00'	<b>16.0' long Sharp-Crested Rectangular Weir</b> 0 End Contraction(s)
#5	Device 1	310.45'	<b>3.0" Vert. Orifice/Grate</b> C= 0.600

**Discarded OutFlow** Max=0.47 cfs @ 13.16 hrs HW=315.07' (Free Discharge)  
 ↗ 2=Exfiltration ( Controls 0.47 cfs)

**Primary OutFlow** Max=1.47 cfs @ 13.16 hrs HW=315.07' TW=0.00' (Dynamic Tailwater)



### Summary for Pond 6P: INFIL BASIN #2

Inflow Area =	10.289 ac, 22.88% Impervious, Inflow Depth = 2.01"	for 10-year event
Inflow =	15.66 cfs @ 12.15 hrs, Volume=	1.727 af
Outflow =	1.28 cfs @ 15.06 hrs, Volume=	1.727 af, Atten= 92%, Lag= 174.5 min
Discarded =	1.01 cfs @ 15.06 hrs, Volume=	1.442 af
Primary =	0.27 cfs @ 15.06 hrs, Volume=	0.286 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 317.03' @ 15.06 hrs Surf.Area= 27,983 sf Storage= 41,465 cf

Plug-Flow detention time= 453.8 min calculated for 1.726 af (100% of inflow)  
 Center-of-Mass det. time= 454.3 min ( 1,290.6 - 836.2 )

Volume	Invert	Avail.Storage	Storage Description		
#1	314.00'	118,862 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
314.00	2,513	223.0	0	0	2,513
316.00	17,939	667.0	18,111	18,111	33,971
318.00	39,591	937.0	56,120	74,231	68,472
319.00	49,868	1,019.0	44,631	118,862	81,273

Device	Routing	Invert	Outlet Devices
#1	Primary	314.00'	<b>24.0" Round Culvert</b> L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.00' / 312.60' S= 0.0280 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Discarded	314.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 312.00'
#3	Primary	317.80'	<b>25.0' long x 20.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#4	Device 1	317.40'	<b>16.0' long Sharp-Crested Rectangular Weir</b> 0 End Contraction(s)
#5	Device 1	315.55'	<b>3.0" Vert. Orifice/Grate</b> C= 0.600

**Discarded OutFlow** Max=1.01 cfs @ 15.06 hrs HW=317.03' (Free Discharge)

↑ 2=Exfiltration (Controls 1.01 cfs)

**Primary OutFlow** Max=0.27 cfs @ 15.06 hrs HW=317.03' TW=0.00' (Dynamic Tailwater)

↑ 1=Culvert (Passes 0.27 cfs of 17.00 cfs potential flow)  
    ↑ 4=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)  
    ↑ 5=Orifice/Grate (Orifice Controls 0.27 cfs @ 5.60 fps)  
    3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Summary for Pond 7P: DMH-2

Inflow Area = 4.772 ac, 49.56% Impervious, Inflow Depth = 2.32" for 10-year event  
 Inflow = 11.80 cfs @ 12.10 hrs, Volume= 0.921 af  
 Outflow = 11.80 cfs @ 12.10 hrs, Volume= 0.921 af, Atten= 0%, Lag= 0.0 min  
 Primary = 11.80 cfs @ 12.10 hrs, Volume= 0.921 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 315.87' @ 12.10 hrs

Flood Elev= 319.42'

Device	Routing	Invert	Outlet Devices
#1	Primary	313.90'	<b>24.0" Round Culvert</b> L= 60.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 313.90' / 312.70' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=11.77 cfs @ 12.10 hrs HW=315.86' TW=312.64' (Dynamic Tailwater)  
 ↑=Culvert (Inlet Controls 11.77 cfs @ 3.76 fps)

### Summary for Pond 8P: CB-5

Inflow Area = 0.536 ac, 69.71% Impervious, Inflow Depth = 3.19" for 10-year event  
 Inflow = 1.94 cfs @ 12.09 hrs, Volume= 0.143 af  
 Outflow = 1.94 cfs @ 12.09 hrs, Volume= 0.143 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.94 cfs @ 12.09 hrs, Volume= 0.143 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 316.18' @ 12.12 hrs  
 Flood Elev= 319.26'

Device	Routing	Invert	Outlet Devices
#1	Primary	315.26'	<b>12.0" Round Culvert</b> L= 20.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 315.26' / 314.90' S= 0.0180 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.65 cfs @ 12.09 hrs HW=316.16' TW=315.82' (Dynamic Tailwater)  
 ↑=Culvert (Inlet Controls 1.65 cfs @ 2.21 fps)

### Summary for Pond 9P: CB-6

Inflow Area = 0.416 ac, 86.61% Impervious, Inflow Depth = 3.82" for 10-year event  
 Inflow = 1.70 cfs @ 12.09 hrs, Volume= 0.132 af  
 Outflow = 1.70 cfs @ 12.09 hrs, Volume= 0.132 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.70 cfs @ 12.09 hrs, Volume= 0.132 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 316.48' @ 12.09 hrs  
 Flood Elev= 319.65'

Device	Routing	Invert	Outlet Devices
#1	Primary	315.65'	<b>12.0" Round Culvert</b> L= 20.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 315.65' / 314.90' S= 0.0375 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.66 cfs @ 12.09 hrs HW=316.46' TW=315.81' (Dynamic Tailwater)  
 ↑=Culvert (Inlet Controls 1.66 cfs @ 2.42 fps)

### Summary for Pond 10P: CB-4

Inflow Area = 0.486 ac, 82.05% Impervious, Inflow Depth = 3.59" for 10-year event  
 Inflow = 1.93 cfs @ 12.09 hrs, Volume= 0.146 af  
 Outflow = 1.93 cfs @ 12.09 hrs, Volume= 0.146 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.93 cfs @ 12.09 hrs, Volume= 0.146 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 315.73' @ 12.09 hrs

Flood Elev= 318.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	314.82'	<b>12.0" Round Culvert</b> L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.82' / 314.40' S= 0.0168 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.88 cfs @ 12.09 hrs HW=315.71' TW=312.54' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 1.88 cfs @ 2.54 fps)

### **Summary for Pond 11P: DMH-3**

Inflow Area = 3.820 ac, 42.71% Impervious, Inflow Depth = 2.03" for 10-year event  
 Inflow = 8.19 cfs @ 12.10 hrs, Volume= 0.646 af  
 Outflow = 8.19 cfs @ 12.10 hrs, Volume= 0.646 af, Atten= 0%, Lag= 0.0 min  
 Primary = 8.19 cfs @ 12.10 hrs, Volume= 0.646 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 318.58' @ 12.10 hrs

Flood Elev= 321.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	317.10'	<b>24.0" Round Culvert</b> L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 317.10' / 314.90' S= 0.0349 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=8.12 cfs @ 12.10 hrs HW=318.58' TW=315.85' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 8.12 cfs @ 3.27 fps)

### **Summary for Pond 12P: DMH-4**

Inflow Area = 3.355 ac, 39.90% Impervious, Inflow Depth = 1.91" for 10-year event  
 Inflow = 6.67 cfs @ 12.11 hrs, Volume= 0.534 af  
 Outflow = 6.67 cfs @ 12.11 hrs, Volume= 0.534 af, Atten= 0%, Lag= 0.0 min  
 Primary = 6.67 cfs @ 12.11 hrs, Volume= 0.534 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 323.26' @ 12.11 hrs

Flood Elev= 326.96'

Device	Routing	Invert	Outlet Devices
#1	Primary	321.96'	<b>24.0" Round Culvert</b> L= 99.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 321.96' / 317.20' S= 0.0481 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=6.56 cfs @ 12.11 hrs HW=323.25' TW=318.57' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 6.56 cfs @ 3.06 fps)

### Summary for Pond 13P: DMH-6

Inflow Area = 0.912 ac, 77.06% Impervious, Inflow Depth = 3.08" for 10-year event  
 Inflow = 3.19 cfs @ 12.09 hrs, Volume= 0.234 af  
 Outflow = 3.19 cfs @ 12.09 hrs, Volume= 0.234 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.19 cfs @ 12.09 hrs, Volume= 0.234 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 336.22' @ 12.09 hrs  
 Flood Elev= 340.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.25'	<b>18.0" Round Culvert</b> L= 124.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.25' / 328.11' S= 0.0576 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.12 cfs @ 12.09 hrs HW=336.20' TW=328.96' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 3.12 cfs @ 2.63 fps)

### Summary for Pond 14P: CB-8

Inflow Area = 0.336 ac, 81.36% Impervious, Inflow Depth = 3.39" for 10-year event  
 Inflow = 1.27 cfs @ 12.09 hrs, Volume= 0.095 af  
 Outflow = 1.27 cfs @ 12.09 hrs, Volume= 0.095 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.27 cfs @ 12.09 hrs, Volume= 0.095 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 337.23' @ 12.09 hrs  
 Flood Elev= 340.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	336.55'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 336.55' / 335.75' S= 0.0571 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.24 cfs @ 12.09 hrs HW=337.22' TW=336.20' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 1.24 cfs @ 2.21 fps)

### Summary for Pond 15P: CB-7

Inflow Area = 0.576 ac, 74.55% Impervious, Inflow Depth = 2.91" for 10-year event  
 Inflow = 1.91 cfs @ 12.09 hrs, Volume= 0.140 af  
 Outflow = 1.91 cfs @ 12.09 hrs, Volume= 0.140 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.91 cfs @ 12.09 hrs, Volume= 0.140 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 336.86' @ 12.09 hrs

Flood Elev= 339.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.95'	<b>12.0" Round Culvert</b> L= 10.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.95' / 335.75' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.88 cfs @ 12.09 hrs HW=336.84' TW=336.21' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 1.88 cfs @ 2.54 fps)

### **Summary for Pond 16P: DMH-5**

Inflow Area = 0.912 ac, 77.06% Impervious, Inflow Depth = 3.08" for 10-year event  
 Inflow = 3.19 cfs @ 12.09 hrs, Volume= 0.234 af  
 Outflow = 3.19 cfs @ 12.09 hrs, Volume= 0.234 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.19 cfs @ 12.09 hrs, Volume= 0.234 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 328.98' @ 12.09 hrs

Flood Elev= 332.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	328.01'	<b>18.0" Round Culvert</b> L= 91.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 328.01' / 322.56' S= 0.0599 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.12 cfs @ 12.09 hrs HW=328.96' TW=323.24' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 3.12 cfs @ 2.63 fps)

### **Summary for Pond 17P: DMH-15**

Inflow Area = 0.505 ac, 63.46% Impervious, Inflow Depth = 3.00" for 10-year event  
 Inflow = 1.66 cfs @ 12.09 hrs, Volume= 0.126 af  
 Outflow = 1.66 cfs @ 12.09 hrs, Volume= 0.126 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.66 cfs @ 12.09 hrs, Volume= 0.126 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 336.36' @ 12.09 hrs

Flood Elev= 340.37'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.55'	<b>12.0" Round Culvert</b> L= 69.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.55' / 334.86' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.63 cfs @ 12.09 hrs HW=336.35' TW=335.56' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 1.63 cfs @ 2.41 fps)

### Summary for Pond 18P: CB-17

Inflow Area = 0.154 ac, 98.52% Impervious, Inflow Depth = 4.25" for 10-year event  
 Inflow = 0.67 cfs @ 12.09 hrs, Volume= 0.054 af  
 Outflow = 0.67 cfs @ 12.09 hrs, Volume= 0.054 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.67 cfs @ 12.09 hrs, Volume= 0.054 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 336.45' @ 12.13 hrs  
 Flood Elev= 339.81'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.81'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.81' / 335.65' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.43 cfs @ 12.09 hrs HW=336.41' TW=336.35' (Dynamic Tailwater)  
 ↑1=Culvert (Outlet Controls 0.43 cfs @ 1.27 fps)

### Summary for Pond 19P: CB-16

Inflow Area = 0.351 ac, 48.14% Impervious, Inflow Depth = 2.46" for 10-year event  
 Inflow = 0.99 cfs @ 12.09 hrs, Volume= 0.072 af  
 Outflow = 0.99 cfs @ 12.09 hrs, Volume= 0.072 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.99 cfs @ 12.09 hrs, Volume= 0.072 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 336.53' @ 12.12 hrs  
 Flood Elev= 339.81'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.81'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.81' / 335.65' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.82 cfs @ 12.09 hrs HW=336.50' TW=336.35' (Dynamic Tailwater)  
 ↑1=Culvert (Outlet Controls 0.82 cfs @ 1.99 fps)

### Summary for Pond 20P: DMH-14

Inflow Area = 0.505 ac, 63.46% Impervious, Inflow Depth = 3.00" for 10-year event  
 Inflow = 1.66 cfs @ 12.09 hrs, Volume= 0.126 af  
 Outflow = 1.66 cfs @ 12.09 hrs, Volume= 0.126 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.66 cfs @ 12.09 hrs, Volume= 0.126 af

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Page 62

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 335.57' @ 12.09 hrs

Flood Elev= 341.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	334.76'	<b>12.0" Round Culvert</b> L= 88.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 334.76' / 333.88' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.63 cfs @ 12.09 hrs HW=335.56' TW=334.41' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 1.63 cfs @ 2.41 fps)

**Summary for Pond 21P: DMH-13**

Inflow Area = 1.039 ac, 73.61% Impervious, Inflow Depth = 3.16" for 10-year event  
 Inflow = 3.60 cfs @ 12.09 hrs, Volume= 0.274 af  
 Outflow = 3.60 cfs @ 12.09 hrs, Volume= 0.274 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.60 cfs @ 12.09 hrs, Volume= 0.274 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 334.42' @ 12.09 hrs

Flood Elev= 342.06'

Device	Routing	Invert	Outlet Devices
#1	Primary	333.38'	<b>18.0" Round Culvert</b> L= 136.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 333.38' / 330.66' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.52 cfs @ 12.09 hrs HW=334.41' TW=331.59' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 3.52 cfs @ 2.73 fps)

**Summary for Pond 22P: CB-14**

Inflow Area = 0.373 ac, 76.57% Impervious, Inflow Depth = 2.91" for 10-year event  
 Inflow = 1.24 cfs @ 12.09 hrs, Volume= 0.090 af  
 Outflow = 1.24 cfs @ 12.09 hrs, Volume= 0.090 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.24 cfs @ 12.09 hrs, Volume= 0.090 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 338.88' @ 12.09 hrs

Flood Elev= 342.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	338.21'	<b>12.0" Round Culvert</b> L= 17.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 338.21' / 338.00' S= 0.0124 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.21 cfs @ 12.09 hrs HW=338.87' TW=334.41' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 1.21 cfs @ 2.19 fps)

### Summary for Pond 23P: CB-15

Inflow Area = 0.161 ac, 98.59% Impervious, Inflow Depth = 4.25" for 10-year event  
 Inflow = 0.70 cfs @ 12.09 hrs, Volume= 0.057 af  
 Outflow = 0.70 cfs @ 12.09 hrs, Volume= 0.057 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.70 cfs @ 12.09 hrs, Volume= 0.057 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 338.79' @ 12.09 hrs  
 Flood Elev= 342.31'

Device	Routing	Invert	Outlet Devices
#1	Primary	338.31'	<b>12.0" Round Culvert</b> L= 28.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 338.31' / 338.00' S= 0.0111 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.68 cfs @ 12.09 hrs HW=338.79' TW=334.40' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 0.68 cfs @ 1.85 fps)

### Summary for Pond 24P: DMH-12

Inflow Area = 1.039 ac, 73.61% Impervious, Inflow Depth = 3.16" for 10-year event  
 Inflow = 3.60 cfs @ 12.09 hrs, Volume= 0.274 af  
 Outflow = 3.60 cfs @ 12.09 hrs, Volume= 0.274 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.60 cfs @ 12.09 hrs, Volume= 0.274 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 331.60' @ 12.09 hrs  
 Flood Elev= 338.71'

Device	Routing	Invert	Outlet Devices
#1	Primary	330.56'	<b>18.0" Round Culvert</b> L= 99.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 330.56' / 327.84' S= 0.0275 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.52 cfs @ 12.09 hrs HW=331.59' TW=328.77' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 3.52 cfs @ 2.73 fps)

### Summary for Pond 25P: DMH-11

Inflow Area = 1.039 ac, 73.61% Impervious, Inflow Depth = 3.16" for 10-year event  
 Inflow = 3.60 cfs @ 12.09 hrs, Volume= 0.274 af  
 Outflow = 3.60 cfs @ 12.09 hrs, Volume= 0.274 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.60 cfs @ 12.09 hrs, Volume= 0.274 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 328.78' @ 12.09 hrs

Flood Elev= 332.34'

Device	Routing	Invert	Outlet Devices
#1	Primary	327.74'	<b>18.0" Round Culvert</b> L= 76.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 327.74' / 322.02' S= 0.0753 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.52 cfs @ 12.09 hrs HW=328.77' TW=323.49' (Dynamic Tailwater)  
  (Inlet Controls 3.52 cfs @ 2.73 fps)

### Summary for Pond 26P: DMH-10

Inflow Area = 1.707 ac, 77.06% Impervious, Inflow Depth = 3.34" for 10-year event  
 Inflow = 6.24 cfs @ 12.09 hrs, Volume= 0.475 af  
 Outflow = 6.24 cfs @ 12.09 hrs, Volume= 0.475 af, Atten= 0%, Lag= 0.0 min  
 Primary = 6.24 cfs @ 12.09 hrs, Volume= 0.475 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 323.53' @ 12.09 hrs

Flood Elev= 327.14'

Device	Routing	Invert	Outlet Devices
#1	Primary	321.92'	<b>18.0" Round Culvert</b> L= 174.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 321.92' / 316.81' S= 0.0294 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=6.09 cfs @ 12.09 hrs HW=323.49' TW=317.96' (Dynamic Tailwater)  
  (Inlet Controls 6.09 cfs @ 3.44 fps)

### Summary for Pond 27P: CB-13

Inflow Area = 0.182 ac, 98.50% Impervious, Inflow Depth = 4.25" for 10-year event  
 Inflow = 0.79 cfs @ 12.09 hrs, Volume= 0.064 af  
 Outflow = 0.79 cfs @ 12.09 hrs, Volume= 0.064 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.79 cfs @ 12.09 hrs, Volume= 0.064 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 324.03' @ 12.09 hrs

Flood Elev= 327.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.51'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.51' / 323.23' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.77 cfs @ 12.09 hrs HW=324.02' TW=323.48' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 0.77 cfs @ 1.92 fps)

### Summary for Pond 28P: CB-12

Inflow Area = 0.486 ac, 76.42% Impervious, Inflow Depth = 3.39" for 10-year event  
 Inflow = 1.84 cfs @ 12.09 hrs, Volume= 0.137 af  
 Outflow = 1.84 cfs @ 12.09 hrs, Volume= 0.137 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.84 cfs @ 12.09 hrs, Volume= 0.137 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 324.39' @ 12.09 hrs  
 Flood Elev= 327.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.51'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.51' / 323.23' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.80 cfs @ 12.09 hrs HW=324.37' TW=323.49' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 1.80 cfs @ 2.50 fps)

### Summary for Pond 29P: DMH-9

Inflow Area = 2.440 ac, 76.17% Impervious, Inflow Depth = 3.33" for 10-year event  
 Inflow = 8.95 cfs @ 12.09 hrs, Volume= 0.677 af  
 Outflow = 8.95 cfs @ 12.09 hrs, Volume= 0.677 af, Atten= 0%, Lag= 0.0 min  
 Primary = 8.95 cfs @ 12.09 hrs, Volume= 0.677 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 317.99' @ 12.09 hrs  
 Flood Elev= 322.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	316.31'	<b>24.0" Round Culvert</b> L= 62.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 316.31' / 316.00' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=8.74 cfs @ 12.09 hrs HW=317.96' TW=315.72' (Dynamic Tailwater)  
 ↑1=Culvert (Barrel Controls 8.74 cfs @ 4.27 fps)

### Summary for Pond 30P: CB-11

Inflow Area = 0.734 ac, 74.09% Impervious, Inflow Depth = 3.29" for 10-year event  
 Inflow = 2.72 cfs @ 12.09 hrs, Volume= 0.201 af  
 Outflow = 2.72 cfs @ 12.09 hrs, Volume= 0.201 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.72 cfs @ 12.09 hrs, Volume= 0.201 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 318.31' @ 12.12 hrs

Flood Elev= 321.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	317.30'	<b>18.0" Round Culvert</b> L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 317.30' / 316.81' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=2.22 cfs @ 12.09 hrs HW=318.27' TW=317.96' (Dynamic Tailwater)  
 ↑ 1=Culvert (Outlet Controls 2.22 cfs @ 2.62 fps)

### Summary for Pond 31P: PROP. CULVERT

Inflow Area = 2.338 ac, 19.03% Impervious, Inflow Depth = 1.26" for 10-year event  
 Inflow = 2.02 cfs @ 12.35 hrs, Volume= 0.246 af  
 Outflow = 2.02 cfs @ 12.35 hrs, Volume= 0.246 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.02 cfs @ 12.35 hrs, Volume= 0.246 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 315.09' @ 13.16 hrs

Flood Elev= 318.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	314.00'	<b>15.0" Round Culvert</b> L= 75.8' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.00' / 313.00' S= 0.0132 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf

**Primary OutFlow** Max=2.01 cfs @ 12.35 hrs HW=314.81' TW=314.08' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 2.01 cfs @ 2.41 fps)

### Summary for Pond 32P: CB-10

Inflow Area = 0.106 ac, 100.00% Impervious, Inflow Depth = 4.36" for 10-year event  
 Inflow = 0.47 cfs @ 12.09 hrs, Volume= 0.039 af  
 Outflow = 0.47 cfs @ 12.09 hrs, Volume= 0.039 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.47 cfs @ 12.09 hrs, Volume= 0.039 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 323.76' @ 12.23 hrs

Flood Elev= 327.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.00'	<b>18.0" Round Culvert</b> L= 55.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.00' / 322.73' S= 0.0049 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=323.48' TW=323.54' (Dynamic Tailwater)  
 ↑  
 └─1=Culvert (Controls 0.00 cfs)

### Summary for Pond 33P: CB-9

Inflow Area = 0.119 ac, 100.00% Impervious, Inflow Depth = 4.36" for 10-year event  
 Inflow = 0.52 cfs @ 12.09 hrs, Volume= 0.043 af  
 Outflow = 0.52 cfs @ 12.09 hrs, Volume= 0.043 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.52 cfs @ 12.09 hrs, Volume= 0.043 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 323.60' @ 12.19 hrs  
 Flood Elev= 327.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.00'	<b>18.0" Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.00' / 322.52' S= 0.0686 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=323.41' TW=323.46' (Dynamic Tailwater)  
 ↑  
 └─1=Culvert (Controls 0.00 cfs)

### Summary for Pond 34P: CB-18

Inflow Area = 0.465 ac, 62.98% Impervious, Inflow Depth = 2.91" for 10-year event  
 Inflow = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af  
 Outflow = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 320.77' @ 12.09 hrs  
 Flood Elev= 325.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	320.00'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 320.00' / 319.04' S= 0.0600 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.51 cfs @ 12.09 hrs HW=320.76' TW=319.70' (Dynamic Tailwater)  
 ↑  
 └─1=Culvert (Inlet Controls 1.51 cfs @ 2.35 fps)

### Summary for Pond 35P: DMH-8

Inflow Area = 2.324 ac, 22.24% Impervious, Inflow Depth = 1.32" for 10-year event  
 Inflow = 3.09 cfs @ 12.13 hrs, Volume= 0.256 af  
 Outflow = 3.09 cfs @ 12.13 hrs, Volume= 0.256 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.09 cfs @ 12.13 hrs, Volume= 0.256 af

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Type III 24-hr 10-year Rainfall=4.60"

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Page 68

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 323.75' @ 12.18 hrs

Flood Elev= 327.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	322.63'	<b>24.0" Round Culvert</b> L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 322.63' / 322.52' S= 0.0052 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.20 cfs @ 12.13 hrs HW=323.69' TW=323.56' (Dynamic Tailwater)  
 ↑ 1=Culvert (Outlet Controls 2.20 cfs @ 1.90 fps)

**Summary for Pond 36P: DMH-16**

Inflow Area = 0.465 ac, 62.98% Impervious, Inflow Depth = 2.91" for 10-year event  
 Inflow = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af  
 Outflow = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 319.71' @ 12.09 hrs

Flood Elev= 326.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	318.94'	<b>12.0" Round Culvert</b> L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 318.94' / 317.20' S= 0.0829 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.51 cfs @ 12.09 hrs HW=319.70' TW=318.56' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 1.51 cfs @ 2.35 fps)

**Summary for Pond 37P: DMH-7**

Inflow Area = 2.443 ac, 26.02% Impervious, Inflow Depth = 1.47" for 10-year event  
 Inflow = 3.58 cfs @ 12.12 hrs, Volume= 0.299 af  
 Outflow = 3.58 cfs @ 12.12 hrs, Volume= 0.299 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.58 cfs @ 12.12 hrs, Volume= 0.299 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 323.59' @ 12.15 hrs

Flood Elev= 327.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	322.42'	<b>24.0" Round Culvert</b> L= 78.1' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 322.42' / 321.96' S= 0.0059 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.32 cfs @ 12.12 hrs HW=323.55' TW=323.23' (Dynamic Tailwater)  
 ↑—1=Culvert (Outlet Controls 3.32 cfs @ 2.62 fps)

### Summary for Pond 39P: CB-2 [EXISTING]

Inflow Area = 0.432 ac, 80.78% Impervious, Inflow Depth = 3.81" for 10-year event  
 Inflow = 1.78 cfs @ 12.09 hrs, Volume= 0.137 af  
 Outflow = 1.78 cfs @ 12.09 hrs, Volume= 0.137 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.78 cfs @ 12.09 hrs, Volume= 0.137 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 311.16' @ 12.10 hrs  
 Flood Elev= 314.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.05'	<b>12.0" Round Culvert</b> L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.05' / 310.00' S= 0.0010 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.63 cfs @ 12.09 hrs HW=311.14' TW=310.84' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 1.63 cfs @ 2.07 fps)

### Summary for Pond 40P: CB-1 [EXISTING]

Inflow Area = 0.174 ac, 81.24% Impervious, Inflow Depth = 3.81" for 10-year event  
 Inflow = 0.72 cfs @ 12.09 hrs, Volume= 0.055 af  
 Outflow = 0.72 cfs @ 12.09 hrs, Volume= 0.055 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.72 cfs @ 12.09 hrs, Volume= 0.055 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 311.30' @ 12.12 hrs  
 Flood Elev= 314.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.75'	<b>12.0" Round Culvert</b> L= 17.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.75' / 310.15' S= 0.0353 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.54 cfs @ 12.09 hrs HW=311.28' TW=311.14' (Dynamic Tailwater)  
 ↑—1=Culvert (Outlet Controls 0.54 cfs @ 1.87 fps)

### Summary for Pond 42P: DMH-1 [EXISTING]

Inflow Area = 0.432 ac, 80.78% Impervious, Inflow Depth = 3.81" for 10-year event  
 Inflow = 1.78 cfs @ 12.09 hrs, Volume= 0.137 af  
 Outflow = 1.78 cfs @ 12.09 hrs, Volume= 0.137 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.78 cfs @ 12.09 hrs, Volume= 0.137 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 310.86' @ 12.09 hrs

Flood Elev= 315.33'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.00'	<b>12.0" Round Culvert</b> L= 92.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.00' / 308.02' S= 0.0215 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.73 cfs @ 12.09 hrs HW=310.84' TW=0.00' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 1.73 cfs @ 2.46 fps)

### Summary for Pond 43P: SWALE & FIELD BASIN

Inflow Area =	2.218 ac, 18.52% Impervious, Inflow Depth = 1.26"	for 10-year event
Inflow =	2.71 cfs @ 12.13 hrs, Volume=	0.233 af
Outflow =	2.72 cfs @ 12.14 hrs, Volume=	0.233 af, Atten= 0%, Lag= 0.7 min
Discarded =	0.02 cfs @ 12.14 hrs, Volume=	0.016 af
Primary =	2.70 cfs @ 12.14 hrs, Volume=	0.217 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 328.16' @ 12.14 hrs Surf.Area= 753 sf Storage= 110 cf

Plug-Flow detention time= 1.2 min calculated for 0.233 af (100% of inflow)  
 Center-of-Mass det. time= 1.2 min ( 875.3 - 874.1 )

Volume	Invert	Avail.Storage	Storage Description		
#1	328.00'	1,150 cf	<b>Custom Stage Data (Irregular) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
328.00	600	119.4	0	0	600
329.00	1,808	444.3	1,150	1,150	15,177

Device	Routing	Invert	Outlet Devices
#1	Discarded	328.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 320.50'
#2	Primary	324.00'	<b>18.0" Round Culvert</b> L= 51.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 324.00' / 322.73' S= 0.0249 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#3	Device 2	328.00'	<b>48.0" Horiz. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.02 cfs @ 12.14 hrs HW=328.16' (Free Discharge)  
 ↑ 1=Exfiltration ( Controls 0.02 cfs )

**Primary OutFlow** Max=2.66 cfs @ 12.14 hrs HW=328.16' TW=323.71' (Dynamic Tailwater)  
 ↑ 2=Culvert (Passes 2.66 cfs of 12.41 cfs potential flow)  
 ↑ 3=Orifice/Grate (Weir Controls 2.66 cfs @ 1.31 fps)

**Summary for Link 4L: DP-A**

Inflow Area = 23.100 ac, 27.90% Impervious, Inflow Depth = 0.86" for 10-year event  
Inflow = 8.78 cfs @ 12.10 hrs, Volume= 1.651 af  
Primary = 8.78 cfs @ 12.10 hrs, Volume= 1.651 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Summary for Link 5L: DP-B**

Inflow Area = 0.891 ac, 0.00% Impervious, Inflow Depth = 0.08" for 10-year event  
Inflow = 0.01 cfs @ 15.20 hrs, Volume= 0.006 af  
Primary = 0.01 cfs @ 15.20 hrs, Volume= 0.006 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment 1S: POST 1.A</b>	Runoff Area=11,232 sf 80.47% Impervious Runoff Depth=4.59" Tc=6.0 min CN=93 Runoff=1.27 cfs 0.099 af
<b>Subcatchment 2S: POST 2.A</b>	Runoff Area=23,362 sf 69.71% Impervious Runoff Depth=3.95" Tc=6.0 min CN=87 Runoff=2.37 cfs 0.176 af
<b>Subcatchment 5S: POST 6.A</b>	Runoff Area=101,865 sf 19.03% Impervious Runoff Depth=1.77" Flow Length=1,092' Tc=22.1 min CN=63 Runoff=2.95 cfs 0.345 af
<b>Subcatchment 6S: POST 7.A</b>	Runoff Area=21,169 sf 82.05% Impervious Runoff Depth=4.37" Tc=6.0 min CN=91 Runoff=2.32 cfs 0.177 af
<b>Subcatchment 7S: POST 8.A</b>	Runoff Area=21,105 sf 5.32% Impervious Runoff Depth=1.93" Tc=6.0 min UI Adjusted CN=65 Runoff=1.04 cfs 0.078 af
<b>Subcatchment 8S: POST 3.A</b>	Runoff Area=5,197 sf 100.00% Impervious Runoff Depth=5.16" Tc=6.0 min CN=98 Runoff=0.62 cfs 0.051 af
<b>Subcatchment 9S: POST 4.A</b>	Runoff Area=25,089 sf 74.55% Impervious Runoff Depth=3.64" Tc=6.0 min CN=84 Runoff=2.38 cfs 0.175 af
<b>Subcatchment 10S: POST 5.A</b>	Runoff Area=14,640 sf 81.36% Impervious Runoff Depth=4.16" Tc=6.0 min CN=89 Runoff=1.55 cfs 0.116 af
<b>Subcatchment 11S: POST 9.A</b>	Runoff Area=12,906 sf 81.22% Impervious Runoff Depth=4.37" Tc=6.0 min CN=91 Runoff=1.41 cfs 0.108 af
<b>Subcatchment 12S: POST 12.A</b>	Runoff Area=96,627 sf 18.52% Impervious Runoff Depth=1.77" Tc=8.0 min UI Adjusted CN=63 Runoff=4.01 cfs 0.327 af
<b>Subcatchment 13S: POST 14.A</b>	Runoff Area=7,009 sf 98.59% Impervious Runoff Depth=5.05" Tc=6.0 min CN=97 Runoff=0.83 cfs 0.068 af
<b>Subcatchment 14S: POST 15.A</b>	Runoff Area=16,242 sf 76.57% Impervious Runoff Depth=3.64" Tc=6.0 min CN=84 Runoff=1.54 cfs 0.113 af
<b>Subcatchment 15S: POST 13.A</b>	Runoff Area=15,308 sf 48.14% Impervious Runoff Depth=3.15" Tc=6.0 min CN=79 Runoff=1.27 cfs 0.092 af
<b>Subcatchment 16S: POST 16.A</b>	Runoff Area=6,688 sf 98.52% Impervious Runoff Depth=5.05" Tc=6.0 min CN=97 Runoff=0.79 cfs 0.065 af
<b>Subcatchment 17S: POST 18.A</b>	Runoff Area=7,933 sf 98.50% Impervious Runoff Depth=5.05" Tc=6.0 min CN=97 Runoff=0.93 cfs 0.077 af
<b>Subcatchment 18S: POST 17.A</b>	Runoff Area=21,158 sf 76.42% Impervious Runoff Depth=4.16" Tc=6.0 min CN=89 Runoff=2.24 cfs 0.168 af

<b>Subcatchment 19S: POST 24.A</b>	Runoff Area=45,444 sf 0.00% Impervious Runoff Depth=2.60" Tc=6.0 min CN=73 Runoff=3.11 cfs 0.226 af
<b>Subcatchment 20S: POST 19.A</b>	Runoff Area=31,969 sf 74.09% Impervious Runoff Depth=4.05" Tc=6.0 min CN=88 Runoff=3.31 cfs 0.248 af
<b>Subcatchment 21S: POST 20.A</b>	Runoff Area=54,029 sf 13.37% Impervious Runoff Depth=2.25" Tc=6.0 min CN=69 Runoff=3.17 cfs 0.233 af
<b>Subcatchment 22S: POST 22.A</b>	Runoff Area=165,736 sf 13.02% Impervious Runoff Depth=1.62" Flow Length=884' Tc=15.2 min CN=61 Runoff=4.96 cfs 0.512 af
<b>Subcatchment 23S: POST 23.A</b>	Runoff Area=176,164 sf 0.00% Impervious Runoff Depth=2.69" Tc=16.7 min CN=74 Runoff=9.16 cfs 0.906 af
<b>Subcatchment 24S: POST 21.A</b>	Runoff Area=74,028 sf 1.61% Impervious Runoff Depth=1.62" Tc=6.0 min UI Adjusted CN=61 Runoff=2.95 cfs 0.229 af
<b>Subcatchment 25S: POST 1.B</b>	Runoff Area=38,793 sf 0.00% Impervious Runoff Depth=0.21" Flow Length=106' Tc=11.1 min CN=37 Runoff=0.03 cfs 0.016 af
<b>Subcatchment 26S: POST 10.A</b>	Runoff Area=4,620 sf 100.00% Impervious Runoff Depth=5.16" Tc=6.0 min CN=98 Runoff=0.55 cfs 0.046 af
<b>Subcatchment 27S: POST 11.A</b>	Runoff Area=5,180 sf 100.00% Impervious Runoff Depth=5.16" Tc=6.0 min CN=98 Runoff=0.61 cfs 0.051 af
<b>Subcatchment 28S: BACK ROOFS OF</b>	Runoff Area=13,690 sf 100.00% Impervious Runoff Depth=5.16" Tc=6.0 min CN=98 Runoff=1.62 cfs 0.135 af
<b>Subcatchment 38S: POST 25.A</b>	Runoff Area=20,255 sf 62.98% Impervious Runoff Depth=3.64" Tc=6.0 min CN=84 Runoff=1.92 cfs 0.141 af
<b>Subcatchment 41S: POST 26.A</b>	Runoff Area=7,581 sf 81.24% Impervious Runoff Depth=4.59" Tc=6.0 min CN=93 Runoff=0.86 cfs 0.067 af
<b>Pond 5P: INFIL BASIN #1</b>	Peak Elev=315.27' Storage=35,472 cf Inflow=21.35 cfs 1.909 af Discarded=0.49 cfs 0.810 af Primary=7.98 cfs 1.100 af Outflow=8.47 cfs 1.909 af
<b>Pond 6P: INFIL BASIN #2</b>	Peak Elev=317.45' Storage=54,251 cf Inflow=20.70 cfs 2.248 af Discarded=1.21 cfs 1.806 af Primary=0.84 cfs 0.442 af Outflow=2.05 cfs 2.248 af
<b>Pond 7P: DMH-2</b>	Peak Elev=316.49' Inflow=15.07 cfs 1.175 af 24.0" Round Culvert n=0.013 L=60.0' S=0.0200 '/' Outflow=15.07 cfs 1.175 af
<b>Pond 8P: CB-5</b>	Peak Elev=316.92' Inflow=2.37 cfs 0.176 af 12.0" Round Culvert n=0.013 L=20.0' S=0.0180 '/' Outflow=2.37 cfs 0.176 af
<b>Pond 9P: CB-6</b>	Peak Elev=316.79' Inflow=2.03 cfs 0.159 af 12.0" Round Culvert n=0.013 L=20.0' S=0.0375 '/' Outflow=2.03 cfs 0.159 af

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Page 74

**Pond 10P: CB-4**Peak Elev=315.92' Inflow=2.32 cfs 0.177 af  
12.0" Round Culvert n=0.013 L=25.0' S=0.0168 '/' Outflow=2.32 cfs 0.177 af**Pond 11P: DMH-3**Peak Elev=318.90' Inflow=10.71 cfs 0.839 af  
24.0" Round Culvert n=0.013 L=63.0' S=0.0349 '/' Outflow=10.71 cfs 0.839 af**Pond 12P: DMH-4**Peak Elev=323.52' Inflow=8.81 cfs 0.698 af  
24.0" Round Culvert n=0.013 L=99.0' S=0.0481 '/' Outflow=8.81 cfs 0.698 af**Pond 13P: DMH-6**Peak Elev=336.35' Inflow=3.93 cfs 0.291 af  
18.0" Round Culvert n=0.013 L=124.0' S=0.0576 '/' Outflow=3.93 cfs 0.291 af**Pond 14P: CB-8**Peak Elev=337.33' Inflow=1.55 cfs 0.116 af  
12.0" Round Culvert n=0.013 L=14.0' S=0.0571 '/' Outflow=1.55 cfs 0.116 af**Pond 15P: CB-7**Peak Elev=337.08' Inflow=2.38 cfs 0.175 af  
12.0" Round Culvert n=0.013 L=10.0' S=0.0200 '/' Outflow=2.38 cfs 0.175 af**Pond 16P: DMH-5**Peak Elev=329.11' Inflow=3.93 cfs 0.291 af  
18.0" Round Culvert n=0.013 L=91.0' S=0.0599 '/' Outflow=3.93 cfs 0.291 af**Pond 17P: DMH-15**Peak Elev=336.52' Inflow=2.06 cfs 0.157 af  
12.0" Round Culvert n=0.013 L=69.0' S=0.0100 '/' Outflow=2.06 cfs 0.157 af**Pond 18P: CB-17**Peak Elev=336.59' Inflow=0.79 cfs 0.065 af  
12.0" Round Culvert n=0.013 L=16.0' S=0.0100 '/' Outflow=0.79 cfs 0.065 af**Pond 19P: CB-16**Peak Elev=336.68' Inflow=1.27 cfs 0.092 af  
12.0" Round Culvert n=0.013 L=16.0' S=0.0100 '/' Outflow=1.27 cfs 0.092 af**Pond 20P: DMH-14**Peak Elev=335.73' Inflow=2.06 cfs 0.157 af  
12.0" Round Culvert n=0.013 L=88.0' S=0.0100 '/' Outflow=2.06 cfs 0.157 af**Pond 21P: DMH-13**Peak Elev=334.57' Inflow=4.42 cfs 0.337 af  
18.0" Round Culvert n=0.013 L=136.0' S=0.0200 '/' Outflow=4.42 cfs 0.337 af**Pond 22P: CB-14**Peak Elev=338.98' Inflow=1.54 cfs 0.113 af  
12.0" Round Culvert n=0.013 L=17.0' S=0.0124 '/' Outflow=1.54 cfs 0.113 af**Pond 23P: CB-15**Peak Elev=338.84' Inflow=0.83 cfs 0.068 af  
12.0" Round Culvert n=0.013 L=28.0' S=0.0111 '/' Outflow=0.83 cfs 0.068 af**Pond 24P: DMH-12**Peak Elev=331.75' Inflow=4.42 cfs 0.337 af  
18.0" Round Culvert n=0.013 L=99.0' S=0.0275 '/' Outflow=4.42 cfs 0.337 af**Pond 25P: DMH-11**Peak Elev=328.93' Inflow=4.42 cfs 0.337 af  
18.0" Round Culvert n=0.013 L=76.0' S=0.0753 '/' Outflow=4.42 cfs 0.337 af**Pond 26P: DMH-10**Peak Elev=323.94' Inflow=7.59 cfs 0.582 af  
18.0" Round Culvert n=0.013 L=174.0' S=0.0294 '/' Outflow=7.59 cfs 0.582 af

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Page 75

Pond 27P: CB-13	Peak Elev=324.12' Inflow=0.93 cfs 0.077 af 12.0" Round Culvert n=0.013 L=14.0' S=0.0200 '/' Outflow=0.93 cfs 0.077 af
Pond 28P: CB-12	Peak Elev=324.57' Inflow=2.24 cfs 0.168 af 12.0" Round Culvert n=0.013 L=14.0' S=0.0200 '/' Outflow=2.24 cfs 0.168 af
Pond 29P: DMH-9	Peak Elev=318.23' Inflow=10.91 cfs 0.830 af 24.0" Round Culvert n=0.013 L=62.0' S=0.0050 '/' Outflow=10.91 cfs 0.830 af
Pond 30P: CB-11	Peak Elev=318.51' Inflow=3.31 cfs 0.248 af 18.0" Round Culvert n=0.013 L=49.0' S=0.0100 '/' Outflow=3.31 cfs 0.248 af
Pond 31P: PROP. CULVERT	Peak Elev=315.56' Inflow=2.95 cfs 0.345 af 15.0" Round Culvert n=0.013 L=75.8' S=0.0132 '/' Outflow=2.95 cfs 0.345 af
Pond 32P: CB-10	Peak Elev=324.04' Inflow=0.55 cfs 0.046 af 18.0" Round Culvert n=0.013 L=55.0' S=0.0049 '/' Outflow=0.55 cfs 0.046 af
Pond 33P: CB-9	Peak Elev=323.87' Inflow=0.61 cfs 0.051 af 18.0" Round Culvert n=0.013 L=7.0' S=0.0686 '/' Outflow=0.61 cfs 0.051 af
Pond 34P: CB-18	Peak Elev=320.91' Inflow=1.92 cfs 0.141 af 12.0" Round Culvert n=0.013 L=16.0' S=0.0600 '/' Outflow=1.92 cfs 0.141 af
Pond 35P: DMH-8	Peak Elev=324.04' Inflow=4.42 cfs 0.356 af 24.0" Round Culvert n=0.013 L=21.0' S=0.0052 '/' Outflow=4.42 cfs 0.356 af
Pond 36P: DMH-16	Peak Elev=319.85' Inflow=1.92 cfs 0.141 af 12.0" Round Culvert n=0.013 L=21.0' S=0.0829 '/' Outflow=1.92 cfs 0.141 af
Pond 37P: DMH-7	Peak Elev=323.86' Inflow=5.01 cfs 0.407 af 24.0" Round Culvert n=0.013 L=78.1' S=0.0059 '/' Outflow=5.01 cfs 0.407 af
Pond 39P: CB-2 [EXISTING]	Peak Elev=311.42' Inflow=2.12 cfs 0.165 af 12.0" Round Culvert n=0.013 L=49.0' S=0.0010 '/' Outflow=2.12 cfs 0.165 af
Pond 40P: CB-1 [EXISTING]	Peak Elev=311.50' Inflow=0.86 cfs 0.067 af 12.0" Round Culvert n=0.013 L=17.0' S=0.0353 '/' Outflow=0.86 cfs 0.067 af
Pond 42P: DMH-1 [EXISTING]	Peak Elev=311.00' Inflow=2.12 cfs 0.165 af 12.0" Round Culvert n=0.013 L=92.0' S=0.0215 '/' Outflow=2.12 cfs 0.165 af
Pond 43P: SWALE & FIELD BASIN	Peak Elev=328.21' Storage=146 cf Inflow=4.01 cfs 0.327 af Discarded=0.02 cfs 0.017 af Primary=3.95 cfs 0.310 af Outflow=3.97 cfs 0.327 af
Link 4L: DP-A	Inflow=11.88 cfs 2.394 af Primary=11.88 cfs 2.394 af
Link 5L: DP-B	Inflow=0.03 cfs 0.016 af Primary=0.03 cfs 0.016 af

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Type III 24-hr 25-year Rainfall=5.40"

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Page 76

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Total Runoff Area = 23.990 ac   Runoff Volume = 5.042 af   Average Runoff Depth = 2.52"  
73.13% Pervious = 17.545 ac   26.87% Impervious = 6.446 ac

**Summary for Subcatchment 1S: POST 1.A**

Runoff = 1.27 cfs @ 12.09 hrs, Volume= 0.099 af, Depth= 4.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
4,679	98	Paved parking, HSG C
1,673	98	Paved parking, HSG B
2,343	98	Unconnected pavement, HSG C
343	98	Unconnected pavement, HSG B
395	61	>75% Grass cover, Good, HSG B
1,799	74	>75% Grass cover, Good, HSG C

11,232	93	Weighted Average
2,194		19.53% Pervious Area
9,038		80.47% Impervious Area
2,686		29.72% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 2S: POST 2.A**

Runoff = 2.37 cfs @ 12.09 hrs, Volume= 0.176 af, Depth= 3.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
757	98	Unconnected pavement, HSG B
8,275	98	Paved parking, HSG B
7,076	61	>75% Grass cover, Good, HSG B

23,362	87	Weighted Average
7,076		30.29% Pervious Area
16,286		69.71% Impervious Area
757		4.65% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 5S: POST 6.A

Runoff = 2.95 cfs @ 12.33 hrs, Volume= 0.345 af, Depth= 1.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
3,460	70	Woods, Good, HSG C
1,929	74	>75% Grass cover, Good, HSG C
11,762	55	Woods, Good, HSG B
43,358	61	>75% Grass cover, Good, HSG B
10,565	98	Roofs, HSG B
1,187	98	Unconnected pavement, HSG B
6,891	98	Roofs, HSG A
741	98	Unconnected pavement, HSG A
21,972	39	>75% Grass cover, Good, HSG A
101,865	63	Weighted Average
82,481		80.97% Pervious Area
19,384		19.03% Impervious Area
1,928		9.95% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3	50	0.0060	0.09		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 3.10"
11.8	859	0.0300	1.21		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
0.9	100	0.0700	1.85		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
0.1	83	0.0200	10.18	31.99	<b>Pipe Channel,</b> 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
22.1	1,092	Total			

### Summary for Subcatchment 6S: POST 7.A

Runoff = 2.32 cfs @ 12.09 hrs, Volume= 0.177 af, Depth= 4.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
9,604	98	Paved parking, HSG B
511	98	Unconnected pavement, HSG B
3,800	61	>75% Grass cover, Good, HSG B
21,169	91	Weighted Average
3,800		17.95% Pervious Area
17,369		82.05% Impervious Area
511		2.94% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 7S: POST 8.A**

Runoff = 1.04 cfs @ 12.10 hrs, Volume= 0.078 af, Depth= 1.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Adj	Description
120	98		Roofs, HSG B
1,579	96		Gravel surface, HSG B
1,003	98		Unconnected pavement, HSG B
18,403	61		>75% Grass cover, Good, HSG B
21,105	66	65	Weighted Average, UI Adjusted
19,982			94.68% Pervious Area
1,123			5.32% Impervious Area
1,003			89.31% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 8S: POST 3.A**

Runoff = 0.62 cfs @ 12.09 hrs, Volume= 0.051 af, Depth= 5.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
5,197	98	Paved parking, HSG B
5,197		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 9S: POST 4.A**

Runoff = 2.38 cfs @ 12.09 hrs, Volume= 0.175 af, Depth= 3.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

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Type III 24-hr 25-year Rainfall=5.40"

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Page 80

Area (sf)	CN	Description			
2,372	98	Roofs, HSG B			
150	98	Unconnected pavement, HSG B			
2,521	98	Paved parking, HSG B			
1,636	61	>75% Grass cover, Good, HSG B			
6,091	98	Roofs, HSG A			
649	98	Unconnected pavement, HSG A			
6,920	98	Paved parking, HSG A			
4,750	39	>75% Grass cover, Good, HSG A			
25,089	84	Weighted Average			
6,386		25.45% Pervious Area			
18,703		74.55% Impervious Area			
799		4.27% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 10S: POST 5.A**

Runoff = 1.55 cfs @ 12.09 hrs, Volume= 0.116 af, Depth= 4.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
338	98	Unconnected pavement, HSG B			
2,578	98	Paved parking, HSG B			
1,011	61	>75% Grass cover, Good, HSG B			
2,418	98	Roofs, HSG A			
494	98	Unconnected pavement, HSG A			
3,665	98	Paved parking, HSG A			
1,718	39	>75% Grass cover, Good, HSG A			
14,640	89	Weighted Average			
2,729		18.64% Pervious Area			
11,911		81.36% Impervious Area			
832		6.99% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 11S: POST 9.A**

Runoff = 1.41 cfs @ 12.09 hrs, Volume= 0.108 af, Depth= 4.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

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Type III 24-hr 25-year Rainfall=5.40"

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Page 81

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
7,363	98	Paved parking, HSG B			
701	98	Unconnected pavement, HSG B			
2,424	61	>75% Grass cover, Good, HSG B			
12,906	91	Weighted Average			
2,424		18.78% Pervious Area			
10,482		81.22% Impervious Area			
701		6.69% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 12S: POST 12.A**

Runoff = 4.01 cfs @ 12.12 hrs, Volume= 0.327 af, Depth= 1.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Adj	Description		
9,152	98		Roofs, HSG B		
2,471	98		Unconnected pavement, HSG B		
59,845	61		>75% Grass cover, Good, HSG B		
4,539	98		Roofs, HSG A		
1,734	98		Unconnected pavement, HSG A		
18,886	39		>75% Grass cover, Good, HSG A		
96,627	64	63	Weighted Average, UI Adjusted		
78,731			81.48% Pervious Area		
17,896			18.52% Impervious Area		
4,205			23.50% Unconnected		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry,

**Summary for Subcatchment 13S: POST 14.A**

Runoff = 0.83 cfs @ 12.09 hrs, Volume= 0.068 af, Depth= 5.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

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Page 82

Area (sf)	CN	Description
1,784	98	Roofs, HSG A
3,387	98	Paved parking, HSG A
99	39	>75% Grass cover, Good, HSG A
1,105	98	Paved parking, HSG B
634	98	Roofs, HSG B

7,009	97	Weighted Average
99		1.41% Pervious Area
6,910		98.59% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 14S: POST 15.A**

Runoff = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af, Depth= 3.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
1,135	98	Paved parking, HSG B
6,045	98	Roofs, HSG A
496	98	Unconnected pavement, HSG A
4,760	98	Paved parking, HSG A
3,806	39	>75% Grass cover, Good, HSG A

16,242	84	Weighted Average
3,806		23.43% Pervious Area
12,436		76.57% Impervious Area
496		3.99% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 15S: POST 13.A**

Runoff = 1.27 cfs @ 12.09 hrs, Volume= 0.092 af, Depth= 3.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

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Type III 24-hr 25-year Rainfall=5.40"

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Page 83

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
1,547	98	Unconnected pavement, HSG B			
3,405	98	Paved parking, HSG B			
7,938	61	>75% Grass cover, Good, HSG B			
15,308	79	Weighted Average			
7,938		51.86% Pervious Area			
7,370		48.14% Impervious Area			
1,547		20.99% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 16S: POST 16.A**

Runoff = 0.79 cfs @ 12.09 hrs, Volume= 0.065 af, Depth= 5.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
4,171	98	Paved parking, HSG B			
99	61	>75% Grass cover, Good, HSG B			
6,688	97	Weighted Average			
99		1.48% Pervious Area			
6,589		98.52% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 17S: POST 18.A**

Runoff = 0.93 cfs @ 12.09 hrs, Volume= 0.077 af, Depth= 5.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
5,396	98	Paved parking, HSG B			
119	61	>75% Grass cover, Good, HSG B			
7,933	97	Weighted Average			
119		1.50% Pervious Area			
7,814		98.50% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

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Page 84

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 18S: POST 17.A**

Runoff = 2.24 cfs @ 12.09 hrs, Volume= 0.168 af, Depth= 4.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
7,406	98	Paved parking, HSG B
1,509	98	Unconnected pavement, HSG B
4,989	61	>75% Grass cover, Good, HSG B
21,158	89	Weighted Average
4,989		23.58% Pervious Area
16,169		76.42% Impervious Area
1,509		9.33% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 19S: POST 24.A**

Runoff = 3.11 cfs @ 12.09 hrs, Volume= 0.226 af, Depth= 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
16,251	77	Woods, Good, HSG D
5,201	96	Gravel surface, HSG D
5,436	80	>75% Grass cover, Good, HSG D
6,221	30	Woods, Good, HSG A
3,810	39	>75% Grass cover, Good, HSG A
4,986	96	Gravel surface, HSG A
1,269	61	>75% Grass cover, Good, HSG B
2,270	96	Gravel surface, HSG B
45,444	73	Weighted Average
45,444		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

### Summary for Subcatchment 20S: POST 19.A

Runoff = 3.31 cfs @ 12.09 hrs, Volume= 0.248 af, Depth= 4.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
15,358	98	Paved parking, HSG B
1,073	98	Unconnected pavement, HSG B
8,284	61	>75% Grass cover, Good, HSG B
31,969	88	Weighted Average
8,284		25.91% Pervious Area
23,685		74.09% Impervious Area
1,073		4.53% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 21S: POST 20.A

Runoff = 3.17 cfs @ 12.10 hrs, Volume= 0.233 af, Depth= 2.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
5,596	98	Roofs, HSG B
1,630	98	Unconnected pavement, HSG B
1,749	96	Gravel surface, HSG B
40,318	61	>75% Grass cover, Good, HSG B
1,861	96	Gravel surface, HSG D
2,875	80	>75% Grass cover, Good, HSG D
54,029	69	Weighted Average
46,803		86.63% Pervious Area
7,226		13.37% Impervious Area
1,630		22.56% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 22S: POST 22.A**

Runoff = 4.96 cfs @ 12.23 hrs, Volume= 0.512 af, Depth= 1.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
6,845	98	Roofs, HSG A
819	98	Unconnected pavement, HSG A
5,708	30	Woods, Good, HSG A
30,256	39	>75% Grass cover, Good, HSG A
11,252	98	Roofs, HSG B
2,667	98	Unconnected pavement, HSG B
88,631	61	>75% Grass cover, Good, HSG B
14,680	55	Woods, Good, HSG B
2,251	96	Gravel surface, HSG B
831	96	Gravel surface, HSG D
1,334	80	>75% Grass cover, Good, HSG D
462	77	Woods, Good, HSG D
165,736	61	Weighted Average
144,153		86.98% Pervious Area
21,583		13.02% Impervious Area
3,486		16.15% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7	50	0.0200	0.15		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 3.10"
3.7	348	0.0500	1.57		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
5.8	486	0.0400	1.40		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
15.2	884	Total			

**Summary for Subcatchment 23S: POST 23.A**

Runoff = 9.16 cfs @ 12.24 hrs, Volume= 0.906 af, Depth= 2.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
24,614	55	Woods, Good, HSG B
1,206	96	Gravel surface, HSG D
1,599	80	>75% Grass cover, Good, HSG D
148,745	77	Woods, Good, HSG D
176,164	74	Weighted Average
176,164		100.00% Pervious Area

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Page 87

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.7					Direct Entry,

**Summary for Subcatchment 24S: POST 21.A**

Runoff = 2.95 cfs @ 12.10 hrs, Volume= 0.229 af, Depth= 1.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Adj	Description
1,194	98		Unconnected pavement, HSG B
248	96		Gravel surface, HSG B
72,586	61		>75% Grass cover, Good, HSG B
74,028	62	61	Weighted Average, UI Adjusted
72,834			98.39% Pervious Area
1,194			1.61% Impervious Area
1,194			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 25S: POST 1.B**

Runoff = 0.03 cfs @ 12.57 hrs, Volume= 0.016 af, Depth= 0.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
7,624	55	Woods, Good, HSG B
176	61	>75% Grass cover, Good, HSG B
24,732	30	Woods, Good, HSG A
6,261	39	>75% Grass cover, Good, HSG A
38,793	37	Weighted Average
38,793		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	50	0.0300	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.10"
0.4	56	0.2700	2.60		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
11.1	106	Total			

**Summary for Subcatchment 26S: POST 10.A**

Runoff = 0.55 cfs @ 12.09 hrs, Volume= 0.046 af, Depth= 5.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description			
4,620	98	Paved parking, HSG B			
4,620		100.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 27S: POST 11.A**

Runoff = 0.61 cfs @ 12.09 hrs, Volume= 0.051 af, Depth= 5.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description			
5,180	98	Paved parking, HSG B			
5,180		100.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 28S: BACK ROOFS OF UNITS 1-10**

Runoff = 1.62 cfs @ 12.09 hrs, Volume= 0.135 af, Depth= 5.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description			
13,690	98	Roofs, HSG B			
13,690		100.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 38S: POST 25.A

Runoff = 1.92 cfs @ 12.09 hrs, Volume= 0.141 af, Depth= 3.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
1,914	98	Roofs, HSG A
10,842	98	Unconnected pavement, HSG B
7,499	61	>75% Grass cover, Good, HSG B
20,255	84	Weighted Average
7,499		37.02% Pervious Area
12,756		62.98% Impervious Area
10,842		85.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 41S: POST 26.A

Runoff = 0.86 cfs @ 12.09 hrs, Volume= 0.067 af, Depth= 4.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25-year Rainfall=5.40"

Area (sf)	CN	Description
4,658	98	Paved parking, HSG C
1,501	98	Paved parking, HSG B
352	61	>75% Grass cover, Good, HSG B
1,070	74	>75% Grass cover, Good, HSG C
7,581	93	Weighted Average
1,422		18.76% Pervious Area
6,159		81.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Pond 5P: INFIL BASIN #1

Inflow Area = 8.395 ac, 42.27% Impervious, Inflow Depth = 2.73" for 25-year event

Inflow = 21.35 cfs @ 12.10 hrs, Volume= 1.909 af

Outflow = 8.47 cfs @ 12.47 hrs, Volume= 1.909 af, Atten= 60%, Lag= 22.3 min

Discarded = 0.49 cfs @ 12.47 hrs, Volume= 0.810 af

Primary = 7.98 cfs @ 12.47 hrs, Volume= 1.100 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**6092 - POST Toll Rev4 (current)**

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Type III 24-hr 25-year Rainfall=5.40"

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Page 90

Peak Elev= 315.27' @ 12.47 hrs Surf.Area= 8,712 sf Storage= 35,472 cf

Plug-Flow detention time= 377.2 min calculated for 1.908 af (100% of inflow)

Center-of-Mass det. time= 377.9 min ( 1,193.3 - 815.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	309.00'	42,092 cf	Custom Stage Data (Irregular) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
309.00	2,439	200.1	0	0	2,439
310.00	3,839	242.8	3,113	3,113	3,960
312.00	5,491	281.0	9,281	12,394	5,636
314.00	7,393	320.0	12,837	25,230	7,596
316.00	9,513	355.0	16,862	42,092	9,594

Device	Routing	Invert	Outlet Devices
#1	Primary	309.00'	<b>24.0" Round Culvert</b> L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 309.00' / 308.00' S= 0.0200 'l' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Discarded	309.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 306.73'
#3	Primary	315.60'	<b>20.0' long x 12.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.57 2.62 2.70 2.67 2.66 2.67 2.66 2.64
#4	Device 1	315.00'	<b>16.0' long Sharp-Crested Rectangular Weir</b> 0 End Contraction(s)
#5	Device 1	310.45'	<b>3.0" Vert. Orifice/Grate</b> C= 0.600

**Discarded OutFlow** Max=0.49 cfs @ 12.47 hrs HW=315.27' (Free Discharge)

↑ 2=Exfiltration (Controls 0.49 cfs)

**Primary OutFlow** Max=7.79 cfs @ 12.47 hrs HW=315.27' TW=0.00' (Dynamic Tailwater)

↑ 1=Culvert (Passes 7.79 cfs of 27.41 cfs potential flow)  
 ↑ 4=Sharp-Crested Rectangular Weir (Weir Controls 7.28 cfs @ 1.69 fps)  
 ↓ 5=Orifice/Grate (Orifice Controls 0.51 cfs @ 10.43 fps)  
 ↓ 3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

**Summary for Pond 6P: INFIL BASIN #2**

Inflow Area =	10.289 ac, 22.88% Impervious, Inflow Depth = 2.62"	for 25-year event
Inflow =	20.70 cfs @ 12.15 hrs, Volume=	2.248 af
Outflow =	2.05 cfs @ 14.13 hrs, Volume=	2.248 af, Atten= 90%, Lag= 118.6 min
Discarded =	1.21 cfs @ 14.13 hrs, Volume=	1.806 af
Primary =	0.84 cfs @ 14.13 hrs, Volume=	0.442 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 317.45' @ 14.13 hrs Surf.Area= 32,756 sf Storage= 54,251 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 478.8 min ( 1,309.5 - 830.6 )

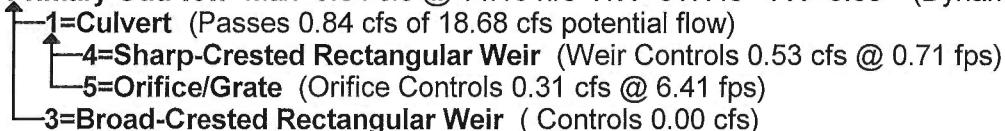
Volume	Invert	Avail.Storage	Storage Description
#1	314.00'	118,862 cf	Custom Stage Data (Irregular) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)
314.00	2,513	223.0	0
316.00	17,939	667.0	18,111
318.00	39,591	937.0	56,120
319.00	49,868	1,019.0	44,631
			118,862
			81,273

Device	Routing	Invert	Outlet Devices
#1	Primary	314.00'	<b>24.0" Round Culvert</b> L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.00' / 312.60' S= 0.0280 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Discarded	314.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 312.00'
#3	Primary	317.80'	<b>25.0' long x 20.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#4	Device 1	317.40'	<b>16.0' long Sharp-Crested Rectangular Weir</b> 0 End Contraction(s)
#5	Device 1	315.55'	<b>3.0" Vert. Orifice/Grate</b> C= 0.600

Discarded OutFlow Max=1.21 cfs @ 14.13 hrs HW=317.45' (Free Discharge)  
 ↗ 2=Exfiltration (Controls 1.21 cfs)

Primary OutFlow Max=0.84 cfs @ 14.13 hrs HW=317.45' TW=0.00' (Dynamic Tailwater)



### Summary for Pond 7P: DMH-2

Inflow Area = 4.772 ac, 49.56% Impervious, Inflow Depth = 2.95" for 25-year event  
 Inflow = 15.07 cfs @ 12.10 hrs, Volume= 1.175 af  
 Outflow = 15.07 cfs @ 12.10 hrs, Volume= 1.175 af, Atten= 0%, Lag= 0.0 min  
 Primary = 15.07 cfs @ 12.10 hrs, Volume= 1.175 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 316.49' @ 12.10 hrs

Flood Elev= 319.42'

Device	Routing	Invert	Outlet Devices
#1	Primary	313.90'	<b>24.0" Round Culvert</b> L= 60.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 313.90' / 312.70' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=15.04 cfs @ 12.10 hrs HW=316.49' TW=313.46' (Dynamic Tailwater)  
 ↪1=Culvert (Inlet Controls 15.04 cfs @ 4.79 fps)

### Summary for Pond 8P: CB-5

Inflow Area = 0.536 ac, 69.71% Impervious, Inflow Depth = 3.95" for 25-year event  
 Inflow = 2.37 cfs @ 12.09 hrs, Volume= 0.176 af  
 Outflow = 2.37 cfs @ 12.09 hrs, Volume= 0.176 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.37 cfs @ 12.09 hrs, Volume= 0.176 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 316.92' @ 12.14 hrs  
 Flood Elev= 319.26'

Device	Routing	Invert	Outlet Devices
#1	Primary	315.26'	<b>12.0" Round Culvert</b> L= 20.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 315.26' / 314.90' S= 0.0180 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.41 cfs @ 12.09 hrs HW=316.63' TW=316.41' (Dynamic Tailwater)  
 ↪1=Culvert (Inlet Controls 1.41 cfs @ 1.79 fps)

### Summary for Pond 9P: CB-6

Inflow Area = 0.416 ac, 86.61% Impervious, Inflow Depth = 4.60" for 25-year event  
 Inflow = 2.03 cfs @ 12.09 hrs, Volume= 0.159 af  
 Outflow = 2.03 cfs @ 12.09 hrs, Volume= 0.159 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.03 cfs @ 12.09 hrs, Volume= 0.159 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 316.79' @ 12.14 hrs  
 Flood Elev= 319.65'

Device	Routing	Invert	Outlet Devices
#1	Primary	315.65'	<b>12.0" Round Culvert</b> L= 20.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 315.65' / 314.90' S= 0.0375 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.26 cfs @ 12.09 hrs HW=316.58' TW=316.39' (Dynamic Tailwater)  
 ↪1=Culvert (Inlet Controls 1.26 cfs @ 1.65 fps)

### Summary for Pond 10P: CB-4

Inflow Area = 0.486 ac, 82.05% Impervious, Inflow Depth = 4.37" for 25-year event  
 Inflow = 2.32 cfs @ 12.09 hrs, Volume= 0.177 af  
 Outflow = 2.32 cfs @ 12.09 hrs, Volume= 0.177 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.32 cfs @ 12.09 hrs, Volume= 0.177 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 315.92' @ 12.09 hrs

Flood Elev= 318.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	314.82'	<b>12.0" Round Culvert</b> L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.82' / 314.40' S= 0.0168 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.26 cfs @ 12.09 hrs HW=315.89' TW=313.34' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.26 cfs @ 2.88 fps)

### Summary for Pond 11P: DMH-3

Inflow Area = 3.820 ac, 42.71% Impervious, Inflow Depth = 2.64" for 25-year event  
 Inflow = 10.71 cfs @ 12.10 hrs, Volume= 0.839 af  
 Outflow = 10.71 cfs @ 12.10 hrs, Volume= 0.839 af, Atten= 0%, Lag= 0.0 min  
 Primary = 10.71 cfs @ 12.10 hrs, Volume= 0.839 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 318.90' @ 12.10 hrs

Flood Elev= 321.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	317.10'	<b>24.0" Round Culvert</b> L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 317.10' / 314.90' S= 0.0349 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=10.61 cfs @ 12.10 hrs HW=318.88' TW=316.46' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 10.61 cfs @ 3.59 fps)

### Summary for Pond 12P: DMH-4

Inflow Area = 3.355 ac, 39.90% Impervious, Inflow Depth = 2.50" for 25-year event  
 Inflow = 8.81 cfs @ 12.11 hrs, Volume= 0.698 af  
 Outflow = 8.81 cfs @ 12.11 hrs, Volume= 0.698 af, Atten= 0%, Lag= 0.0 min  
 Primary = 8.81 cfs @ 12.11 hrs, Volume= 0.698 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 323.52' @ 12.11 hrs

Flood Elev= 326.96'

Device	Routing	Invert	Outlet Devices
#1	Primary	321.96'	<b>24.0" Round Culvert</b> L= 99.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 321.96' / 317.20' S= 0.0481 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=8.67 cfs @ 12.11 hrs HW=323.50' TW=318.87' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 8.67 cfs @ 3.34 fps)

### Summary for Pond 13P: DMH-6

Inflow Area =	0.912 ac, 77.06% Impervious, Inflow Depth = 3.83"	for 25-year event
Inflow =	3.93 cfs @ 12.09 hrs, Volume=	0.291 af
Outflow =	3.93 cfs @ 12.09 hrs, Volume=	0.291 af, Atten= 0%, Lag= 0.0 min
Primary =	3.93 cfs @ 12.09 hrs, Volume=	0.291 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 336.35' @ 12.09 hrs  
 Flood Elev= 340.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.25'	<b>18.0" Round Culvert</b> L= 124.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.25' / 328.11' S= 0.0576 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.84 cfs @ 12.09 hrs HW=336.34' TW=329.10' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 3.84 cfs @ 2.80 fps)

### Summary for Pond 14P: CB-8

Inflow Area =	0.336 ac, 81.36% Impervious, Inflow Depth = 4.16"	for 25-year event
Inflow =	1.55 cfs @ 12.09 hrs, Volume=	0.116 af
Outflow =	1.55 cfs @ 12.09 hrs, Volume=	0.116 af, Atten= 0%, Lag= 0.0 min
Primary =	1.55 cfs @ 12.09 hrs, Volume=	0.116 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 337.33' @ 12.09 hrs  
 Flood Elev= 340.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	336.55'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 336.55' / 335.75' S= 0.0571 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.51 cfs @ 12.09 hrs HW=337.31' TW=336.33' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 1.51 cfs @ 2.35 fps)

### Summary for Pond 15P: CB-7

Inflow Area =	0.576 ac, 74.55% Impervious, Inflow Depth = 3.64"	for 25-year event
Inflow =	2.38 cfs @ 12.09 hrs, Volume=	0.175 af
Outflow =	2.38 cfs @ 12.09 hrs, Volume=	0.175 af, Atten= 0%, Lag= 0.0 min
Primary =	2.38 cfs @ 12.09 hrs, Volume=	0.175 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 337.08' @ 12.09 hrs

Flood Elev= 339.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.95'	<b>12.0" Round Culvert</b> L= 10.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.95' / 335.75' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.33 cfs @ 12.09 hrs HW=337.06' TW=336.34' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.33 cfs @ 2.96 fps)

### **Summary for Pond 16P: DMH-5**

Inflow Area = 0.912 ac, 77.06% Impervious, Inflow Depth = 3.83" for 25-year event  
 Inflow = 3.93 cfs @ 12.09 hrs, Volume= 0.291 af  
 Outflow = 3.93 cfs @ 12.09 hrs, Volume= 0.291 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.93 cfs @ 12.09 hrs, Volume= 0.291 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 329.11' @ 12.09 hrs

Flood Elev= 332.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	328.01'	<b>18.0" Round Culvert</b> L= 91.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 328.01' / 322.56' S= 0.0599 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.84 cfs @ 12.09 hrs HW=329.10' TW=323.48' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 3.84 cfs @ 2.80 fps)

### **Summary for Pond 17P: DMH-15**

Inflow Area = 0.505 ac, 63.46% Impervious, Inflow Depth = 3.73" for 25-year event  
 Inflow = 2.06 cfs @ 12.09 hrs, Volume= 0.157 af  
 Outflow = 2.06 cfs @ 12.09 hrs, Volume= 0.157 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.06 cfs @ 12.09 hrs, Volume= 0.157 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 336.52' @ 12.09 hrs

Flood Elev= 340.37'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.55'	<b>12.0" Round Culvert</b> L= 69.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.55' / 334.86' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.01 cfs @ 12.09 hrs HW=336.50' TW=335.71' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 2.01 cfs @ 2.62 fps)

### Summary for Pond 18P: CB-17

Inflow Area =	0.154 ac, 98.52% Impervious, Inflow Depth = 5.05"	for 25-year event
Inflow =	0.79 cfs @ 12.09 hrs, Volume=	0.065 af
Outflow =	0.79 cfs @ 12.09 hrs, Volume=	0.065 af, Atten= 0%, Lag= 0.0 min
Primary =	0.79 cfs @ 12.09 hrs, Volume=	0.065 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 336.59' @ 12.13 hrs  
 Flood Elev= 339.81'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.81'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.81' / 335.65' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.31 cfs @ 12.09 hrs HW=336.51' TW=336.49' (Dynamic Tailwater)  
 ↗1=Culvert (Outlet Controls 0.31 cfs @ 0.73 fps)

### Summary for Pond 19P: CB-16

Inflow Area =	0.351 ac, 48.14% Impervious, Inflow Depth = 3.15"	for 25-year event
Inflow =	1.27 cfs @ 12.09 hrs, Volume=	0.092 af
Outflow =	1.27 cfs @ 12.09 hrs, Volume=	0.092 af, Atten= 0%, Lag= 0.0 min
Primary =	1.27 cfs @ 12.09 hrs, Volume=	0.092 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 336.68' @ 12.12 hrs  
 Flood Elev= 339.81'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.81'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.81' / 335.65' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.97 cfs @ 12.09 hrs HW=336.64' TW=336.50' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 0.97 cfs @ 1.39 fps)

### Summary for Pond 20P: DMH-14

Inflow Area =	0.505 ac, 63.46% Impervious, Inflow Depth = 3.73"	for 25-year event
Inflow =	2.06 cfs @ 12.09 hrs, Volume=	0.157 af
Outflow =	2.06 cfs @ 12.09 hrs, Volume=	0.157 af, Atten= 0%, Lag= 0.0 min
Primary =	2.06 cfs @ 12.09 hrs, Volume=	0.157 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 335.73' @ 12.09 hrs  
 Flood Elev= 341.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	334.76'	<b>12.0" Round Culvert</b> L= 88.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 334.76' / 333.88' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.01 cfs @ 12.09 hrs HW=335.71' TW=334.55' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.01 cfs @ 2.62 fps)

### Summary for Pond 21P: DMH-13

Inflow Area = 1.039 ac, 73.61% Impervious, Inflow Depth = 3.90" for 25-year event  
 Inflow = 4.42 cfs @ 12.09 hrs, Volume= 0.337 af  
 Outflow = 4.42 cfs @ 12.09 hrs, Volume= 0.337 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.42 cfs @ 12.09 hrs, Volume= 0.337 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 334.57' @ 12.09 hrs  
 Flood Elev= 342.06'

Device	Routing	Invert	Outlet Devices
#1	Primary	333.38'	<b>18.0" Round Culvert</b> L= 136.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 333.38' / 330.66' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=4.32 cfs @ 12.09 hrs HW=334.55' TW=331.73' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 4.32 cfs @ 2.91 fps)

### Summary for Pond 22P: CB-14

Inflow Area = 0.373 ac, 76.57% Impervious, Inflow Depth = 3.64" for 25-year event  
 Inflow = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af  
 Outflow = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.54 cfs @ 12.09 hrs, Volume= 0.113 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 338.98' @ 12.09 hrs  
 Flood Elev= 342.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	338.21'	<b>12.0" Round Culvert</b> L= 17.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 338.21' / 338.00' S= 0.0124 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.51 cfs @ 12.09 hrs HW=338.97' TW=334.55' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 1.51 cfs @ 2.35 fps)

### Summary for Pond 23P: CB-15

Inflow Area =	0.161 ac, 98.59% Impervious, Inflow Depth = 5.05"	for 25-year event
Inflow =	0.83 cfs @ 12.09 hrs, Volume=	0.068 af
Outflow =	0.83 cfs @ 12.09 hrs, Volume=	0.068 af, Atten= 0%, Lag= 0.0 min
Primary =	0.83 cfs @ 12.09 hrs, Volume=	0.068 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 338.84' @ 12.09 hrs  
 Flood Elev= 342.31'

Device	Routing	Invert	Outlet Devices
#1	Primary	338.31'	<b>12.0" Round Culvert</b> L= 28.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 338.31' / 338.00' S= 0.0111 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.80 cfs @ 12.09 hrs HW=338.83' TW=334.55' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 0.80 cfs @ 1.94 fps)

### Summary for Pond 24P: DMH-12

Inflow Area =	1.039 ac, 73.61% Impervious, Inflow Depth = 3.90"	for 25-year event
Inflow =	4.42 cfs @ 12.09 hrs, Volume=	0.337 af
Outflow =	4.42 cfs @ 12.09 hrs, Volume=	0.337 af, Atten= 0%, Lag= 0.0 min
Primary =	4.42 cfs @ 12.09 hrs, Volume=	0.337 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 331.75' @ 12.09 hrs  
 Flood Elev= 338.71'

Device	Routing	Invert	Outlet Devices
#1	Primary	330.56'	<b>18.0" Round Culvert</b> L= 99.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 330.56' / 327.84' S= 0.0275 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=4.32 cfs @ 12.09 hrs HW=331.73' TW=328.91' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 4.32 cfs @ 2.91 fps)

### Summary for Pond 25P: DMH-11

Inflow Area =	1.039 ac, 73.61% Impervious, Inflow Depth = 3.90"	for 25-year event
Inflow =	4.42 cfs @ 12.09 hrs, Volume=	0.337 af
Outflow =	4.42 cfs @ 12.09 hrs, Volume=	0.337 af, Atten= 0%, Lag= 0.0 min
Primary =	4.42 cfs @ 12.09 hrs, Volume=	0.337 af

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Type III 24-hr 25-year Rainfall=5.40"

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Page 99

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 328.93' @ 12.09 hrs

Flood Elev= 332.34'

Device	Routing	Invert	Outlet Devices
#1	Primary	327.74'	<b>18.0" Round Culvert</b> L= 76.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 327.74' / 322.02' S= 0.0753 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=4.32 cfs @ 12.09 hrs HW=328.91' TW=323.89' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 4.32 cfs @ 2.91 fps)

**Summary for Pond 26P: DMH-10**

Inflow Area = 1.707 ac, 77.06% Impervious, Inflow Depth = 4.09" for 25-year event  
 Inflow = 7.59 cfs @ 12.09 hrs, Volume= 0.582 af  
 Outflow = 7.59 cfs @ 12.09 hrs, Volume= 0.582 af, Atten= 0%, Lag= 0.0 min  
 Primary = 7.59 cfs @ 12.09 hrs, Volume= 0.582 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 323.94' @ 12.09 hrs

Flood Elev= 327.14'

Device	Routing	Invert	Outlet Devices
#1	Primary	321.92'	<b>18.0" Round Culvert</b> L= 174.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 321.92' / 316.81' S= 0.0294 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=7.41 cfs @ 12.09 hrs HW=323.89' TW=318.20' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 7.41 cfs @ 4.20 fps)

**Summary for Pond 27P: CB-13**

Inflow Area = 0.182 ac, 98.50% Impervious, Inflow Depth = 5.05" for 25-year event  
 Inflow = 0.93 cfs @ 12.09 hrs, Volume= 0.077 af  
 Outflow = 0.93 cfs @ 12.09 hrs, Volume= 0.077 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.93 cfs @ 12.09 hrs, Volume= 0.077 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 324.12' @ 12.13 hrs

Flood Elev= 327.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.51'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.51' / 323.23' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.70 cfs @ 12.09 hrs HW=324.07' TW=323.88' (Dynamic Tailwater)  
 ↑1=Culvert (Outlet Controls 0.70 cfs @ 2.25 fps)

### Summary for Pond 28P: CB-12

Inflow Area = 0.486 ac, 76.42% Impervious, Inflow Depth = 4.16" for 25-year event  
 Inflow = 2.24 cfs @ 12.09 hrs, Volume= 0.168 af  
 Outflow = 2.24 cfs @ 12.09 hrs, Volume= 0.168 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.24 cfs @ 12.09 hrs, Volume= 0.168 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 324.57' @ 12.09 hrs  
 Flood Elev= 327.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.51'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.51' / 323.23' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.18 cfs @ 12.09 hrs HW=324.54' TW=323.89' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 2.18 cfs @ 2.77 fps)

### Summary for Pond 29P: DMH-9

Inflow Area = 2.440 ac, 76.17% Impervious, Inflow Depth = 4.08" for 25-year event  
 Inflow = 10.91 cfs @ 12.09 hrs, Volume= 0.830 af  
 Outflow = 10.91 cfs @ 12.09 hrs, Volume= 0.830 af, Atten= 0%, Lag= 0.0 min  
 Primary = 10.91 cfs @ 12.09 hrs, Volume= 0.830 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 318.23' @ 12.09 hrs  
 Flood Elev= 322.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	316.31'	<b>24.0" Round Culvert</b> L= 62.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 316.31' / 316.00' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=10.64 cfs @ 12.09 hrs HW=318.20' TW=316.04' (Dynamic Tailwater)  
 ↑1=Culvert (Barrel Controls 10.64 cfs @ 4.48 fps)

### Summary for Pond 30P: CB-11

Inflow Area = 0.734 ac, 74.09% Impervious, Inflow Depth = 4.05" for 25-year event  
 Inflow = 3.31 cfs @ 12.09 hrs, Volume= 0.248 af  
 Outflow = 3.31 cfs @ 12.09 hrs, Volume= 0.248 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.31 cfs @ 12.09 hrs, Volume= 0.248 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 318.51' @ 12.12 hrs

Flood Elev= 321.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	317.30'	<b>18.0" Round Culvert</b> L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 317.30' / 316.81' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=2.53 cfs @ 12.09 hrs HW=318.44' TW=318.20' (Dynamic Tailwater)  
 ↑—1=Culvert (Outlet Controls 2.53 cfs @ 2.42 fps)

### **Summary for Pond 31P: PROP. CULVERT**

Inflow Area = 2.338 ac, 19.03% Impervious, Inflow Depth = 1.77" for 25-year event  
 Inflow = 2.95 cfs @ 12.33 hrs, Volume= 0.345 af  
 Outflow = 2.95 cfs @ 12.33 hrs, Volume= 0.345 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.95 cfs @ 12.33 hrs, Volume= 0.345 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 315.56' @ 12.47 hrs

Flood Elev= 318.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	314.00'	<b>15.0" Round Culvert</b> L= 75.8' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.00' / 313.00' S= 0.0132 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf

**Primary OutFlow** Max=2.08 cfs @ 12.33 hrs HW=315.27' TW=315.05' (Dynamic Tailwater)  
 ↑—1=Culvert (Outlet Controls 2.08 cfs @ 2.08 fps)

### **Summary for Pond 32P: CB-10**

Inflow Area = 0.106 ac, 100.00% Impervious, Inflow Depth = 5.16" for 25-year event  
 Inflow = 0.55 cfs @ 12.09 hrs, Volume= 0.046 af  
 Outflow = 0.55 cfs @ 12.09 hrs, Volume= 0.046 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.55 cfs @ 12.09 hrs, Volume= 0.046 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 324.04' @ 12.23 hrs

Flood Elev= 327.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.00'	<b>18.0" Round Culvert</b> L= 55.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.00' / 322.73' S= 0.0049 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=323.62' TW=323.77' (Dynamic Tailwater)  
 ↪1=Culvert ( Controls 0.00 cfs)

### Summary for Pond 33P: CB-9

Inflow Area = 0.119 ac, 100.00% Impervious, Inflow Depth = 5.16" for 25-year event  
 Inflow = 0.61 cfs @ 12.09 hrs, Volume= 0.051 af  
 Outflow = 0.61 cfs @ 12.09 hrs, Volume= 0.051 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.61 cfs @ 12.09 hrs, Volume= 0.051 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 323.87' @ 12.19 hrs  
 Flood Elev= 327.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.00'	<b>18.0" Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.00' / 322.52' S= 0.0686 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=323.53' TW=323.69' (Dynamic Tailwater)  
 ↪1=Culvert ( Controls 0.00 cfs)

### Summary for Pond 34P: CB-18

Inflow Area = 0.465 ac, 62.98% Impervious, Inflow Depth = 3.64" for 25-year event  
 Inflow = 1.92 cfs @ 12.09 hrs, Volume= 0.141 af  
 Outflow = 1.92 cfs @ 12.09 hrs, Volume= 0.141 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.92 cfs @ 12.09 hrs, Volume= 0.141 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 320.91' @ 12.09 hrs  
 Flood Elev= 325.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	320.00'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 320.00' / 319.04' S= 0.0600 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.88 cfs @ 12.09 hrs HW=320.89' TW=319.83' (Dynamic Tailwater)  
 ↪1=Culvert (Inlet Controls 1.88 cfs @ 2.54 fps)

### Summary for Pond 35P: DMH-8

Inflow Area = 2.324 ac, 22.24% Impervious, Inflow Depth = 1.84" for 25-year event  
 Inflow = 4.42 cfs @ 12.13 hrs, Volume= 0.356 af  
 Outflow = 4.42 cfs @ 12.13 hrs, Volume= 0.356 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.42 cfs @ 12.13 hrs, Volume= 0.356 af

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Page 103

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 324.04' @ 12.18 hrs

Flood Elev= 327.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	322.63'	<b>24.0" Round Culvert</b> L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 322.63' / 322.52' S= 0.0052 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.84 cfs @ 12.13 hrs HW=323.94' TW=323.82' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.84 cfs @ 1.31 fps)

**Summary for Pond 36P: DMH-16**

Inflow Area = 0.465 ac, 62.98% Impervious, Inflow Depth = 3.64" for 25-year event  
 Inflow = 1.92 cfs @ 12.09 hrs, Volume= 0.141 af  
 Outflow = 1.92 cfs @ 12.09 hrs, Volume= 0.141 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.92 cfs @ 12.09 hrs, Volume= 0.141 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 319.85' @ 12.09 hrs  
 Flood Elev= 326.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	318.94'	<b>12.0" Round Culvert</b> L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 318.94' / 317.20' S= 0.0829 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.88 cfs @ 12.09 hrs HW=319.83' TW=318.85' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 1.88 cfs @ 2.54 fps)

**Summary for Pond 37P: DMH-7**

Inflow Area = 2.443 ac, 26.02% Impervious, Inflow Depth = 2.00" for 25-year event  
 Inflow = 5.01 cfs @ 12.12 hrs, Volume= 0.407 af  
 Outflow = 5.01 cfs @ 12.12 hrs, Volume= 0.407 af, Atten= 0%, Lag= 0.0 min  
 Primary = 5.01 cfs @ 12.12 hrs, Volume= 0.407 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 323.86' @ 12.15 hrs  
 Flood Elev= 327.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	322.42'	<b>24.0" Round Culvert</b> L= 78.1' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 322.42' / 321.96' S= 0.0059 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.55 cfs @ 12.12 hrs HW=323.81' TW=323.47' (Dynamic Tailwater)  
 ↗1=Culvert (Outlet Controls 4.55 cfs @ 2.76 fps)

### Summary for Pond 39P: CB-2 [EXISTING]

Inflow Area = 0.432 ac, 80.78% Impervious, Inflow Depth = 4.59" for 25-year event  
 Inflow = 2.12 cfs @ 12.09 hrs, Volume= 0.165 af  
 Outflow = 2.12 cfs @ 12.09 hrs, Volume= 0.165 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.12 cfs @ 12.09 hrs, Volume= 0.165 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 311.42' @ 12.11 hrs  
 Flood Elev= 314.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.05'	<b>12.0" Round Culvert</b> L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.05' / 310.00' S= 0.0010 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.88 cfs @ 12.09 hrs HW=311.37' TW=310.98' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 1.88 cfs @ 2.39 fps)

### Summary for Pond 40P: CB-1 [EXISTING]

Inflow Area = 0.174 ac, 81.24% Impervious, Inflow Depth = 4.59" for 25-year event  
 Inflow = 0.86 cfs @ 12.09 hrs, Volume= 0.067 af  
 Outflow = 0.86 cfs @ 12.09 hrs, Volume= 0.067 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.86 cfs @ 12.09 hrs, Volume= 0.067 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 311.50' @ 12.15 hrs  
 Flood Elev= 314.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.75'	<b>12.0" Round Culvert</b> L= 17.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.75' / 310.15' S= 0.0353 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.07 cfs @ 12.09 hrs HW=311.37' TW=311.37' (Dynamic Tailwater)  
 ↗1=Culvert (Outlet Controls 0.07 cfs @ 0.19 fps)

### Summary for Pond 42P: DMH-1 [EXISTING]

Inflow Area = 0.432 ac, 80.78% Impervious, Inflow Depth = 4.59" for 25-year event  
 Inflow = 2.12 cfs @ 12.09 hrs, Volume= 0.165 af  
 Outflow = 2.12 cfs @ 12.09 hrs, Volume= 0.165 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.12 cfs @ 12.09 hrs, Volume= 0.165 af

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Page 105

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 311.00' @ 12.09 hrs

Flood Elev= 315.33'

DeviceRoutingInvertOutlet Devices

#1	Primary	310.00'	<b>12.0" Round Culvert</b> L= 92.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.00' / 308.02' S= 0.0215 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
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**Primary OutFlow** Max=2.08 cfs @ 12.09 hrs HW=310.98' TW=0.00' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 2.08 cfs @ 2.66 fps)

**Summary for Pond 43P: SWALE & FIELD BASIN**

Inflow Area =	2.218 ac, 18.52% Impervious, Inflow Depth = 1.77"	for 25-year event
Inflow =	4.01 cfs @ 12.12 hrs, Volume=	0.327 af
Outflow =	3.97 cfs @ 12.14 hrs, Volume=	0.327 af, Atten= 1%, Lag= 0.8 min
Discarded =	0.02 cfs @ 12.14 hrs, Volume=	0.017 af
Primary =	3.95 cfs @ 12.14 hrs, Volume=	0.310 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 328.21' @ 12.14 hrs Surf.Area= 800 sf Storage= 146 cf

Plug-Flow detention time= 1.1 min calculated for 0.327 af (100% of inflow)  
 Center-of-Mass det. time= 1.1 min ( 864.4 - 863.3 )

VolumeInvertAvail.StorageStorage Description

#1	328.00'	1,150 cf	<b>Custom Stage Data (Irregular) Listed below (Recalc)</b>
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Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
328.00	600	119.4	0	0	600
329.00	1,808	444.3	1,150	1,150	15,177

DeviceRoutingInvertOutlet Devices

#1	Discarded	328.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 320.50'
#2	Primary	324.00'	<b>18.0" Round Culvert</b> L= 51.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 324.00' / 322.73' S= 0.0249 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#3	Device 2	328.00'	<b>48.0" Horiz. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.02 cfs @ 12.14 hrs HW=328.21' (Free Discharge)  
 ↑ 1=Exfiltration ( Controls 0.02 cfs)

**Primary OutFlow** Max=3.87 cfs @ 12.14 hrs HW=328.21' TW=323.96' (Dynamic Tailwater)  
 ↑ 2=Culvert (Passes 3.87 cfs of 12.49 cfs potential flow)  
 ↑ 3=Orifice/Grate (Weir Controls 3.87 cfs @ 1.49 fps)

**Summary for Link 4L: DP-A**

Inflow Area = 23.100 ac, 27.90% Impervious, Inflow Depth = 1.24" for 25-year event  
Inflow = 11.88 cfs @ 12.10 hrs, Volume= 2.394 af  
Primary = 11.88 cfs @ 12.10 hrs, Volume= 2.394 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Summary for Link 5L: DP-B**

Inflow Area = 0.891 ac, 0.00% Impervious, Inflow Depth = 0.21" for 25-year event  
Inflow = 0.03 cfs @ 12.57 hrs, Volume= 0.016 af  
Primary = 0.03 cfs @ 12.57 hrs, Volume= 0.016 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment 1S: POST 1.A</b>	Runoff Area=11,232 sf 80.47% Impervious Runoff Depth=6.17" Tc=6.0 min CN=93 Runoff=1.68 cfs 0.133 af
<b>Subcatchment 2S: POST 2.A</b>	Runoff Area=23,362 sf 69.71% Impervious Runoff Depth=5.48" Tc=6.0 min CN=87 Runoff=3.24 cfs 0.245 af
<b>Subcatchment 5S: POST 6.A</b>	Runoff Area=101,865 sf 19.03% Impervious Runoff Depth=2.90" Flow Length=1,092' Tc=22.1 min CN=63 Runoff=5.03 cfs 0.565 af
<b>Subcatchment 6S: POST 7.A</b>	Runoff Area=21,169 sf 82.05% Impervious Runoff Depth=5.94" Tc=6.0 min CN=91 Runoff=3.09 cfs 0.241 af
<b>Subcatchment 7S: POST 8.A</b>	Runoff Area=21,105 sf 5.32% Impervious Runoff Depth=3.10" Tc=6.0 min UI Adjusted CN=65 Runoff=1.71 cfs 0.125 af
<b>Subcatchment 8S: POST 3.A</b>	Runoff Area=5,197 sf 100.00% Impervious Runoff Depth=6.76" Tc=6.0 min CN=98 Runoff=0.80 cfs 0.067 af
<b>Subcatchment 9S: POST 4.A</b>	Runoff Area=25,089 sf 74.55% Impervious Runoff Depth=5.14" Tc=6.0 min CN=84 Runoff=3.32 cfs 0.247 af
<b>Subcatchment 10S: POST 5.A</b>	Runoff Area=14,640 sf 81.36% Impervious Runoff Depth=5.71" Tc=6.0 min CN=89 Runoff=2.09 cfs 0.160 af
<b>Subcatchment 11S: POST 9.A</b>	Runoff Area=12,906 sf 81.22% Impervious Runoff Depth=5.94" Tc=6.0 min CN=91 Runoff=1.89 cfs 0.147 af
<b>Subcatchment 12S: POST 12.A</b>	Runoff Area=96,627 sf 18.52% Impervious Runoff Depth=2.90" Tc=8.0 min UI Adjusted CN=63 Runoff=6.83 cfs 0.536 af
<b>Subcatchment 13S: POST 14.A</b>	Runoff Area=7,009 sf 98.59% Impervious Runoff Depth=6.64" Tc=6.0 min CN=97 Runoff=1.07 cfs 0.089 af
<b>Subcatchment 14S: POST 15.A</b>	Runoff Area=16,242 sf 76.57% Impervious Runoff Depth=5.14" Tc=6.0 min CN=84 Runoff=2.15 cfs 0.160 af
<b>Subcatchment 15S: POST 13.A</b>	Runoff Area=15,308 sf 48.14% Impervious Runoff Depth=4.58" Tc=6.0 min CN=79 Runoff=1.84 cfs 0.134 af
<b>Subcatchment 16S: POST 16.A</b>	Runoff Area=6,688 sf 98.52% Impervious Runoff Depth=6.64" Tc=6.0 min CN=97 Runoff=1.03 cfs 0.085 af
<b>Subcatchment 17S: POST 18.A</b>	Runoff Area=7,933 sf 98.50% Impervious Runoff Depth=6.64" Tc=6.0 min CN=97 Runoff=1.22 cfs 0.101 af
<b>Subcatchment 18S: POST 17.A</b>	Runoff Area=21,158 sf 76.42% Impervious Runoff Depth=5.71" Tc=6.0 min CN=89 Runoff=3.02 cfs 0.231 af

**Subcatchment 19S: POST 24.A**

Runoff Area=45,444 sf 0.00% Impervious Runoff Depth=3.94"  
Tc=6.0 min CN=73 Runoff=4.72 cfs 0.342 af

**Subcatchment 20S: POST 19.A**

Runoff Area=31,969 sf 74.09% Impervious Runoff Depth=5.59"  
Tc=6.0 min CN=88 Runoff=4.50 cfs 0.342 af

**Subcatchment 21S: POST 20.A**

Runoff Area=54,029 sf 13.37% Impervious Runoff Depth=3.51"  
Tc=6.0 min CN=69 Runoff=5.00 cfs 0.363 af

**Subcatchment 22S: POST 22.A**

Runoff Area=165,736 sf 13.02% Impervious Runoff Depth=2.70"  
Flow Length=884' Tc=15.2 min CN=61 Runoff=8.78 cfs 0.857 af

**Subcatchment 23S: POST 23.A**

Runoff Area=176,164 sf 0.00% Impervious Runoff Depth=4.04"  
Tc=16.7 min CN=74 Runoff=13.82 cfs 1.362 af

**Subcatchment 24S: POST 21.A**

Runoff Area=74,028 sf 1.61% Impervious Runoff Depth=2.70"  
Tc=6.0 min UI Adjusted CN=61 Runoff=5.16 cfs 0.383 af

**Subcatchment 25S: POST 1.B**

Runoff Area=38,793 sf 0.00% Impervious Runoff Depth=0.63"  
Flow Length=106' Tc=11.1 min CN=37 Runoff=0.24 cfs 0.047 af

**Subcatchment 26S: POST 10.A**

Runoff Area=4,620 sf 100.00% Impervious Runoff Depth=6.76"  
Tc=6.0 min CN=98 Runoff=0.71 cfs 0.060 af

**Subcatchment 27S: POST 11.A**

Runoff Area=5,180 sf 100.00% Impervious Runoff Depth=6.76"  
Tc=6.0 min CN=98 Runoff=0.80 cfs 0.067 af

**Subcatchment 28S: BACK ROOFS OF**

Runoff Area=13,690 sf 100.00% Impervious Runoff Depth=6.76"  
Tc=6.0 min CN=98 Runoff=2.11 cfs 0.177 af

**Subcatchment 38S: POST 25.A**

Runoff Area=20,255 sf 62.98% Impervious Runoff Depth=5.14"  
Tc=6.0 min CN=84 Runoff=2.68 cfs 0.199 af

**Subcatchment 41S: POST 26.A**

Runoff Area=7,581 sf 81.24% Impervious Runoff Depth=6.17"  
Tc=6.0 min CN=93 Runoff=1.13 cfs 0.090 af

**Pond 5P: INFIL BASIN #1**

Peak Elev=315.57' Storage=38,066 cf Inflow=31.27 cfs 2.817 af

Discarded=0.52 cfs 0.891 af Primary=22.76 cfs 1.927 af Outflow=23.27 cfs 2.818 af

**Pond 6P: INFIL BASIN #2**

Peak Elev=317.74' Storage=64,304 cf Inflow=31.44 cfs 3.361 af

Discarded=1.36 cfs 2.013 af Primary=10.63 cfs 1.348 af Outflow=11.99 cfs 3.361 af

**Pond 7P: DMH-2**

Peak Elev=318.26' Inflow=21.88 cfs 1.709 af  
24.0" Round Culvert n=0.013 L=60.0' S=0.0200 '/' Outflow=21.88 cfs 1.709 af

**Pond 8P: CB-5**

Peak Elev=319.03' Inflow=3.24 cfs 0.245 af  
12.0" Round Culvert n=0.013 L=20.0' S=0.0180 '/' Outflow=3.24 cfs 0.245 af

**Pond 9P: CB-6**

Peak Elev=318.77' Inflow=2.69 cfs 0.214 af  
12.0" Round Culvert n=0.013 L=20.0' S=0.0375 '/' Outflow=2.69 cfs 0.214 af

**Pond 10P: CB-4**

Peak Elev=316.39' Inflow=3.09 cfs 0.241 af  
12.0" Round Culvert n=0.013 L=25.0' S=0.0168 '/' Outflow=3.09 cfs 0.241 af

**Pond 11P: DMH-3**

Peak Elev=319.91' Inflow=16.01 cfs 1.251 af  
24.0" Round Culvert n=0.013 L=63.0' S=0.0349 '/' Outflow=16.01 cfs 1.251 af

**Pond 12P: DMH-4**

Peak Elev=324.21' Inflow=13.37 cfs 1.051 af  
24.0" Round Culvert n=0.013 L=99.0' S=0.0481 '/' Outflow=13.37 cfs 1.051 af

**Pond 13P: DMH-6**

Peak Elev=336.64' Inflow=5.40 cfs 0.407 af  
18.0" Round Culvert n=0.013 L=124.0' S=0.0576 '/' Outflow=5.40 cfs 0.407 af

**Pond 14P: CB-8**

Peak Elev=337.53' Inflow=2.09 cfs 0.160 af  
12.0" Round Culvert n=0.013 L=14.0' S=0.0571 '/' Outflow=2.09 cfs 0.160 af

**Pond 15P: CB-7**

Peak Elev=337.76' Inflow=3.32 cfs 0.247 af  
12.0" Round Culvert n=0.013 L=10.0' S=0.0200 '/' Outflow=3.32 cfs 0.247 af

**Pond 16P: DMH-5**

Peak Elev=329.40' Inflow=5.40 cfs 0.407 af  
18.0" Round Culvert n=0.013 L=91.0' S=0.0599 '/' Outflow=5.40 cfs 0.407 af

**Pond 17P: DMH-15**

Peak Elev=336.96' Inflow=2.86 cfs 0.219 af  
12.0" Round Culvert n=0.013 L=69.0' S=0.0100 '/' Outflow=2.86 cfs 0.219 af

**Pond 18P: CB-17**

Peak Elev=337.04' Inflow=1.03 cfs 0.085 af  
12.0" Round Culvert n=0.013 L=16.0' S=0.0100 '/' Outflow=1.03 cfs 0.085 af

**Pond 19P: CB-16**

Peak Elev=337.24' Inflow=1.84 cfs 0.134 af  
12.0" Round Culvert n=0.013 L=16.0' S=0.0100 '/' Outflow=1.84 cfs 0.134 af

**Pond 20P: DMH-14**

Peak Elev=336.18' Inflow=2.86 cfs 0.219 af  
12.0" Round Culvert n=0.013 L=88.0' S=0.0100 '/' Outflow=2.86 cfs 0.219 af

**Pond 21P: DMH-13**

Peak Elev=334.95' Inflow=6.08 cfs 0.468 af  
18.0" Round Culvert n=0.013 L=136.0' S=0.0200 '/' Outflow=6.08 cfs 0.468 af

**Pond 22P: CB-14**

Peak Elev=339.22' Inflow=2.15 cfs 0.160 af  
12.0" Round Culvert n=0.013 L=17.0' S=0.0124 '/' Outflow=2.15 cfs 0.160 af

**Pond 23P: CB-15**

Peak Elev=338.93' Inflow=1.07 cfs 0.089 af  
12.0" Round Culvert n=0.013 L=28.0' S=0.0111 '/' Outflow=1.07 cfs 0.089 af

**Pond 24P: DMH-12**

Peak Elev=332.13' Inflow=6.08 cfs 0.468 af  
18.0" Round Culvert n=0.013 L=99.0' S=0.0275 '/' Outflow=6.08 cfs 0.468 af

**Pond 25P: DMH-11**

Peak Elev=329.31' Inflow=6.08 cfs 0.468 af  
18.0" Round Culvert n=0.013 L=76.0' S=0.0753 '/' Outflow=6.08 cfs 0.468 af

**Pond 26P: DMH-10**

Peak Elev=325.02' Inflow=10.32 cfs 0.800 af  
18.0" Round Culvert n=0.013 L=174.0' S=0.0294 '/' Outflow=10.32 cfs 0.800 af

**Pond 27P: CB-13**

Peak Elev=325.13' Inflow=1.22 cfs 0.101 af  
12.0" Round Culvert n=0.013 L=14.0' S=0.0200 '/' Outflow=1.22 cfs 0.101 af

**Pond 28P: CB-12**

Peak Elev=325.75' Inflow=3.02 cfs 0.231 af  
12.0" Round Culvert n=0.013 L=14.0' S=0.0200 '/' Outflow=3.02 cfs 0.231 af

**Pond 29P: DMH-9**

Peak Elev=318.84' Inflow=14.81 cfs 1.142 af  
24.0" Round Culvert n=0.013 L=62.0' S=0.0050 '/' Outflow=14.81 cfs 1.142 af

**Pond 30P: CB-11**

Peak Elev=319.16' Inflow=4.50 cfs 0.342 af  
18.0" Round Culvert n=0.013 L=49.0' S=0.0100 '/' Outflow=4.50 cfs 0.342 af

**Pond 31P: PROP. CULVERT**

Peak Elev=316.69' Inflow=5.03 cfs 0.565 af  
15.0" Round Culvert n=0.013 L=75.8' S=0.0132 '/' Outflow=5.03 cfs 0.565 af

**Pond 32P: CB-10**

Peak Elev=324.89' Inflow=0.71 cfs 0.060 af  
18.0" Round Culvert n=0.013 L=55.0' S=0.0049 '/' Outflow=0.71 cfs 0.060 af

**Pond 33P: CB-9**

Peak Elev=324.64' Inflow=0.80 cfs 0.067 af  
18.0" Round Culvert n=0.013 L=7.0' S=0.0686 '/' Outflow=0.80 cfs 0.067 af

**Pond 34P: CB-18**

Peak Elev=321.30' Inflow=2.68 cfs 0.199 af  
12.0" Round Culvert n=0.013 L=16.0' S=0.0600 '/' Outflow=2.68 cfs 0.199 af

**Pond 35P: DMH-8**

Peak Elev=324.89' Inflow=7.41 cfs 0.578 af  
24.0" Round Culvert n=0.013 L=21.0' S=0.0052 '/' Outflow=7.41 cfs 0.578 af

**Pond 36P: DMH-16**

Peak Elev=320.42' Inflow=2.68 cfs 0.199 af  
12.0" Round Culvert n=0.013 L=21.0' S=0.0829 '/' Outflow=2.68 cfs 0.199 af

**Pond 37P: DMH-7**

Peak Elev=324.64' Inflow=8.15 cfs 0.645 af  
24.0" Round Culvert n=0.013 L=78.1' S=0.0059 '/' Outflow=8.15 cfs 0.645 af

**Pond 39P: CB-2 [EXISTING]**

Peak Elev=312.14' Inflow=2.81 cfs 0.222 af  
12.0" Round Culvert n=0.013 L=49.0' S=0.0010 '/' Outflow=2.81 cfs 0.222 af

**Pond 40P: CB-1 [EXISTING]**

Peak Elev=312.22' Inflow=1.13 cfs 0.090 af  
12.0" Round Culvert n=0.013 L=17.0' S=0.0353 '/' Outflow=1.13 cfs 0.090 af

**Pond 42P: DMH-1 [EXISTING]**

Peak Elev=311.38' Inflow=2.81 cfs 0.222 af  
12.0" Round Culvert n=0.013 L=92.0' S=0.0215 '/' Outflow=2.81 cfs 0.222 af

**Pond 43P: SWALE & FIELD BASIN**

Peak Elev=328.30' Storage=222 cf Inflow=6.83 cfs 0.536 af  
Discarded=0.02 cfs 0.018 af Primary=6.72 cfs 0.518 af Outflow=6.75 cfs 0.536 af

**Link 4L: DP-A**

Inflow=33.53 cfs 4.585 af  
Primary=33.53 cfs 4.585 af

**Link 5L: DP-B**

Inflow=0.24 cfs 0.047 af  
Primary=0.24 cfs 0.047 af

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*Type III 24-hr 100-year Rainfall=7.00"*

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Page 111

**Total Runoff Area = 23.990 ac Runoff Volume = 7.553 af Average Runoff Depth = 3.78"**  
**73.13% Pervious = 17.545 ac 26.87% Impervious = 6.446 ac**

**Summary for Subcatchment 1S: POST 1.A**

Runoff = 1.68 cfs @ 12.09 hrs, Volume= 0.133 af, Depth= 6.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description			
4,679	98	Paved parking, HSG C			
1,673	98	Paved parking, HSG B			
2,343	98	Unconnected pavement, HSG C			
343	98	Unconnected pavement, HSG B			
395	61	>75% Grass cover, Good, HSG B			
1,799	74	>75% Grass cover, Good, HSG C			
11,232	93	Weighted Average			
2,194		19.53% Pervious Area			
9,038		80.47% Impervious Area			
2,686		29.72% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 2S: POST 2.A**

Runoff = 3.24 cfs @ 12.09 hrs, Volume= 0.245 af, Depth= 5.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description			
7,254	98	Roofs, HSG B			
757	98	Unconnected pavement, HSG B			
8,275	98	Paved parking, HSG B			
7,076	61	>75% Grass cover, Good, HSG B			
23,362	87	Weighted Average			
7,076		30.29% Pervious Area			
16,286		69.71% Impervious Area			
757		4.65% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 5S: POST 6.A

Runoff = 5.03 cfs @ 12.32 hrs, Volume= 0.565 af, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
3,460	70	Woods, Good, HSG C
1,929	74	>75% Grass cover, Good, HSG C
11,762	55	Woods, Good, HSG B
43,358	61	>75% Grass cover, Good, HSG B
10,565	98	Roofs, HSG B
1,187	98	Unconnected pavement, HSG B
6,891	98	Roofs, HSG A
741	98	Unconnected pavement, HSG A
21,972	39	>75% Grass cover, Good, HSG A
101,865	63	Weighted Average
82,481		80.97% Pervious Area
19,384		19.03% Impervious Area
1,928		9.95% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3	50	0.0060	0.09		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 3.10"
11.8	859	0.0300	1.21		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
0.9	100	0.0700	1.85		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
0.1	83	0.0200	10.18	31.99	<b>Pipe Channel,</b> 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
22.1	1,092	Total			

### Summary for Subcatchment 6S: POST 7.A

Runoff = 3.09 cfs @ 12.09 hrs, Volume= 0.241 af, Depth= 5.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
9,604	98	Paved parking, HSG B
511	98	Unconnected pavement, HSG B
3,800	61	>75% Grass cover, Good, HSG B
21,169	91	Weighted Average
3,800		17.95% Pervious Area
17,369		82.05% Impervious Area
511		2.94% Unconnected

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Type III 24-hr 100-year Rainfall=7.00"

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Page 114

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 7S: POST 8.A**

Runoff = 1.71 cfs @ 12.10 hrs, Volume= 0.125 af, Depth= 3.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Adj	Description
120	98		Roofs, HSG B
1,579	96		Gravel surface, HSG B
1,003	98		Unconnected pavement, HSG B
18,403	61		>75% Grass cover, Good, HSG B
21,105	66	65	Weighted Average, UI Adjusted
19,982			94.68% Pervious Area
1,123			5.32% Impervious Area
1,003			89.31% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 8S: POST 3.A**

Runoff = 0.80 cfs @ 12.09 hrs, Volume= 0.067 af, Depth= 6.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
5,197	98	Paved parking, HSG B
5,197		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 9S: POST 4.A**

Runoff = 3.32 cfs @ 12.09 hrs, Volume= 0.247 af, Depth= 5.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

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Type III 24-hr 100-year Rainfall=7.00"

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Page 115

Area (sf)	CN	Description
2,372	98	Roofs, HSG B
150	98	Unconnected pavement, HSG B
2,521	98	Paved parking, HSG B
1,636	61	>75% Grass cover, Good, HSG B
6,091	98	Roofs, HSG A
649	98	Unconnected pavement, HSG A
6,920	98	Paved parking, HSG A
4,750	39	>75% Grass cover, Good, HSG A

25,089	84	Weighted Average
6,386		25.45% Pervious Area
18,703		74.55% Impervious Area
799		4.27% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 10S: POST 5.A**

Runoff = 2.09 cfs @ 12.09 hrs, Volume= 0.160 af, Depth= 5.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
2,418	98	Roofs, HSG B
338	98	Unconnected pavement, HSG B
2,578	98	Paved parking, HSG B
1,011	61	>75% Grass cover, Good, HSG B
2,418	98	Roofs, HSG A
494	98	Unconnected pavement, HSG A
3,665	98	Paved parking, HSG A
1,718	39	>75% Grass cover, Good, HSG A

14,640	89	Weighted Average
2,729		18.64% Pervious Area
11,911		81.36% Impervious Area
832		6.99% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 11S: POST 9.A**

Runoff = 1.89 cfs @ 12.09 hrs, Volume= 0.147 af, Depth= 5.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

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Type III 24-hr 100-year Rainfall=7.00"

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Page 116

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
7,363	98	Paved parking, HSG B			
701	98	Unconnected pavement, HSG B			
2,424	61	>75% Grass cover, Good, HSG B			
12,906	91	Weighted Average			
2,424		18.78% Pervious Area			
10,482		81.22% Impervious Area			
701		6.69% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 12S: POST 12.A**

Runoff = 6.83 cfs @ 12.12 hrs, Volume= 0.536 af, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Adj	Description		
9,152	98		Roofs, HSG B		
2,471	98		Unconnected pavement, HSG B		
59,845	61		>75% Grass cover, Good, HSG B		
4,539	98		Roofs, HSG A		
1,734	98		Unconnected pavement, HSG A		
18,886	39		>75% Grass cover, Good, HSG A		
96,627	64	63	Weighted Average, UI Adjusted		
78,731			81.48% Pervious Area		
17,896			18.52% Impervious Area		
4,205			23.50% Unconnected		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry,

**Summary for Subcatchment 13S: POST 14.A**

Runoff = 1.07 cfs @ 12.09 hrs, Volume= 0.089 af, Depth= 6.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

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Page 117

Area (sf)	CN	Description
1,784	98	Roofs, HSG A
3,387	98	Paved parking, HSG A
99	39	>75% Grass cover, Good, HSG A
1,105	98	Paved parking, HSG B
634	98	Roofs, HSG B
7,009	97	Weighted Average
99		1.41% Pervious Area
6,910		98.59% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 14S: POST 15.A**

Runoff = 2.15 cfs @ 12.09 hrs, Volume= 0.160 af, Depth= 5.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
1,135	98	Paved parking, HSG B
6,045	98	Roofs, HSG A
496	98	Unconnected pavement, HSG A
4,760	98	Paved parking, HSG A
3,806	39	>75% Grass cover, Good, HSG A
16,242	84	Weighted Average
3,806		23.43% Pervious Area
12,436		76.57% Impervious Area
496		3.99% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 15S: POST 13.A**

Runoff = 1.84 cfs @ 12.09 hrs, Volume= 0.134 af, Depth= 4.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
1,547	98	Unconnected pavement, HSG B			
3,405	98	Paved parking, HSG B			
7,938	61	>75% Grass cover, Good, HSG B			
15,308	79	Weighted Average			
7,938		51.86% Pervious Area			
7,370		48.14% Impervious Area			
1,547		20.99% Unconnected			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 16S: POST 16.A**

Runoff = 1.03 cfs @ 12.09 hrs, Volume= 0.085 af, Depth= 6.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
4,171	98	Paved parking, HSG B			
99	61	>75% Grass cover, Good, HSG B			
6,688	97	Weighted Average			
99		1.48% Pervious Area			
6,589		98.52% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 17S: POST 18.A**

Runoff = 1.22 cfs @ 12.09 hrs, Volume= 0.101 af, Depth= 6.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description			
2,418	98	Roofs, HSG B			
5,396	98	Paved parking, HSG B			
119	61	>75% Grass cover, Good, HSG B			
7,933	97	Weighted Average			
119		1.50% Pervious Area			
7,814		98.50% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 18S: POST 17.A**

Runoff = 3.02 cfs @ 12.09 hrs, Volume= 0.231 af, Depth= 5.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
7,406	98	Paved parking, HSG B
1,509	98	Unconnected pavement, HSG B
4,989	61	>75% Grass cover, Good, HSG B
21,158	89	Weighted Average
4,989		23.58% Pervious Area
16,169		76.42% Impervious Area
1,509		9.33% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 19S: POST 24.A**

Runoff = 4.72 cfs @ 12.09 hrs, Volume= 0.342 af, Depth= 3.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
16,251	77	Woods, Good, HSG D
5,201	96	Gravel surface, HSG D
5,436	80	>75% Grass cover, Good, HSG D
6,221	30	Woods, Good, HSG A
3,810	39	>75% Grass cover, Good, HSG A
4,986	96	Gravel surface, HSG A
1,269	61	>75% Grass cover, Good, HSG B
2,270	96	Gravel surface, HSG B
45,444	73	Weighted Average
45,444		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 20S: POST 19.A**

Runoff = 4.50 cfs @ 12.09 hrs, Volume= 0.342 af, Depth= 5.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
7,254	98	Roofs, HSG B
15,358	98	Paved parking, HSG B
1,073	98	Unconnected pavement, HSG B
8,284	61	>75% Grass cover, Good, HSG B
31,969	88	Weighted Average
8,284		25.91% Pervious Area
23,685		74.09% Impervious Area
1,073		4.53% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 21S: POST 20.A**

Runoff = 5.00 cfs @ 12.09 hrs, Volume= 0.363 af, Depth= 3.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
5,596	98	Roofs, HSG B
1,630	98	Unconnected pavement, HSG B
1,749	96	Gravel surface, HSG B
40,318	61	>75% Grass cover, Good, HSG B
1,861	96	Gravel surface, HSG D
2,875	80	>75% Grass cover, Good, HSG D
54,029	69	Weighted Average
46,803		86.63% Pervious Area
7,226		13.37% Impervious Area
1,630		22.56% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

### Summary for Subcatchment 22S: POST 22.A

Runoff = 8.78 cfs @ 12.22 hrs, Volume= 0.857 af, Depth= 2.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
6,845	98	Roofs, HSG A
819	98	Unconnected pavement, HSG A
5,708	30	Woods, Good, HSG A
30,256	39	>75% Grass cover, Good, HSG A
11,252	98	Roofs, HSG B
2,667	98	Unconnected pavement, HSG B
88,631	61	>75% Grass cover, Good, HSG B
14,680	55	Woods, Good, HSG B
2,251	96	Gravel surface, HSG B
831	96	Gravel surface, HSG D
1,334	80	>75% Grass cover, Good, HSG D
462	77	Woods, Good, HSG D
165,736	61	Weighted Average
144,153		86.98% Pervious Area
21,583		13.02% Impervious Area
3,486		16.15% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.7	50	0.0200	0.15		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 3.10"
3.7	348	0.0500	1.57		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
5.8	486	0.0400	1.40		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
15.2	884	Total			

### Summary for Subcatchment 23S: POST 23.A

Runoff = 13.82 cfs @ 12.23 hrs, Volume= 1.362 af, Depth= 4.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
24,614	55	Woods, Good, HSG B
1,206	96	Gravel surface, HSG D
1,599	80	>75% Grass cover, Good, HSG D
148,745	77	Woods, Good, HSG D
176,164	74	Weighted Average
176,164		100.00% Pervious Area

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Type III 24-hr 100-year Rainfall=7.00"

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Page 122

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.7	Direct Entry,				

**Summary for Subcatchment 24S: POST 21.A**

Runoff = 5.16 cfs @ 12.10 hrs, Volume= 0.383 af, Depth= 2.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Adj	Description
1,194	98		Unconnected pavement, HSG B
248	96		Gravel surface, HSG B
72,586	61		>75% Grass cover, Good, HSG B
74,028	62	61	Weighted Average, UI Adjusted
72,834			98.39% Pervious Area
1,194			1.61% Impervious Area
1,194			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	Direct Entry,				

**Summary for Subcatchment 25S: POST 1.B**

Runoff = 0.24 cfs @ 12.38 hrs, Volume= 0.047 af, Depth= 0.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
7,624	55	Woods, Good, HSG B
176	61	>75% Grass cover, Good, HSG B
24,732	30	Woods, Good, HSG A
6,261	39	>75% Grass cover, Good, HSG A
38,793	37	Weighted Average
38,793		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.7	50	0.0300	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.10"
0.4	56	0.2700	2.60		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
11.1	106	Total			

**Summary for Subcatchment 26S: POST 10.A**

Runoff = 0.71 cfs @ 12.09 hrs, Volume= 0.060 af, Depth= 6.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description			
4,620	98	Paved parking, HSG B			
4,620		100.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 27S: POST 11.A**

Runoff = 0.80 cfs @ 12.09 hrs, Volume= 0.067 af, Depth= 6.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description			
5,180	98	Paved parking, HSG B			
5,180		100.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 28S: BACK ROOFS OF UNITS 1-10**

Runoff = 2.11 cfs @ 12.09 hrs, Volume= 0.177 af, Depth= 6.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description			
13,690	98	Roofs, HSG B			
13,690		100.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

**Summary for Subcatchment 38S: POST 25.A**

Runoff = 2.68 cfs @ 12.09 hrs, Volume= 0.199 af, Depth= 5.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
1,914	98	Roofs, HSG A
10,842	98	Unconnected pavement, HSG B
7,499	61	>75% Grass cover, Good, HSG B
20,255	84	Weighted Average
7,499		37.02% Pervious Area
12,756		62.98% Impervious Area
10,842		85.00% Unconnected

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0	Direct Entry,				

**Summary for Subcatchment 41S: POST 26.A**

Runoff = 1.13 cfs @ 12.09 hrs, Volume= 0.090 af, Depth= 6.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100-year Rainfall=7.00"

Area (sf)	CN	Description
4,658	98	Paved parking, HSG C
1,501	98	Paved parking, HSG B
352	61	>75% Grass cover, Good, HSG B
1,070	74	>75% Grass cover, Good, HSG C
7,581	93	Weighted Average
1,422		18.76% Pervious Area
6,159		81.24% Impervious Area

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0	Direct Entry,				

**Summary for Pond 5P: INFIL BASIN #1**

Inflow Area = 8.395 ac, 42.27% Impervious, Inflow Depth = 4.03" for 100-year event

Inflow = 31.27 cfs @ 12.10 hrs, Volume= 2.817 af

Outflow = 23.27 cfs @ 12.22 hrs, Volume= 2.818 af, Atten= 26%, Lag= 7.0 min

Discarded = 0.52 cfs @ 12.22 hrs, Volume= 0.891 af

Primary = 22.76 cfs @ 12.22 hrs, Volume= 1.927 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Type III 24-hr 100-year Rainfall=7.00"

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Page 125

Peak Elev= 315.57' @ 12.22 hrs Surf.Area= 9,030 sf Storage= 38,066 cf

Plug-Flow detention time= 288.3 min calculated for 2.816 af (100% of inflow)  
Center-of-Mass det. time= 289.2 min ( 1,098.4 - 809.2 )

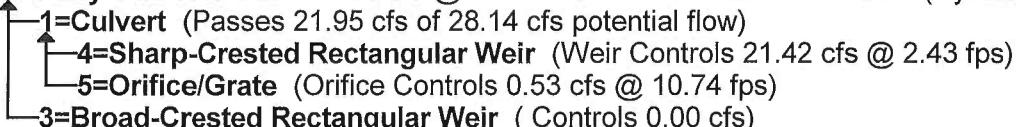
Volume	Invert	Avail.Storage	Storage Description		
#1	309.00'	42,092 cf	<b>Custom Stage Data (Irregular) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
309.00	2,439	200.1	0	0	2,439
310.00	3,839	242.8	3,113	3,113	3,960
312.00	5,491	281.0	9,281	12,394	5,636
314.00	7,393	320.0	12,837	25,230	7,596
316.00	9,513	355.0	16,862	42,092	9,594

Device	Routing	Invert	Outlet Devices
#1	Primary	309.00'	<b>24.0" Round Culvert</b> L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 309.00' / 308.00' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Discarded	309.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 306.73'
#3	Primary	315.60'	<b>20.0' long x 12.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.57 2.62 2.70 2.67 2.66 2.67 2.66 2.64
#4	Device 1	315.00'	<b>16.0' long Sharp-Crested Rectangular Weir</b> 0 End Contraction(s)
#5	Device 1	310.45'	<b>3.0" Vert. Orifice/Grate</b> C= 0.600

**Discarded OutFlow** Max=0.51 cfs @ 12.22 hrs HW=315.55' (Free Discharge)  
 ↑ 2=Exfiltration ( Controls 0.51 cfs)

**Primary OutFlow** Max=21.95 cfs @ 12.22 hrs HW=315.55' TW=0.00' (Dynamic Tailwater)



### Summary for Pond 6P: INFIL BASIN #2

Inflow Area =	10.289 ac, 22.88% Impervious, Inflow Depth = 3.92"	for 100-year event
Inflow =	31.44 cfs @ 12.16 hrs, Volume=	3.361 af
Outflow =	11.99 cfs @ 12.60 hrs, Volume=	3.361 af, Atten= 62%, Lag= 26.2 min
Discarded =	1.36 cfs @ 12.60 hrs, Volume=	2.013 af
Primary =	10.63 cfs @ 12.60 hrs, Volume=	1.348 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 317.74' @ 12.60 hrs Surf.Area= 36,276 sf Storage= 64,304 cf

Plug-Flow detention time= 366.1 min calculated for 3.359 af (100% of inflow)  
 Center-of-Mass det. time= 366.8 min ( 1,188.6 - 821.8 )

Volume	Invert	Avail.Storage	Storage Description		
#1	314.00'	118,862 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
314.00	2,513	223.0	0	0	2,513
316.00	17,939	667.0	18,111	18,111	33,971
318.00	39,591	937.0	56,120	74,231	68,472
319.00	49,868	1,019.0	44,631	118,862	81,273

Device	Routing	Invert	Outlet Devices
#1	Primary	314.00'	<b>24.0" Round Culvert</b> L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.00' / 312.60' S= 0.0280 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Discarded	314.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 312.00'
#3	Primary	317.80'	<b>25.0' long x 20.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#4	Device 1	317.40'	<b>16.0' long Sharp-Crested Rectangular Weir</b> 0 End Contraction(s)
#5	Device 1	315.55'	<b>3.0" Vert. Orifice/Grate</b> C= 0.600

**Discarded OutFlow** Max=1.36 cfs @ 12.60 hrs HW=317.74' (Free Discharge)

↑ 2=Exfiltration (Controls 1.36 cfs)

**Primary OutFlow** Max=10.62 cfs @ 12.60 hrs HW=317.74' TW=0.00' (Dynamic Tailwater)

↑ 1=Culvert (Passes 10.62 cfs of 19.76 cfs potential flow)  
↑ 4=Sharp-Crested Rectangular Weir (Weir Controls 10.28 cfs @ 1.90 fps)  
↓ 5=Orifice/Grate (Orifice Controls 0.34 cfs @ 6.92 fps)  
3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Summary for Pond 7P: DMH-2

Inflow Area = 4.772 ac, 49.56% Impervious, Inflow Depth = 4.30" for 100-year event  
 Inflow = 21.88 cfs @ 12.10 hrs, Volume= 1.709 af  
 Outflow = 21.88 cfs @ 12.10 hrs, Volume= 1.709 af, Atten= 0%, Lag= 0.0 min  
 Primary = 21.88 cfs @ 12.10 hrs, Volume= 1.709 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 318.26' @ 12.10 hrs

Flood Elev= 319.42'

Device	Routing	Invert	Outlet Devices
#1	Primary	313.90'	<b>24.0" Round Culvert</b> L= 60.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 313.90' / 312.70' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=21.46 cfs @ 12.10 hrs HW=318.24' TW=315.01' (Dynamic Tailwater)  
 ↑=Culvert (Inlet Controls 21.46 cfs @ 6.83 fps)

### Summary for Pond 8P: CB-5

Inflow Area = 0.536 ac, 69.71% Impervious, Inflow Depth = 5.48" for 100-year event  
 Inflow = 3.24 cfs @ 12.09 hrs, Volume= 0.245 af  
 Outflow = 3.24 cfs @ 12.09 hrs, Volume= 0.245 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.24 cfs @ 12.09 hrs, Volume= 0.245 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 319.03' @ 12.14 hrs  
 Flood Elev= 319.26'

Device	Routing	Invert	Outlet Devices
#1	Primary	315.26'	<b>12.0" Round Culvert</b> L= 20.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 315.26' / 314.90' S= 0.0180 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.26 cfs @ 12.09 hrs HW=318.24' TW=318.07' (Dynamic Tailwater)  
 ↑=Culvert (Inlet Controls 1.26 cfs @ 1.60 fps)

### Summary for Pond 9P: CB-6

Inflow Area = 0.416 ac, 86.61% Impervious, Inflow Depth = 6.17" for 100-year event  
 Inflow = 2.69 cfs @ 12.09 hrs, Volume= 0.214 af  
 Outflow = 2.69 cfs @ 12.09 hrs, Volume= 0.214 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.69 cfs @ 12.09 hrs, Volume= 0.214 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 318.77' @ 12.14 hrs  
 Flood Elev= 319.65'

Device	Routing	Invert	Outlet Devices
#1	Primary	315.65'	<b>12.0" Round Culvert</b> L= 20.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 315.65' / 314.90' S= 0.0375 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=317.87' TW=318.05' (Dynamic Tailwater)  
 ↑=Culvert (Controls 0.00 cfs)

### Summary for Pond 10P: CB-4

Inflow Area = 0.486 ac, 82.05% Impervious, Inflow Depth = 5.94" for 100-year event  
 Inflow = 3.09 cfs @ 12.09 hrs, Volume= 0.241 af  
 Outflow = 3.09 cfs @ 12.09 hrs, Volume= 0.241 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.09 cfs @ 12.09 hrs, Volume= 0.241 af

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Page 128

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 316.39' @ 12.09 hrs

Flood Elev= 318.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	314.82'	<b>12.0" Round Culvert</b> L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.82' / 314.40' S= 0.0168 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=3.02 cfs @ 12.09 hrs HW=316.34' TW=314.87' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 3.02 cfs @ 3.84 fps)

**Summary for Pond 11P: DMH-3**

Inflow Area = 3.820 ac, 42.71% Impervious, Inflow Depth = 3.93" for 100-year event  
 Inflow = 16.01 cfs @ 12.10 hrs, Volume= 1.251 af  
 Outflow = 16.01 cfs @ 12.10 hrs, Volume= 1.251 af, Atten= 0%, Lag= 0.0 min  
 Primary = 16.01 cfs @ 12.10 hrs, Volume= 1.251 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 319.91' @ 12.11 hrs

Flood Elev= 321.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	317.10'	<b>24.0" Round Culvert</b> L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 317.10' / 314.90' S= 0.0349 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=15.47 cfs @ 12.10 hrs HW=319.88' TW=318.20' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 15.47 cfs @ 4.92 fps)

**Summary for Pond 12P: DMH-4**

Inflow Area = 3.355 ac, 39.90% Impervious, Inflow Depth = 3.76" for 100-year event  
 Inflow = 13.37 cfs @ 12.11 hrs, Volume= 1.051 af  
 Outflow = 13.37 cfs @ 12.11 hrs, Volume= 1.051 af, Atten= 0%, Lag= 0.0 min  
 Primary = 13.37 cfs @ 12.11 hrs, Volume= 1.051 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 324.21' @ 12.11 hrs

Flood Elev= 326.96'

Device	Routing	Invert	Outlet Devices
#1	Primary	321.96'	<b>24.0" Round Culvert</b> L= 99.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 321.96' / 317.20' S= 0.0481 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=13.17 cfs @ 12.11 hrs HW=324.18' TW=319.86' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 13.17 cfs @ 4.19 fps)

### Summary for Pond 13P: DMH-6

Inflow Area = 0.912 ac, 77.06% Impervious, Inflow Depth = 5.35" for 100-year event  
 Inflow = 5.40 cfs @ 12.09 hrs, Volume= 0.407 af  
 Outflow = 5.40 cfs @ 12.09 hrs, Volume= 0.407 af, Atten= 0%, Lag= 0.0 min  
 Primary = 5.40 cfs @ 12.09 hrs, Volume= 0.407 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 336.64' @ 12.09 hrs  
 Flood Elev= 340.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.25'	<b>18.0" Round Culvert</b> L= 124.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.25' / 328.11' S= 0.0576 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=5.28 cfs @ 12.09 hrs HW=336.61' TW=329.37' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 5.28 cfs @ 3.13 fps)

### Summary for Pond 14P: CB-8

Inflow Area = 0.336 ac, 81.36% Impervious, Inflow Depth = 5.71" for 100-year event  
 Inflow = 2.09 cfs @ 12.09 hrs, Volume= 0.160 af  
 Outflow = 2.09 cfs @ 12.09 hrs, Volume= 0.160 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.09 cfs @ 12.09 hrs, Volume= 0.160 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 337.53' @ 12.09 hrs  
 Flood Elev= 340.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	336.55'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 336.55' / 335.75' S= 0.0571 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.04 cfs @ 12.09 hrs HW=337.51' TW=336.61' (Dynamic Tailwater)  
 ↗1=Culvert (Inlet Controls 2.04 cfs @ 2.63 fps)

### Summary for Pond 15P: CB-7

Inflow Area = 0.576 ac, 74.55% Impervious, Inflow Depth = 5.14" for 100-year event  
 Inflow = 3.32 cfs @ 12.09 hrs, Volume= 0.247 af  
 Outflow = 3.32 cfs @ 12.09 hrs, Volume= 0.247 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.32 cfs @ 12.09 hrs, Volume= 0.247 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 337.76' @ 12.10 hrs

Flood Elev= 339.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.95'	<b>12.0" Round Culvert</b> L= 10.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.95' / 335.75' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=3.12 cfs @ 12.09 hrs HW=337.70' TW=336.61' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 3.12 cfs @ 3.97 fps)

### Summary for Pond 16P: DMH-5

Inflow Area = 0.912 ac, 77.06% Impervious, Inflow Depth = 5.35" for 100-year event  
 Inflow = 5.40 cfs @ 12.09 hrs, Volume= 0.407 af  
 Outflow = 5.40 cfs @ 12.09 hrs, Volume= 0.407 af, Atten= 0%, Lag= 0.0 min  
 Primary = 5.40 cfs @ 12.09 hrs, Volume= 0.407 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 329.40' @ 12.09 hrs

Flood Elev= 332.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	328.01'	<b>18.0" Round Culvert</b> L= 91.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 328.01' / 322.56' S= 0.0599 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=5.28 cfs @ 12.09 hrs HW=329.37' TW=324.12' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 5.28 cfs @ 3.13 fps)

### Summary for Pond 17P: DMH-15

Inflow Area = 0.505 ac, 63.46% Impervious, Inflow Depth = 5.21" for 100-year event  
 Inflow = 2.86 cfs @ 12.09 hrs, Volume= 0.219 af  
 Outflow = 2.86 cfs @ 12.09 hrs, Volume= 0.219 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.86 cfs @ 12.09 hrs, Volume= 0.219 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 336.96' @ 12.09 hrs

Flood Elev= 340.37'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.55'	<b>12.0" Round Culvert</b> L= 69.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.55' / 334.86' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.65 cfs @ 12.09 hrs HW=336.93' TW=336.14' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.65 cfs @ 3.38 fps)

### Summary for Pond 18P: CB-17

Inflow Area = 0.154 ac, 98.52% Impervious, Inflow Depth = 6.64" for 100-year event  
 Inflow = 1.03 cfs @ 12.09 hrs, Volume= 0.085 af  
 Outflow = 1.03 cfs @ 12.09 hrs, Volume= 0.085 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.03 cfs @ 12.09 hrs, Volume= 0.085 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 337.04' @ 12.14 hrs  
 Flood Elev= 339.81'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.81'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.81' / 335.65' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=336.83' TW=336.92' (Dynamic Tailwater)  
 ↑—1=Culvert (Controls 0.00 cfs)

### Summary for Pond 19P: CB-16

Inflow Area = 0.351 ac, 48.14% Impervious, Inflow Depth = 4.58" for 100-year event  
 Inflow = 1.84 cfs @ 12.09 hrs, Volume= 0.134 af  
 Outflow = 1.84 cfs @ 12.09 hrs, Volume= 0.134 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.84 cfs @ 12.09 hrs, Volume= 0.134 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 337.24' @ 12.13 hrs  
 Flood Elev= 339.81'

Device	Routing	Invert	Outlet Devices
#1	Primary	335.81'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.81' / 335.65' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.26 cfs @ 12.09 hrs HW=337.11' TW=336.93' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 1.26 cfs @ 1.60 fps)

### Summary for Pond 20P: DMH-14

Inflow Area = 0.505 ac, 63.46% Impervious, Inflow Depth = 5.21" for 100-year event  
 Inflow = 2.86 cfs @ 12.09 hrs, Volume= 0.219 af  
 Outflow = 2.86 cfs @ 12.09 hrs, Volume= 0.219 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.86 cfs @ 12.09 hrs, Volume= 0.219 af

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Page 132

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 336.18' @ 12.09 hrs

Flood Elev= 341.49'

<u>Device</u>	<u>Routing</u>	<u>Invert</u>	<u>Outlet Devices</u>
#1	Primary	334.76'	<b>12.0" Round Culvert</b> L= 88.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 334.76' / 333.88' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.79 cfs @ 12.09 hrs HW=336.14' TW=334.91' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 2.79 cfs @ 3.56 fps)

**Summary for Pond 21P: DMH-13**

Inflow Area = 1.039 ac, 73.61% Impervious, Inflow Depth = 5.41" for 100-year event  
 Inflow = 6.08 cfs @ 12.09 hrs, Volume= 0.468 af  
 Outflow = 6.08 cfs @ 12.09 hrs, Volume= 0.468 af, Atten= 0%, Lag= 0.0 min  
 Primary = 6.08 cfs @ 12.09 hrs, Volume= 0.468 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 334.95' @ 12.09 hrs

Flood Elev= 342.06'

<u>Device</u>	<u>Routing</u>	<u>Invert</u>	<u>Outlet Devices</u>
#1	Primary	333.38'	<b>18.0" Round Culvert</b> L= 136.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 333.38' / 330.66' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=5.93 cfs @ 12.09 hrs HW=334.91' TW=332.09' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 5.93 cfs @ 3.35 fps)

**Summary for Pond 22P: CB-14**

Inflow Area = 0.373 ac, 76.57% Impervious, Inflow Depth = 5.14" for 100-year event  
 Inflow = 2.15 cfs @ 12.09 hrs, Volume= 0.160 af  
 Outflow = 2.15 cfs @ 12.09 hrs, Volume= 0.160 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.15 cfs @ 12.09 hrs, Volume= 0.160 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 339.22' @ 12.09 hrs

Flood Elev= 342.21'

<u>Device</u>	<u>Routing</u>	<u>Invert</u>	<u>Outlet Devices</u>
#1	Primary	338.21'	<b>12.0" Round Culvert</b> L= 17.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 338.21' / 338.00' S= 0.0124 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.10 cfs @ 12.09 hrs HW=339.20' TW=334.91' (Dynamic Tailwater)  
 ↑=Culvert (Inlet Controls 2.10 cfs @ 2.68 fps)

### Summary for Pond 23P: CB-15

Inflow Area = 0.161 ac, 98.59% Impervious, Inflow Depth = 6.64" for 100-year event  
 Inflow = 1.07 cfs @ 12.09 hrs, Volume= 0.089 af  
 Outflow = 1.07 cfs @ 12.09 hrs, Volume= 0.089 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.07 cfs @ 12.09 hrs, Volume= 0.089 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 338.93' @ 12.09 hrs  
 Flood Elev= 342.31'

Device	Routing	Invert	Outlet Devices
#1	Primary	338.31'	<b>12.0" Round Culvert</b> L= 28.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 338.31' / 338.00' S= 0.0111 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.05 cfs @ 12.09 hrs HW=338.92' TW=334.90' (Dynamic Tailwater)  
 ↑=Culvert (Inlet Controls 1.05 cfs @ 2.09 fps)

### Summary for Pond 24P: DMH-12

Inflow Area = 1.039 ac, 73.61% Impervious, Inflow Depth = 5.41" for 100-year event  
 Inflow = 6.08 cfs @ 12.09 hrs, Volume= 0.468 af  
 Outflow = 6.08 cfs @ 12.09 hrs, Volume= 0.468 af, Atten= 0%, Lag= 0.0 min  
 Primary = 6.08 cfs @ 12.09 hrs, Volume= 0.468 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 332.13' @ 12.09 hrs  
 Flood Elev= 338.71'

Device	Routing	Invert	Outlet Devices
#1	Primary	330.56'	<b>18.0" Round Culvert</b> L= 99.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 330.56' / 327.84' S= 0.0275 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=5.93 cfs @ 12.09 hrs HW=332.09' TW=329.27' (Dynamic Tailwater)  
 ↑=Culvert (Inlet Controls 5.93 cfs @ 3.35 fps)

### Summary for Pond 25P: DMH-11

Inflow Area = 1.039 ac, 73.61% Impervious, Inflow Depth = 5.41" for 100-year event  
 Inflow = 6.08 cfs @ 12.09 hrs, Volume= 0.468 af  
 Outflow = 6.08 cfs @ 12.09 hrs, Volume= 0.468 af, Atten= 0%, Lag= 0.0 min  
 Primary = 6.08 cfs @ 12.09 hrs, Volume= 0.468 af

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Page 134

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 329.31' @ 12.09 hrs

Flood Elev= 332.34'

Device	Routing	Invert	Outlet Devices
#1	Primary	327.74'	<b>18.0" Round Culvert</b> L= 76.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 327.74' / 322.02' S= 0.0753 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=5.93 cfs @ 12.09 hrs HW=329.27' TW=324.92' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 5.93 cfs @ 3.35 fps)

**Summary for Pond 26P: DMH-10**

Inflow Area = 1.707 ac, 77.06% Impervious, Inflow Depth = 5.62" for 100-year event  
 Inflow = 10.32 cfs @ 12.09 hrs, Volume= 0.800 af  
 Outflow = 10.32 cfs @ 12.09 hrs, Volume= 0.800 af, Atten= 0%, Lag= 0.0 min  
 Primary = 10.32 cfs @ 12.09 hrs, Volume= 0.800 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 325.02' @ 12.09 hrs

Flood Elev= 327.14'

Device	Routing	Invert	Outlet Devices
#1	Primary	321.92'	<b>18.0" Round Culvert</b> L= 174.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 321.92' / 316.81' S= 0.0294 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=10.06 cfs @ 12.09 hrs HW=324.91' TW=318.77' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 10.06 cfs @ 5.69 fps)

**Summary for Pond 27P: CB-13**

Inflow Area = 0.182 ac, 98.50% Impervious, Inflow Depth = 6.64" for 100-year event  
 Inflow = 1.22 cfs @ 12.09 hrs, Volume= 0.101 af  
 Outflow = 1.22 cfs @ 12.09 hrs, Volume= 0.101 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.22 cfs @ 12.09 hrs, Volume= 0.101 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 325.13' @ 12.14 hrs

Flood Elev= 327.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.51'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.51' / 323.23' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=324.64' TW=324.90' (Dynamic Tailwater)  
 ↑—1=Culvert (Controls 0.00 cfs)

### Summary for Pond 28P: CB-12

Inflow Area = 0.486 ac, 76.42% Impervious, Inflow Depth = 5.71" for 100-year event  
 Inflow = 3.02 cfs @ 12.09 hrs, Volume= 0.231 af  
 Outflow = 3.02 cfs @ 12.09 hrs, Volume= 0.231 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.02 cfs @ 12.09 hrs, Volume= 0.231 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 325.75' @ 12.12 hrs  
 Flood Elev= 327.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.51'	<b>12.0" Round Culvert</b> L= 14.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.51' / 323.23' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.24 cfs @ 12.09 hrs HW=325.48' TW=324.91' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.24 cfs @ 2.85 fps)

### Summary for Pond 29P: DMH-9

Inflow Area = 2.440 ac, 76.17% Impervious, Inflow Depth = 5.62" for 100-year event  
 Inflow = 14.81 cfs @ 12.09 hrs, Volume= 1.142 af  
 Outflow = 14.81 cfs @ 12.09 hrs, Volume= 1.142 af, Atten= 0%, Lag= 0.0 min  
 Primary = 14.81 cfs @ 12.09 hrs, Volume= 1.142 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 318.84' @ 12.09 hrs  
 Flood Elev= 322.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	316.31'	<b>24.0" Round Culvert</b> L= 62.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 316.31' / 316.00' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=14.45 cfs @ 12.09 hrs HW=318.77' TW=316.63' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 14.45 cfs @ 4.60 fps)

### Summary for Pond 30P: CB-11

Inflow Area = 0.734 ac, 74.09% Impervious, Inflow Depth = 5.59" for 100-year event  
 Inflow = 4.50 cfs @ 12.09 hrs, Volume= 0.342 af  
 Outflow = 4.50 cfs @ 12.09 hrs, Volume= 0.342 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.50 cfs @ 12.09 hrs, Volume= 0.342 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 319.16' @ 12.13 hrs

Flood Elev= 321.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	317.30'	<b>18.0" Round Culvert</b> L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 317.30' / 316.81' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=2.69 cfs @ 12.09 hrs HW=318.93' TW=318.77' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.69 cfs @ 1.52 fps)

### Summary for Pond 31P: PROP. CULVERT

Inflow Area = 2.338 ac, 19.03% Impervious, Inflow Depth = 2.90" for 100-year event  
 Inflow = 5.03 cfs @ 12.32 hrs, Volume= 0.565 af  
 Outflow = 5.03 cfs @ 12.32 hrs, Volume= 0.565 af, Atten= 0%, Lag= 0.0 min  
 Primary = 5.03 cfs @ 12.32 hrs, Volume= 0.565 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 316.69' @ 12.31 hrs

Flood Elev= 318.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	314.00'	<b>15.0" Round Culvert</b> L= 75.8' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 314.00' / 313.00' S= 0.0132 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf

**Primary OutFlow** Max=5.07 cfs @ 12.32 hrs HW=316.67' TW=315.49' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 5.07 cfs @ 4.13 fps)

### Summary for Pond 32P: CB-10

Inflow Area = 0.106 ac, 100.00% Impervious, Inflow Depth = 6.76" for 100-year event  
 Inflow = 0.71 cfs @ 12.09 hrs, Volume= 0.060 af  
 Outflow = 0.71 cfs @ 12.09 hrs, Volume= 0.060 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.71 cfs @ 12.09 hrs, Volume= 0.060 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 324.89' @ 12.24 hrs

Flood Elev= 327.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.00'	<b>18.0" Round Culvert</b> L= 55.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.00' / 322.73' S= 0.0049 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=323.94' TW=324.23' (Dynamic Tailwater)  
 ↑—1=Culvert (Controls 0.00 cfs)

### Summary for Pond 33P: CB-9

Inflow Area = 0.119 ac, 100.00% Impervious, Inflow Depth = 6.76" for 100-year event  
 Inflow = 0.80 cfs @ 12.09 hrs, Volume= 0.067 af  
 Outflow = 0.80 cfs @ 12.09 hrs, Volume= 0.067 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.80 cfs @ 12.09 hrs, Volume= 0.067 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 324.64' @ 12.20 hrs  
 Flood Elev= 327.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	323.00'	<b>18.0" Round Culvert</b> L= 7.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 323.00' / 322.52' S= 0.0686 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=323.85' TW=324.18' (Dynamic Tailwater)  
 ↑—1=Culvert (Controls 0.00 cfs)

### Summary for Pond 34P: CB-18

Inflow Area = 0.465 ac, 62.98% Impervious, Inflow Depth = 5.14" for 100-year event  
 Inflow = 2.68 cfs @ 12.09 hrs, Volume= 0.199 af  
 Outflow = 2.68 cfs @ 12.09 hrs, Volume= 0.199 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.68 cfs @ 12.09 hrs, Volume= 0.199 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 321.30' @ 12.09 hrs  
 Flood Elev= 325.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	320.00'	<b>12.0" Round Culvert</b> L= 16.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 320.00' / 319.04' S= 0.0600 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.61 cfs @ 12.09 hrs HW=321.27' TW=320.21' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.61 cfs @ 3.33 fps)

### Summary for Pond 35P: DMH-8

Inflow Area = 2.324 ac, 22.24% Impervious, Inflow Depth = 2.98" for 100-year event  
 Inflow = 7.41 cfs @ 12.12 hrs, Volume= 0.578 af  
 Outflow = 7.41 cfs @ 12.12 hrs, Volume= 0.578 af, Atten= 0%, Lag= 0.0 min  
 Primary = 7.41 cfs @ 12.12 hrs, Volume= 0.578 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 324.89' @ 12.19 hrs

Flood Elev= 327.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	322.63'	<b>24.0" Round Culvert</b> L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 322.63' / 322.52' S= 0.0052 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.97 cfs @ 12.12 hrs HW=324.49' TW=324.46' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 1.97 cfs @ 0.65 fps)

### Summary for Pond 36P: DMH-16

Inflow Area = 0.465 ac, 62.98% Impervious, Inflow Depth = 5.14" for 100-year event  
 Inflow = 2.68 cfs @ 12.09 hrs, Volume= 0.199 af  
 Outflow = 2.68 cfs @ 12.09 hrs, Volume= 0.199 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.68 cfs @ 12.09 hrs, Volume= 0.199 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 320.42' @ 12.14 hrs

Flood Elev= 326.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	318.94'	<b>12.0" Round Culvert</b> L= 21.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 318.94' / 317.20' S= 0.0829 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=1.94 cfs @ 12.09 hrs HW=320.21' TW=319.79' (Dynamic Tailwater)  
 ↑ 1=Culvert (Inlet Controls 1.94 cfs @ 2.47 fps)

### Summary for Pond 37P: DMH-7

Inflow Area = 2.443 ac, 26.02% Impervious, Inflow Depth = 3.17" for 100-year event  
 Inflow = 8.15 cfs @ 12.12 hrs, Volume= 0.645 af  
 Outflow = 8.15 cfs @ 12.12 hrs, Volume= 0.645 af, Atten= 0%, Lag= 0.0 min  
 Primary = 8.15 cfs @ 12.12 hrs, Volume= 0.645 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 324.64' @ 12.15 hrs

Flood Elev= 327.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	322.42'	<b>24.0" Round Culvert</b> L= 78.1' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 322.42' / 321.96' S= 0.0059 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=6.74 cfs @ 12.12 hrs HW=324.43' TW=324.11' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 6.74 cfs @ 2.15 fps)

### Summary for Pond 39P: CB-2 [EXISTING]

Inflow Area = 0.432 ac, 80.78% Impervious, Inflow Depth = 6.17" for 100-year event  
 Inflow = 2.81 cfs @ 12.09 hrs, Volume= 0.222 af  
 Outflow = 2.81 cfs @ 12.09 hrs, Volume= 0.222 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.81 cfs @ 12.09 hrs, Volume= 0.222 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 312.14' @ 12.11 hrs  
 Flood Elev= 314.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.05'	<b>12.0" Round Culvert</b> L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.05' / 310.00' S= 0.0010 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.42 cfs @ 12.09 hrs HW=311.99' TW=311.34' (Dynamic Tailwater)  
 ↑—1=Culvert (Inlet Controls 2.42 cfs @ 3.08 fps)

### Summary for Pond 40P: CB-1 [EXISTING]

Inflow Area = 0.174 ac, 81.24% Impervious, Inflow Depth = 6.17" for 100-year event  
 Inflow = 1.13 cfs @ 12.09 hrs, Volume= 0.090 af  
 Outflow = 1.13 cfs @ 12.09 hrs, Volume= 0.090 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.13 cfs @ 12.09 hrs, Volume= 0.090 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 312.22' @ 12.16 hrs  
 Flood Elev= 314.39'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.75'	<b>12.0" Round Culvert</b> L= 17.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.75' / 310.15' S= 0.0353 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=0.00 cfs @ 12.09 hrs HW=311.67' TW=311.99' (Dynamic Tailwater)  
 ↑—1=Culvert (Controls 0.00 cfs)

### Summary for Pond 42P: DMH-1 [EXISTING]

Inflow Area = 0.432 ac, 80.78% Impervious, Inflow Depth = 6.17" for 100-year event  
 Inflow = 2.81 cfs @ 12.09 hrs, Volume= 0.222 af  
 Outflow = 2.81 cfs @ 12.09 hrs, Volume= 0.222 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.81 cfs @ 12.09 hrs, Volume= 0.222 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Peak Elev= 311.38' @ 12.09 hrs

Flood Elev= 315.33'

Device	Routing	Invert	Outlet Devices
#1	Primary	310.00'	<b>12.0" Round Culvert</b> L= 92.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 310.00' / 308.02' S= 0.0215 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

**Primary OutFlow** Max=2.73 cfs @ 12.09 hrs HW=311.34' TW=0.00' (Dynamic Tailwater)  
 ↑1=Culvert (Inlet Controls 2.73 cfs @ 3.48 fps)

### Summary for Pond 43P: SWALE & FIELD BASIN

Inflow Area = 2.218 ac, 18.52% Impervious, Inflow Depth = 2.90" for 100-year event  
 Inflow = 6.83 cfs @ 12.12 hrs, Volume= 0.536 af  
 Outflow = 6.75 cfs @ 12.13 hrs, Volume= 0.536 af, Atten= 1%, Lag= 0.6 min  
 Discarded = 0.02 cfs @ 12.13 hrs, Volume= 0.018 af  
 Primary = 6.72 cfs @ 12.13 hrs, Volume= 0.518 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs  
 Peak Elev= 328.30' @ 12.13 hrs Surf.Area= 893 sf Storage= 222 cf

Plug-Flow detention time= 1.0 min calculated for 0.536 af (100% of inflow)  
 Center-of-Mass det. time= 1.0 min ( 849.3 - 848.4 )

Volume	Invert	Avail.Storage	Storage Description		
#1	328.00'	1,150 cf	<b>Custom Stage Data (Irregular) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
328.00	600	119.4	0	0	600
329.00	1,808	444.3	1,150	1,150	15,177

Device	Routing	Invert	Outlet Devices
#1	Discarded	328.00'	<b>1.020 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 320.50'
#2	Primary	324.00'	<b>18.0" Round Culvert</b> L= 51.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 324.00' / 322.73' S= 0.0249 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#3	Device 2	328.00'	<b>48.0" Horiz. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Discarded OutFlow** Max=0.02 cfs @ 12.13 hrs HW=328.29' (Free Discharge)  
 ↑1=Exfiltration ( Controls 0.02 cfs )

**Primary OutFlow** Max=6.57 cfs @ 12.13 hrs HW=328.29' TW=324.53' (Dynamic Tailwater)  
 ↑2=Culvert (Passes 6.57 cfs of 12.65 cfs potential flow)  
 ↑3=Orifice/Grate (Weir Controls 6.57 cfs @ 1.78 fps)

**Summary for Link 4L: DP-A**

Inflow Area = 23.100 ac, 27.90% Impervious, Inflow Depth = 2.38" for 100-year event  
Inflow = 33.53 cfs @ 12.20 hrs, Volume= 4.585 af  
Primary = 33.53 cfs @ 12.20 hrs, Volume= 4.585 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Summary for Link 5L: DP-B**

Inflow Area = 0.891 ac, 0.00% Impervious, Inflow Depth = 0.63" for 100-year event  
Inflow = 0.24 cfs @ 12.38 hrs, Volume= 0.047 af  
Primary = 0.24 cfs @ 12.38 hrs, Volume= 0.047 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

**Appendix F – Stormwater Calculations**

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**INSTRUCTIONS:**

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Location: **TSS removal with pretreatment calculation.**

# TSS Removal

## Calculation Worksheet

BMP <sup>1</sup>	C	TSS Removal Rate <sup>1</sup>	D	Starting TSS Load*	E	Amount Removed (C*D)	F	Remaining Load (D-E)
Deep Sump and Hooded Catch Basin	0.25			1.00		0.25		0.75
Sediment Forebay	0.25			0.75		0.19		0.56
	0.00			0.56		0.00		0.56
	0.00			0.56		0.00		0.56
	0.00			0.56		0.00		0.56

**Separate storm needs to be completed for Each Outlet or BMP Train**

**Total TSS Removal =**

Project: Enclave at Boxborough
Prepared By: RPV
Date: 15-Jul-19

\* Equals remaining load from previous BMP (E)  
which enters the BMP

**INSTRUCTIONS:**

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Location: **TSS removal for overall site.**

**TSS Removal  
Calculation Worksheet**

B	C	D	E	F
BMP <sup>1</sup>	TSS Removal Rate <sup>1</sup>	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
Deep Sump and Hooded Catch Basin	0.25	1.00	0.25	0.75
Infiltration Basin	0.80	0.75	0.60	0.15
	0.00	0.15	0.00	0.15
	0.00	0.15	0.00	0.15
	0.00	0.15	0.00	0.15

Separate row needs to be completed for Each Outlet or BMP Train

Total TSS Removal =

Project: Enclave at Boxborough
Prepared By: RPV
Date: 15-Jul-19

\* Equals remaining load from previous BMP (E)  
which enters the BMP

**INSTRUCTIONS:**

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Location: **TSS removal for overall site.**

# TSS Removal

## Calculation Worksheet

BMP <sup>1</sup>	C TSS Removal Rate <sup>1</sup>	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Deep Sump and Hooded Catch Basin	0.25	1.00	0.25	0.75
Proprietary Treatment Practice	0.89	0.75	0.67	0.08
	0.00	0.08	0.00	0.08
	0.00	0.08	0.00	0.08
	0.00	0.08	0.00	0.08

**Separate row needs to be completed for Each Outlet or BMP Train**

**Total TSS Removal = 92%**

Project: Enclave at Boxborough  
 Prepared By: RPV  
 Date: 15-Jul-19

\* Equals remaining load from previous BMP (E)  
 which enters the BMP

## Infiltration Basin #1

### Stormwater Recharge Calculations

#### CALCULATIONS

##### Recharge Volume, Rv:

$$R_v = A_c \times F$$

Hydrologic Soil Group	Impervious Area (Ac) <sup>1</sup>	Target Depth (F)	Recharge Volume (Rv) Ac-feet
A	0.828	0.6	0.041
B	2.721	0.35	0.079
C		0.25	0.000
Total	3.549		0.121

Total Recharge Volume Required = 0.038 Ac-ft

Total Recharge Volume Required (Rv) = 1,673 C.ft

\*Recharge Vol. Provided (from Infil. Basin 1) = 4,917.5 C.ft

\*Stage Area Storage at elevation of outlet control structure = 310.45 (See HydroCAD Report)

##### Required Sediment Forebay vol, Fv:

$$F_v = A_c (\text{cu. ft}) \times 0.1 \text{ inch of impervious area}$$

<sup>1</sup> Imp. area captured by ponds, Ap = 3.549 Ac

Required Sediment Forebay vol, Fv= 1,288 C.ft

Sediment Forebay Volume Provided = 1,387 C.ft

#### REFERENCES

Table 2.3.2: Recharge Target Depth by Hydrologic Soil Group

NRCS Hydrologic Soil Group	Approx. Soil Texture	Target Depth Factor (F)
A	sand	0.6 inch
B	loam	0.35 inch
C	silty loam	0.25 inch
D	clay	0.1 inch

#### *Drawdown Calculations*

#### CALCULATIONS

##### Proposed Infiltration Area Calculations:

$$\text{Drawdown} = \frac{R_v}{(\text{Rawls Rate})(\text{Bottom Area})}$$

#### REFERENCES

Table 2.3.3: 1982 Rawls Rates

Texture Class	NRCS Hydrologic Soil Group	Infiltration Rate
1 Sand	A	8.27 in/hr
2 Loamy Sand	A	2.41 in/hr
3 Sandy Loam	B	1.02 in/hr
4 Loam	B	0.52 in/hr
5 Silt Loam	C	0.27 in/hr
6 Sandy Clay Loam	C	0.17 in/hr
7 Clay Loam	D	0.09 in/hr
8 Silty Clay Loam	D	0.06 in/hr
9 Sandy Clay	D	0.05 in/hr
10 Silty Clay	D	0.04 in/hr
11 Clay	D	0.02 in/hr

#### NOTES:

##### Input Values

<sup>1</sup> = Refer to Proposed Conditions HydroCAD modeling report

## Stage-Area-Storage for Pond 5P: INFIL BASIN #1

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
309.00	2,439	0	310.04	3,869	3,267
309.02	2,464	49	310.06	3,884	3,344
309.04	2,489	99	310.08	3,899	3,422
309.06	2,514	149	310.10	3,915	3,500
309.08	2,539	199	310.12	3,930	3,579
309.10	2,565	250	310.14	3,945	3,658
309.12	2,590	302	310.16	3,960	3,737
309.14	2,616	354	310.18	3,976	3,816
309.16	2,642	406	310.20	3,991	3,896
309.18	2,668	459	310.22	4,006	3,976
309.20	2,694	513	310.24	4,022	4,056
309.22	2,720	567	310.26	4,037	4,136
309.24	2,746	622	310.28	4,053	4,217
309.26	2,773	677	310.30	4,068	4,299
309.28	2,799	733	310.32	4,084	4,380
309.30	2,826	789	310.34	4,099	4,462
309.32	2,853	846	310.36	4,115	4,544
309.34	2,880	903	310.38	4,130	4,626
309.36	2,907	961	310.40	4,146	4,709
309.38	2,934	1,019	310.42	4,161	4,792
309.40	2,961	1,078	310.44	4,177	4,876
309.42	2,988	1,138	310.46	4,193	4,959
309.44	3,016	1,198	310.48	4,209	5,043
309.46	3,044	1,258	310.50	4,224	5,128
309.48	3,072	1,320	310.52	4,240	5,212
309.50	3,099	1,381	310.54	4,256	5,297
309.52	3,128	1,444	310.56	4,272	5,383
309.54	3,156	1,506	310.58	4,288	5,468
309.56	3,184	1,570	310.60	4,304	5,554
309.58	3,212	1,634	310.62	4,320	5,640
309.60	3,241	1,698	310.64	4,336	5,727
309.62	3,270	1,763	310.66	4,352	5,814
309.64	3,299	1,829	310.68	4,368	5,901
309.66	3,328	1,895	310.70	4,384	5,988
309.68	3,357	1,962	310.72	4,400	6,076
309.70	3,386	2,030	310.74	4,416	6,164
309.72	3,415	2,098	310.76	4,432	6,253
309.74	3,445	2,166	310.78	4,448	6,342
309.76	3,474	2,235	310.80	4,464	6,431
309.78	3,504	2,305	310.82	4,481	6,520
309.80	3,534	2,376	310.84	4,497	6,610
309.82	3,564	2,447	310.86	4,513	6,700
309.84	3,594	2,518	310.88	4,530	6,791
309.86	3,624	2,590	310.90	4,546	6,881
309.88	3,654	2,663	310.92	4,562	6,972
309.90	3,685	2,736	310.94	4,579	7,064
309.92	3,715	2,810	310.96	4,595	7,156
309.94	3,746	2,885	310.98	4,612	7,248
309.96	3,777	2,960	311.00	4,628	7,340
309.98	3,808	3,036	311.02	4,645	7,433
310.00	3,839	3,113	311.04	4,661	7,526
310.02	3,854	3,190	311.06	4,678	7,619

→ 4917.5 Cf of  
Storage below  
3" orifice @  
Inv. = 310.45'

**6092 - POST Toll Rev4 (current)**

Prepared by {enter your company name here}

HydroCAD® 10.00-21 s/n 03590 © 2018 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=7.00"

Printed 7/16/2019

**Stage-Area-Storage for Pond 5P: INFIL BASIN #1 (continued)**

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
311.08	4,694	7,713	312.12	5,597	13,059
311.10	4,711	7,807	312.14	5,615	13,171
311.12	4,728	7,901	312.16	5,633	13,283
311.14	4,745	7,996	312.18	5,651	13,396
311.16	4,761	8,091	312.20	5,668	13,509
311.18	4,778	8,187	312.22	5,686	13,623
311.20	4,795	8,282	312.24	5,704	13,737
311.22	4,812	8,378	312.26	5,722	13,851
311.24	4,829	8,475	312.28	5,740	13,966
311.26	4,845	8,572	312.30	5,758	14,081
311.28	4,862	8,669	312.32	5,776	14,196
311.30	4,879	8,766	312.34	5,794	14,312
311.32	4,896	8,864	312.36	5,813	14,428
311.34	4,913	8,962	312.38	5,831	14,544
311.36	4,930	9,060	312.40	5,849	14,661
311.38	4,947	9,159	312.42	5,867	14,778
311.40	4,964	9,258	312.44	5,885	14,896
311.42	4,982	9,358	312.46	5,903	15,014
311.44	4,999	9,457	312.48	5,922	15,132
311.46	5,016	9,558	312.50	5,940	15,251
311.48	5,033	9,658	312.52	5,958	15,370
311.50	5,050	9,759	312.54	5,977	15,489
311.52	5,068	9,860	312.56	5,995	15,609
311.54	5,085	9,962	312.58	6,014	15,729
311.56	5,102	10,064	312.60	6,032	15,849
311.58	5,120	10,166	312.62	6,050	15,970
311.60	5,137	10,268	312.64	6,069	16,091
311.62	5,154	10,371	312.66	6,087	16,213
311.64	5,172	10,474	312.68	6,106	16,335
311.66	5,189	10,578	312.70	6,125	16,457
311.68	5,207	10,682	312.72	6,143	16,580
311.70	5,224	10,786	312.74	6,162	16,703
311.72	5,242	10,891	312.76	6,181	16,826
311.74	5,260	10,996	312.78	6,199	16,950
311.76	5,277	11,101	312.80	6,218	17,074
311.78	5,295	11,207	312.82	6,237	17,199
311.80	5,313	11,313	312.84	6,255	17,324
311.82	5,330	11,420	312.86	6,274	17,449
311.84	5,348	11,526	312.88	6,293	17,575
311.86	5,366	11,634	312.90	6,312	17,701
311.88	5,384	11,741	312.92	6,331	17,827
311.90	5,401	11,849	312.94	6,350	17,954
311.92	5,419	11,957	312.96	6,369	18,081
311.94	5,437	12,066	312.98	6,388	18,209
311.96	5,455	12,175	313.00	6,407	18,336
311.98	5,473	12,284	313.02	6,426	18,465
312.00	5,491	12,394	313.04	6,445	18,594
312.02	5,509	12,504	313.06	6,464	18,723
312.04	5,526	12,614	313.08	6,483	18,852
312.06	5,544	12,725	313.10	6,502	18,982
312.08	5,562	12,836	313.12	6,521	19,112
312.10	5,579	12,947	313.14	6,541	19,243

## Infiltration Area #2

### Stormwater Recharge Calculations

#### CALCULATIONS

##### Recharge Volume, Rv:

$$R_v = A_c \times F$$

Hydrologic Soil Group	Impervious Area (Ac) <sup>1</sup>	Target Depth (F)	Recharge Volume (Rv) Ac-feet
A	0.554	0.6	0.028
B	1.843	0.35	0.054
Total	2.397		0.081

Total Recharge Volume Required = 0.045 Ac-ft

Total Recharge Volume Required (Rv) = 1,952 C.ft

\*Recharge Vol. Provided (from Infil. Area 2) = 11,121.0 C.ft

\*Stage Area Storage at elevation of outlet control structure = 315.55 (See HydroCAD Report)

##### Required Sediment Forebay vol, Fv:

$$F_v = A_c (\text{cu. ft}) \times 0.1 \text{inch of impervious area}$$

<sup>1</sup> Imp. area captured by ponds, Ap = 2.397 Ac

Required Sediment Forebay vol, Fv= 870 C.ft

Sediment Forebay Volume Provided = 3,151.0 C.ft

#### REFERENCES

Table 2.3.2: Recharge Target Depth by Hydrologic Soil Group

NRCS Hydrologic Soil Group	Approx. Soil Texture	Target Depth Factor (F)
A	sand	0.6 inch
B	loam	0.35 inch
C	silty loam	0.25 inch
D	clay	0.1 inch

#### *Drawdown Calculations*

#### CALCULATIONS

##### Proposed Infiltration Area Calculations:

$$\text{Drawdown} = \frac{R_v}{(\text{Rawls Rate})(\text{Bottom Area})}$$

#### REFERENCES

Table 2.3.3: 1982 Rawls Rates

Texture Class	NRCS Hydrologic Soil Group	Infiltration Rate
1 Sand	A	8.27 in/hr
2 Loamy Sand	A	2.41 in/hr
3 Sandy Loam	B	1.02 in/hr
4 Loam	B	0.52 in/hr
5 Silt Loam	C	0.27 in/hr
6 Sandy Clay Loam	C	0.17 in/hr
7 Clay Loam	D	0.09 in/hr
8 Silty Clay Loam	D	0.06 in/hr
9 Sandy Clay	D	0.05 in/hr
10 Silty Clay	D	0.04 in/hr
11 Clay	D	0.02 in/hr

#### NOTES:

##### Input Values

<sup>1</sup> = Refer to Proposed Conditions HydroCAD modeling report

## Stage-Area-Storage for Pond 6P: INFIL BASIN #2 (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
315.04	8,781	5,544	315.56	13,340	11,254
315.05	8,860	5,632	315.57	13,437	11,388
315.06	8,939	5,721	315.58	13,534	11,523
315.07	9,019	5,811	315.59	13,632	11,659
315.08	9,098	5,901	315.60	13,730	11,796
315.09	9,179	5,993	315.61	13,828	11,934
315.10	9,259	6,085	315.62	13,927	12,072
315.11	9,340	6,178	315.63	14,026	12,212
315.12	9,421	6,272	315.64	14,126	12,353
315.13	9,502	6,366	315.65	14,225	12,495
315.14	9,584	6,462	315.66	14,326	12,637
315.15	9,667	6,558	315.67	14,426	12,781
315.16	9,749	6,655	315.68	14,527	12,926
315.17	9,832	6,753	315.69	14,628	13,072
315.18	9,915	6,852	315.70	14,730	13,218
315.19	9,999	6,951	315.71	14,831	13,366
315.20	10,083	7,052	315.72	14,934	13,515
315.21	10,167	7,153	315.73	15,036	13,665
315.22	10,252	7,255	315.74	15,139	13,816
315.23	10,337	7,358	315.75	15,243	13,968
315.24	10,422	7,462	315.76	15,346	14,121
315.25	10,508	7,567	315.77	15,450	14,275
315.26	10,594	7,672	315.78	15,555	14,430
315.27	10,681	7,778	315.79	15,659	14,586
315.28	10,767	7,886	315.80	15,764	14,743
315.29	10,855	7,994	315.81	15,870	14,901
315.30	10,942	8,103	315.82	15,975	15,060
315.31	11,030	8,213	315.83	16,082	15,221
315.32	11,118	8,323	315.84	16,188	15,382
315.33	11,207	8,435	315.85	16,295	15,544
315.34	11,296	8,548	315.86	16,402	15,708
315.35	11,385	8,661	315.87	16,509	15,872
315.36	11,474	8,775	315.88	16,617	16,038
315.37	11,564	8,890	315.89	16,726	16,205
315.38	11,655	9,007	315.90	16,834	16,372
315.39	11,745	9,124	315.91	16,943	16,541
315.40	11,836	9,241	315.92	17,052	16,711
315.41	11,928	9,360	315.93	17,162	16,882
315.42	12,019	9,480	315.94	17,272	17,055
315.43	12,111	9,601	315.95	17,382	17,228
315.44	12,204	9,722	315.96	17,493	17,402
315.45	12,297	9,845	315.97	17,604	17,578
315.46	12,390	9,968	315.98	17,715	17,754
315.47	12,483	10,093	315.99	17,827	17,932
315.48	12,577	10,218	316.00	17,939	18,111
315.49	12,671	10,344	316.01	18,026	18,291
315.50	12,766	10,471	316.02	18,114	18,471
315.51	12,860	10,599	316.03	18,201	18,653
315.52	12,956	10,728	316.04	18,289	18,835
315.53	13,051	10,858	316.05	18,377	19,019
315.54	13,147	10,989	316.06	18,465	19,203
315.55	13,243	11,121	316.07	18,554	19,388

11,121 cf of storage  
below 3" orifice @  
Inv. = 315.55'

**6092 - POST Toll Rev4 (current)**

Prepared by {enter your company name here}

HydroCAD® 10.00-21 s/n 03590 © 2018 HydroCAD Software Solutions LLC

Type III 24-hr 100-year Rainfall=7.00"

Printed 7/16/2019

**Stage-Area-Storage for Pond 6P: INFIL BASIN #2**

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
314.00	2,513	0	314.52	5,172	1,957
314.01	2,555	25	314.53	5,233	2,009
314.02	2,598	51	314.54	5,294	2,062
314.03	2,641	77	314.55	5,355	2,115
314.04	2,684	104	314.56	5,416	2,169
314.05	2,727	131	314.57	5,478	2,223
314.06	2,771	158	314.58	5,540	2,278
314.07	2,816	186	314.59	5,603	2,334
314.08	2,860	215	314.60	5,666	2,390
314.09	2,905	244	314.61	5,729	2,447
314.10	2,951	273	314.62	5,793	2,505
314.11	2,996	303	314.63	5,857	2,563
314.12	3,042	333	314.64	5,921	2,622
314.13	3,089	363	314.65	5,986	2,682
314.14	3,136	395	314.66	6,051	2,742
314.15	3,183	426	314.67	6,116	2,803
314.16	3,230	458	314.68	6,182	2,864
314.17	3,278	491	314.69	6,248	2,926
314.18	3,326	524	314.70	6,314	2,989
314.19	3,375	557	314.71	6,381	3,053
314.20	3,423	591	314.72	6,448	3,117
314.21	3,473	626	314.73	6,516	3,182
314.22	3,522	661	314.74	6,583	3,247
314.23	3,572	696	314.75	6,652	3,313
314.24	3,622	732	314.76	6,720	3,380
314.25	3,673	769	314.77	6,789	3,448
314.26	3,724	806	314.78	6,858	3,516
314.27	3,775	843	314.79	6,928	3,585
314.28	3,827	881	314.80	6,998	3,654
314.29	3,879	920	314.81	7,068	3,725
314.30	3,931	959	314.82	7,139	3,796
314.31	3,984	998	314.83	7,210	3,868
314.32	4,037	1,038	314.84	7,281	3,940
314.33	4,091	1,079	314.85	7,353	4,013
314.34	4,144	1,120	314.86	7,425	4,087
314.35	4,199	1,162	314.87	7,497	4,162
314.36	4,253	1,204	314.88	7,570	4,237
314.37	4,308	1,247	314.89	7,643	4,313
314.38	4,363	1,290	314.90	7,716	4,390
314.39	4,419	1,334	314.91	7,790	4,467
314.40	4,474	1,379	314.92	7,864	4,546
314.41	4,531	1,424	314.93	7,939	4,625
314.42	4,587	1,469	314.94	8,014	4,704
314.43	4,644	1,516	314.95	8,089	4,785
314.44	4,701	1,562	314.96	8,164	4,866
314.45	4,759	1,610	314.97	8,240	4,948
314.46	4,817	1,657	314.98	8,317	5,031
314.47	4,875	1,706	314.99	8,393	5,115
314.48	4,934	1,755	315.00	8,470	5,199
314.49	4,993	1,805	315.01	8,547	5,284
314.50	5,053	1,855	315.02	8,625	5,370
314.51	5,112	1,906	315.03	8,703	5,456

Report To: [Signature]  
Date: [Signature]  
22.81k = 1m

## Adjusted Recharge/WQV Calcs

### Stormwater Recharge Calculations

#### Capture Area Adjustment, Rvadj:

$$R_{vadj} = \frac{A_t}{A_p} x R_v$$

<sup>1</sup> Imp. area captured by ponds, Ap = 5.946 Ac

Total impervious area on site, AT = 6.446 Ac

Recharge volume required, Rv = 14,039 C.ft

Capture Rate= 92% OK

Capture Area Adjustment Factor= 1.08

Adjusted Recharge Volume Required Rvadj = 15,220 C.ft

<sup>1</sup> Total Recharge Volume Provided = 16,038.5 C.ft

#### NOTES:

##### Input Values

<sup>1</sup> = Sum of Recharge Vol. Provided from Infil. Basin 1, Infil. Basin 2

### Water Quality Calculations

#### CALCULATIONS

##### Water Quality Calculation:

$$V_{WQ} = D_{WQ}(ft) x A_T(ft^2)$$

Water Quality Depth = 1 in  
Water Quality Depth , Dwq = 0.08 ft.  
Total impervious area on site, AT = 6,446 Ac.  
AT= 280,788 ft<sup>2</sup>  
Required Water Quality Volume, Vwq = 23,399 C.ft.

#### REFERENCES

1 inch depth
Zone II discharges
IWPA discharges
Critical Area
Runoff from LUHPPPL
Infiltration rate >2.4 inches/hour
1/2 inch depth
Discharge to other areas
8 inch
9 inch
10 inch
11 inch



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## Groundwater Mound Beneath Rectangular Recharge Area

by Glenn M. Duffield, President, HydroSOLVE, Inc.

Share

Hantush (1967) presented the following equations for predicting the maximum height of the water table beneath a rectangular recharge area:

$$h_m^2 - h_i^2 = Z_m(t) = (2w/K)vtS^*(0.5A/(4vt)^{1/2}, 0.5B/(4vt)^{1/2}) \dots \dots (1)$$

$$v = Kb/\epsilon \dots \dots (2)$$

$$\bar{b} = 0.5[h_i(0) + h(t)] \dots \dots (3)$$

where  $h_m$  is maximum height of mound above aquifer base (i.e., maximum saturated thickness of aquifer beneath recharge area);  $h_i$  is initial height of water table above aquifer base (i.e., initial saturated thickness of aquifer);  $K$  and  $\epsilon$  are hydraulic conductivity and storativity (specific yield) of aquifer, respectively;  $w$  is constant rate of percolation from rectangular recharge area of length  $A$  and width  $B$ ;  $\bar{b}$  is a constant of linearization; and the function  $S^*$  is an integral expression (see Hantush 1967). The aquifer is unconfined and assumed to have infinite extent.

If infiltration ends at time  $t=t_0$ , Hantush (1967) applied the principle of superposition to compute the decay of the mound as follows:

$$h_m^2 - h_i^2 = Z_m(t) - Z_m(t-t_0) \dots \dots (4)$$

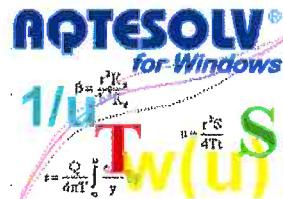
Equation (1) is nonlinear owing to the definition of  $\bar{b}$  in Equation (3); however, the solution is readily obtained by successive approximation.



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### Infil. Basin #1 .

L=Length = ft.

T= time = hours

\* Use RAWLS RATE OF

1.02 in/hr

(0.085 ft/hr) For W

\* K = W x 10

→ A 0.21ft Rise in  
GW does not reach  
bottom of Basin.

Results of Groundwater Mounding Calculation							
Iteration	Solution by Successive Approximation						
	$\bar{b}$	$h_m^*$	% Change				
1	10	10.2106052216473	2.10605221647333				
2	10.1053026108237	10.210737806324	1.29849968484308E-03				
3	10.105368903162	10.2107378889557	8.09262856726889E-07				
K [L/T]	$\epsilon$	$h_i$ [L]	A [L]	B [L]	w [L/T]	t [T]	$h_m$ [L]
.85	.26	10	58.2	58.2	.085	72	10.2107378889557

maximum water-table rise ( $h_m - h_i$ ) at time  $t = 72$  is 0.210737888955665  
decay of mound computed after time  $t = 5.8$

[Return to Groundwater Mounding Calculator](#)

Click [here](#) for a benchmark for this calculator.

Time that pond stops infiltration  
(see Stormwater Spreadsheet)  
Calculation

Hantush mounding calculations with contouring now available in [AQTESOLV](#).



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## Groundwater Mound Beneath Rectangular Recharge Area

by Glenn M. Duffield, President, HydroSOLVE, Inc.

Share

Hantush (1967) presented the following equations for predicting the maximum height of the water table beneath a rectangular recharge area:

$$h_m^2 - h_i^2 = Z_m(t) = (2w/K)vtS^*(0.5A/(4vt)^{1/2}, 0.5B/(4vt)^{1/2}) \dots \dots (1)$$

$$v = Kb/\epsilon \dots \dots (2)$$

$$\bar{b} = 0.5[h_i(0) + h(t)] \dots \dots (3)$$

where  $h_m$  is maximum height of mound above aquifer base (i.e., maximum saturated thickness of aquifer beneath recharge area);  $h_i$  is initial height of water table above aquifer base (i.e., initial saturated thickness of aquifer);  $K$  and  $\epsilon$  are hydraulic conductivity and storativity (specific yield) of aquifer, respectively;  $w$  is constant rate of percolation from rectangular recharge area of length  $A$  and width  $B$ ;  $\bar{b}$  is a constant of linearization; and the function  $S^*$  is an integral expression (see Hantush 1967). The aquifer is unconfined and assumed to have infinite extent.

If infiltration ends at time  $t=t_0$ , Hantush (1967) applied the principle of superposition to compute the decay of the mound as follows:

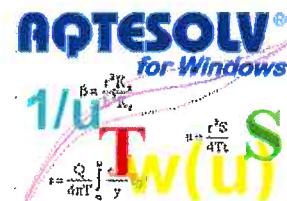
$$h_m^2 - h_i^2 = Z_m(t) - Z_m(t-t_0) \dots \dots (4)$$

Equation (1) is nonlinear owing to the definition of  $\bar{b}$  in Equation (3); however, the solution is readily obtained by successive approximation.



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[in](#) Follow 898

Infil Basin #2

L=Length = ft.

T= Time = hours

\* Use RAWLS RATE OF  
1.02 in/hr  
(0.085 ft/hr) For W

\* K=Wx10

A 0.22 ft Rise in  
GW does not reach  
bottom of basin.

Results of Groundwater Mounding Calculation							
Iteration	Solution by Successive Approximation						
	$\bar{b}$	$h_m^*$	% Change				
1	10	10.2261258279249	2.26125827924901				
2	10.113062913962510	10.2264343735486	3.0172288989716E-03				
3	10.113217186774310	10.2264347902451	4.07469968877194E-06				
K [L/T]	$\epsilon$	$h_i$ [L]	A [L]	B [L]	w [L/T]	t [T]	$h_m$ [L]
.85	.26	10	84.7	84.7	.085	72	10.2264347902451

maximum water-table rise ( $h_m - h_i$ ) at time  $t = 72$  is 0.226434790245117  
decay of mound computed after time  $t = 3.2$

[Return to Groundwater Mounding Calculator](#)

Click [here](#) for a benchmark for this calculator.

Hantush mounding calculations with contouring now available in [AQUESOLV](#).

Time that pond stops infiltration  
( see Stormwater calculation  
spreadsheet )

## **Appendix G – Construction Period Pollution Prevention**

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The project is covered under the National Pollutant Discharge Elimination System (NPDES) Construction General Permit, which will be submitted in place of the Construction Period Pollution Prevention Plan, prior to any land disturbance.

**Appendix H - Operation and Maintenance Plan**

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# STORMWATER OPERATION & MAINTENANCE MANUAL

## ENCLAVE AT BOXBOROUGH

STOW ROAD  
BOXBOROUGH, MASSACHUSETTS

Prepared For:      **BOXBOROUGH TOWN CENTER, LLC**  
                          P.O. Box 985  
                          ACTON, MA

Prepared By:      **DUCHARME & DILLIS CIVIL DESIGN GROUP, INC.**  
                          1092 MAIN STREET  
                          BOLTON, MA 01740

Revised July 15<sup>th</sup>, 2019  
April 16<sup>th</sup>, 2019  
6092



**TABLE OF CONTENTS:**

**1.0 Project Narrative**

- 1.1 *Overview of Drainage System*
- 1.2 *Routine Operation & Maintenance Tasks*
- 1.3 *O&M Schedule*

**2.0 Appendices**

*Appendix A – Stormwater Management System Owners/Operators*

## 1.0 Project Narrative

### 1.1 Proposed Stormwater Management System

The proposed stormwater management system was designed to reduce the peak rate of stormwater leaving the site, promote groundwater recharge, and increase the water quality. Runoff from the proposed development will be conveyed and treated using sedimentation forebays & infiltration basins.

#### Infiltration Basin with Sediment Forebay

Two infiltration basins with sediment forebays will treat the runoff. The volumes of the infiltration basins were designed to reduce runoff rates up to the 100-year storm event, infiltrate the required recharge volume and sized to handle the appropriate water quality volume. The basins are combined with sediment forebays. The sediment forebays are designed to reduce the velocity of flow which will increase the settlement of heavy solids before emptying to the basins. Riprap will also be installed at the inlet of the sediment forebays to reduce the potential for scouring.

#### Deep Sump Hooded Catch Basins

Deep sump hooded catch basins are proposed to convey the runoff from the proposed roadway & roofs to the infiltration basins. These catch basins will discharge to manholes and conventional storm drains.

### 1.2 Operation & Maintenance Tasks

The following activities should be performed routinely to allow for proper functioning of the stormwater system. The following are guidelines referring to each major component of the stormwater management system.

#### Street Sweeping

Street sweeping should be performed at least semiannually. For most effective results, sweeping should be performed by a vacuum style truck in the early spring before spring rain events can wash silt and sediment into the stormwater system. Silt and sediment should be disposed of in accordance with local, state and federal guidelines for hazardous waste.

#### Drain Manholes

Manholes shall be inspected semi-annually for signs of wear, settling, cracking or other fatigue. Manhole casting should be inspected for signs of root intrusion, or significant water infiltration. Manhole sumps should be checked for silt /sediment buildup and cleaned as necessary. Cleaning should be performed by a vacuum truck. Manholes should be resealed as

required and outlets should be inspected incidentally with all structure inspections.

#### Storm Drain Lines

Storm drainage inlets and outlets should be inspected incidentally with all structure inspections. Evidence of debris intrusion or excessive siltation or sedimentation could result in the need to clean a storm drain line. Flushing or jetting should be performed as required. All flushing and jetting should be performed in the direction away from any outlet devices. A vacuum truck should be used at the opposite end of the flushing or jetting to remove any silt or sediment that is cleaned from the storm drain.

#### Deep Sump Catch Basins

Deep sump catch basins shall be inspected at least semi-annually for signs of wear, settling, cracking or other fatigue. Catch basin castings should be inspected for signs of root intrusion, or significant water infiltration. Catch basin sump should be checked for silt/sediment buildup and cleaned as necessary. Cleaning should be performed by a vacuum truck. Catch basins should be resealed as required and outlets should be inspected incidentally with all structure inspections.

#### Infiltration Basins

Infiltration basins are stormwater runoff impoundments that are constructed over permeable soils and require pretreatment from sediment forebays. Runoff from the design storm is stored until it exfiltrates through the soil of the basin floor. The basins were located to capture most of the runoff from the impervious areas of the site.

Infiltration basins are prone to clogging and failure if proper maintenance is not scheduled. The basin should be inspected at least twice per year or after a major storm event to ensure that the basin is operating as intended. The outlet structures should be inspected for clogging or overflow release velocities that are causing scouring or erosion. The upper stage, side slopes, embankments and emergency spillway should be mowed twice a year.

#### Sediment forebay

A sediment forebay is required as a pretreatment device prior to discharging stormwater to the extended dry detention basin. It will provide pretreatment by slowing stormwater runoff and increasing settlement of the sediment. The sediment forebay should be inspected monthly and cleaned of accumulated sediment on a quarterly basis. After sediment removal, repair any damaged vegetation by reseeding or

resodding. Maintain grass at a height of 4-6 inches.

*Stone Rip Rap*

The proposed swales have been designed with angular stone riprap. The stone riprap will be placed approximately 1-foot deep over Tencate Mirafi filter fabric.

Rip Rap should be inspected periodically for signs of failure. Such signs would include, undermining, high velocity wear (displacement of stones downstream), sliding, settlement, siltation, etc. Riprap should be repaired immediately upon the observation of such conditions mentioned.

Periodically, rip rap should be cleaned of silt. Siltation will be most prevalent in low velocity areas (such as directly up-stream of outlet control structures). Silt and sediment should be removed from these areas by hand.

*Grass Swales*

Swales should be checked for scouring, sloughing, erosion and/or accumulation of silt. The vegetation helps reduce velocity of runoff, which helps to maintain the swale, and encourages the sedimentation filtrations prior to exfiltration. Grass should be mowed and kept below 6 inches. Debris and trash should be removed as encountered.

*Contech Vortsentry*

The Vortsentry should be inspected at a minimum of twice a year for evidence of sediment accumulation. When sediment has accumulated to 2 feet or more in the treatment chamber, the vorsentry should be cleaned. Cleaning of the system should be performed during dry conditions when there is little to no flow entering the system. A vacuum truck should be used to clean the sump area of the basin.

*O&M Schedule*

O&M Task	Monthly	Quarterly	Spring	Fall	2-years	As-required
<b>1. Infiltration Basin</b>						
<i>Inspection</i>			x	x		x
<i>Mowing</i>			<b>3-4 times during the growing season</b>			
<i>Remove Debris</i>			x	x		x
<i>Remove Sediment</i>						x
<i>Re-seed</i>						x
<b>2. Sediment Forebay</b>						
<i>Inspection</i>	x		x	x		x
<i>Mowing</i>			<b>3-4 times during the growing season</b>			
<i>Remove Debris</i>			x			x
<i>Remove Sediment</i>			x			x
<i>Re-seed</i>						x
<b>3. Stone Rip Rap</b>						
<i>Inspection</i>			x			
<i>Remove Debris</i>			x			x
<i>Remove Silt/Sediment</i>					x	x
<i>Repair</i>						x
<b>4.</b>						
<b>  Storm drain Lines</b>						
<i>Inspection</i>				x		x
<i>Clean</i>						x
<b>5. Catchbasins</b>						
<i>Inspection</i>			x	x		
<i>Remove Debris</i>						x
<i>Remove Silt/Sediment</i>						x
<b>6. Grass Swales</b>						
<i>Inspection</i>			x			x
<i>Clean</i>			x			x
<b>7. Drain Manholes</b>						
<i>Inspect Rims</i>						
<i>Inspect inside/inlet and outlet pipes</i>			x	x		
<i>Remove sediment</i>					x	x
<b>8. Contech Vortsentry</b>						
<i>Inspect inside/inlet and outlet pipes</i>			x	x		
<i>Remove sediment</i>					x	x

Stormwater Report  
Enclave at Boxborough

July 15<sup>th</sup>, 2019  
Boxborough Town Center, LLC

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**APPENDIX A**

*Stormwater Management System Owners/Operators*

*Stormwater Management System Owners/Operators*

1. Stormwater Management System Owners: TBD
2. Current and future operators: TBD
3. Emergency contact information: TBD
4. Change of trustee: TBD
5. Financial Responsible Party: TBD
6. Routine Maintenance: TBD
7. O&M activities: TBD
8. Record keeping: TBD

**Appendix I - Long Term Pollution Prevention Plan**

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# LONG TERM POLLUTION PREVENTION PLAN

## ENCLAVE AT BOXBOROUGH

STOW ROAD  
BOXBOROUGH, MASSACHUSETTS

**Prepared For:**      **BOXBOROUGH TOWN CENTER, LLC**  
**P.O. Box 985**  
**ACTON, MA**

**Prepared By:**      **DUCHARME & DILLIS CIVIL DESIGN GROUP, INC.**  
**1092 MAIN STREET**  
**BOLTON, MA 01740**

Revised July 15<sup>th</sup>, 2019  
April 16<sup>th</sup>, 2019  
6092



## **1.0      Summary**

This Long-Term Pollution Prevention Plan (LTPPP) has been prepared by Ducharme & Dillis Civil Design Group, Inc. pursuant to the Massachusetts Stormwater Regulations. The proposed development consists of the construction of a 50-unit Active Adult Home Development on the south side of Route 111 just northerly of Burroughs Road.

The layout of the development including the roadways and the locations of buildings and septic systems has been carefully planned to minimize disturbance to the existing land and natural features. The stormwater management system has been designed in accordance with the Massachusetts Stormwater Regulations to provide pretreatment of the stormwater prior to discharge to the resource areas.

## **2.0      Spill Prevention Plan**

No hazardous materials other than normal and common household items are expected to be stored on site after the construction period has ended (refer to the Stormwater Pollution Prevention Plan for details pertaining to spill prevention during construction).

It is expected that normal DEP notification procedures would be triggered for major spills such as home heating oil or propane and natural gas leaks.

## **3.0      Stormwater System O&M**

A Stormwater Operation & Maintenance plan has been prepared for the proposed stormwater management system. Refer to this document for details pertaining to the required inspections, routine maintenance and operation details.

Implementation of the stormwater operation and maintenance plan is critical in order for the site to function as designed, and for the protection of the downstream areas from the potential for scour and erosion.

Special care should be paid to the protection and maintenance of the existing and proposed catch basins that support the drainage system. Refer to the O&M Plan for specific instructions.

## **4.0      Fertilizers, herbicides and pesticides**

Application of fertilizer, herbicides and pesticides shall be performed in a manner consistent with the industry standards for the application.

No application of chemicals is to be performed within the stormwater management areas on the site.

## **5.0     Snow/Salt Management**

### **5.1     *Snow Plowing***

The roadway and driveways are designed to comply with the Town of Boxborough Standards. It is expected that snow plowing practices and procedures will be used similar to those currently employed by Boxborough residents.

### **5.2     *Street Sweeping***

The streets should be swept as needed to reduce the potential for silt build up in the drainage pipes and sump catch basins.

## **6.0     Waste Management**

### **6.1     *Septic Systems***

An on-site sewage disposal system is proposed to service each of the proposed dwellings. The operation & maintenance of these systems will be in accordance with the requirements of the 310 CMR 15,0 (Title 5) regulations. The septic system has been designed in accordance with Title V regulations, and therefore will provide adequate protection against potential pollution.

On-site portable restrooms will be used during construction. The portable restrooms will be cleaned and maintained on a regular basis and disposal will be performed weekly or as required with a private or public waste removal company. All portable restrooms will be removed after construction.

### **6.2     *Solid Waste***

It is expected that the homeowner's will contact directly with a private or public waste removal company.

A dumpster will be located on the site during construction. This area will be the primary area for the on-site storage of solid waste prior to pick-up by a waste management company.

#### **4.0 Plans**

Stormwater Report  
Enclave at Boxborough

July 15<sup>th</sup>, 2019  
Boxborough Town Center, LLC

**Pre-development Watershed Plan**

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Stormwater Report  
Enclave at Boxborough

July 15<sup>th</sup>, 2019  
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**Post-development Watershed Plan**

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